1.0 Abbreviations

1.1 The following abbreviations are used:

CA: Contract Administrator (for definition refer to preliminaries).
ROC: RoC Consulting.
SSL: Structural Slab Level.
FFL: Finished Floor Level.
SOP: Setting Out Point.
TOS: Top of Steel at intersection with column centreline.
TOC: Top of Concrete at support locations.
UNO: Unless Noted Otherwise.
TBC: To be confirmed.
FW: Fillet Weld.
CFW: Continuous Fillet Weld.
FPFW: Full Profile Fillet Weld.
FPBW: Full Penetration Butt Weld.
PPBW: Part Penetration Butt Weld.
EA: Equal Approved.
MMT: Beam end connection to be designed for bending and/or torsional moments.

2.0 General

2.1 ROC drawings are to be read in conjunction with all relevant documents, specifications, architectural and services drawings, including approved builders work drawings. The contractor should notify ROC of any discrepancies between the structural drawings and the specifications or other drawings.

2.2 Do not scale from the drawings or use the computer digital data. Only figured dimensions are to be used.

2.3 It is the responsibility of the lead consultant to check the setting out provided on these drawings and to check the coordination of the details indicated with other elements of the building fabric.

2.4 All dimensions are in millimetres unless noted otherwise.

2.5 All levels are in metres above Ordnance Datum unless noted otherwise.

2.6 SSL is the top surface level of the concrete slab immediately adjacent to a vertical support.

2.7 Columns are located centrally about intersecting grid lines, UNO.

2.8 Refer to Architect's drawings for the following information or to specialist design & detail:
2.8.1 Setting out dimensions not shown on structural drawings,
2.8.2 Chamfers, concrete kerbs, slopes,
2.8.3 Surface finishes to concrete elements and details of applied finishes,
2.8.4 Details and specification of all damp proofing, waterproofing and sealants,
2.8.5 Details and specification of insulation and other applied finishes
2.8.5 Details and specification of all fire protection,
2.8.6 Junction details of gas membranes, where present.

2.9 Refer to Services Engineer's drawings for the following information:
2.9.1 Drainage layouts and details at all levels,
2.9.2 Setting out and details of gullies and pop-ups etc.
2.9.2 Details of lightning protection to be fixed to the structure,
2.9.3 Builders work details,
2.9.4 Service pit requirements.

2.10 Refer to Lift Manufacturer's drawings for the following information:
2.10.1 All secondary lift steelwork, including lift support and lifting beams,
2.10.2 All brackets, inserts and fixings required around lift shaft walls and pits.

2.10 Refer to Contractor and/or Specialist Subcontractor for the following information:
2.11.1 Any dewatering requirements,
2.11.2 Details of cast-in fixings, brackets, inserts, etc. for cladding, roofing, balustrades, etc.
2.11.3 Details of plinths, shoes or other supports to floor-mounted plant and equipment.

2.11 ROC Consulting are responsible for the design of the primary frame elements generally to satisfy the permanent in-service loading. The following elements are to be designed and detailed by the contractor:

- All steel frame connections including baseplates and other steel to concrete connections
- Post-tensioned concrete slabs including any areas requiring traditional reinforcement and any downstand or upstand beams integral with the slabs including transfer beams
- Stairs in steel precast concrete or timber
- Precast concrete frames and floor slabs
- Metal decking forming composite concrete floor slabs.
- Piles, bored or pre-cast
- Hand rails & balustrades
- Secondary steelwork supporting cladding or roof sheeting
- All temporary steelwork
- All other secondary steelwork.

2.12 The Contractor shall be responsible for the design, fabrication, erection and removal of all temporary works and shall provide all temporary bracing and back propping necessary to maintain structural stability during construction.

2.13 Specific temporary works requirements or construction sequence constraints if and where applicable are noted on the ROC drawings to which they relate.

3.0 Basis of Design

3.1 To be read in conjunction with the ROC Structural Design Statement.

3.2 Vertical imposed loads and superimposed dead loads are described on the relevant ROC drawings. Dead loads shall be calculated to reflect the material utilised. Refer also to the Structural Loading Plans where provided.

3.3 The structure has been designed to achieve the serviceability criteria set out in the movement and tolerance report.

3.4 Due account should be taken in the design of the cladding and finishes of anticipated thermal movements in addition to the structural deflections as noted above.

3.5 The structure is required to achieve the following fire resistance periods (TBC by the fire strategist if there is one or for the architect to agree with the Building Control officer if not):

3.5.1	All structural elements, generally	120 Minutes
3.5.2	Compartment floors, beams	120 Minutes
3.5.3	Composite slabs	120 Minutes
3.5.4	Lift shaft walls / stair walls	120 Minutes
3.5.5	Roof structure	120 Minutes

3.6 For concrete frame structures the assumption is that the fire rating will be inherent in the design of the elements without the requirement for any fire protection UNO. Where additional protective measures are required to concrete frame elements this is to be identified on the drawings.

3.7 For steel frame structures the assumption is that the fire rating will be achieved by the application of fire protection UNO. Where fire protection is not required to steel frame elements this is to be identified on the drawings.

3.8 Refer to architect's drawings and specifications for details of all fire protection.

3.9 For reinforced concrete ground slabs and basement structures, it is assumed that the concrete will be protected from contact with the ground and ground water (where present) via the application of a proprietary externally applied tanking membrane to the architect's design and specification and the concrete and reinforcement have been designed, detailed and specified on this basis. Any changes to the method of water exclusion may affect the durability of the concrete.

3.10 The design and detailing of waterproofing measures to the basement slab and retaining walls is subject to detailed design by a waterproofing specialist in accordance with NHBC requirements. For the purpose of costing an allowance should be made for all concrete in contact with the ground to require waterproofing admixtures

3.11 The Consequence Class of the building has been assessed as **CC3**

3.12 The Execution Class required for fabrication and construction has been assessed as **EX3**

4.0 Foundations

4.1 Note Removed

4.2 The Contractor will be responsible for the validation of the formation for pad/raft foundations and should make allowance for any additional excavation to remove weak materials which do not achieve the specified minimum strength required at formation level. Where excavation is required below the pad/raft formation levels this shall be backfilled with mass concrete.

4.3 Where mine workings are present the contractor will be responsible for probing and/or grouting of the site in accordance with the recommendations of the site investigation.

4.4 Material at formation level shall be blinded with lean mix concrete within 4 hours of exposure, unless agreed otherwise with the CA.

4.5 Services passing through structure are to be isolated and protected in accordance with the Services Engineer's specification and typical details on ROC drawings. Where drainage passes through foundations either provide rocker connection or use oversized sleeves with 50 clear gap all round. For duct and service pit locations, incoming services, drainage, mat wells, etc. see drawings by others. The Contractor is to co-ordinate the requirements and provide sleeves through foundations, etc. as required.

4.6 For preparation of the formation below ground bearing floor slabs refer to the Building Earthworks Specification and any relevant details provided on the drawings.
4.7 For suspended ground floor slabs the formation level must be assessed by the Contractor and where necessary improved to ensure it is capable of adequately supporting the construction loads for the slab.

4.8 All items passing through ground floor slabs are to be surrounded with a compressible filler (e.g. Miothene or EA) with damp proof membrane installed as the Architect's details.

4.9 Refer to Steel Fabricator's details for holding down bolt assemblies to be cast into foundation.

4.10 Provide pairs of diagonal B16 bars in ground slabs at all re-entrant corners and box outs in the slab.

4.11 Areas of over excavation below slabs are to be backfilled with 6N fill material compacted in layers. In particular, thorough compaction is required to fill in areas of excess excavation below ground bearing slabs around foundations, lift pits, etc.

5.0 Earthworks

5.1 Refer to the Building Earthworks Specification, Site Investigation Report and, where appropriate, any Remediation Strategy and/or Materials Management Plan. Give notice if the conditions encountered differ from those anticipated from the information provided.

5.2 Any contaminated soils should be treated or disposed of in accordance with the requirements of Site Investigation Report and any Remediation Strategy. Seek advice if unexpected sources of contamination are encountered.

5.3 Prior to the commencement of excavation, refer to the utilities plans provided for details of underground services. Notwithstanding this it is the responsibility of the contractor to scan the areas to be excavated and satisfy themselves that they are clear of buried services prior to starting work.

5.4 The Contractor is to protect all sub-structures, foundations, underground services and drainage, including any cast in protruding reinforcement, from any mechanical damage when undertaking excavation and filling activities.

5.5 For excavations adjacent to existing footings no excavation is permitted that extends below a line taken at 30% to the horizontal from the base of the existing footing. Should such excavations be required obtain instructions before proceeding and submit all temporary works proposals for review/comment.

5.6 All ground bearing earth retaining structures in external works areas have been designed upon an assumed net allowable ground bearing pressure of **150** kN/m2 and should be founded in the **Stiff Clay.** Prior to construction the formation is to be inspected and unsuitable material is to be removed as directed and replaced with mass concrete fill down to an approved bearing strata.

5.7 Fill is not to be placed behind reinforced concrete or masonry retaining walls or pits until the concrete or masonry has attained its full design strength and the structural configuration complies with the design intent.

5.8 Do not use fill materials which would, either in themselves or in combination with other materials or ground water, give rise to a health hazard, damage to building structures, an instability in the filling or in other way be considered deleterious.

5.9 Where required, gas protection measures, including passive venting layers and membranes below buildings, shall be installed in accordance with the manufacturer's recommendations and details. The Contractor is responsible for the design and construction of the gas protection measures to suit the performance requirements as detailed in Site Investigation Report.

5.10 Any mine shaft cap design or treatment if identified by the Site Investigation report(s) shall include the installation of a gas vent pipe where required by the Coal Authority (or other relevant body). The contractor's design shall include for connection and carrier pipe to vent this pipe at the perimeter of the building. Design of the mine shaft capping slab is to be in accordance with the Coal Authority requirements and the Contractor shall obtain Coal Authority approval of the details prior to construction.

5.11 Refer to Service Engineers drawings and specifications for identification and resolution of utilities easements.

6.0 Piling/Ground Improvement

6.1 Refer to the Piling/Ground Improvement Specification, the Site Investigation Report and foundation drawings. Give notice if the conditions encountered differ from those anticipated from the information provided.

6.2 All piles/ground improvement are to be contractor designed to suit the requirements of the performance specification and design loads as specified on the drawings. The piling/ground improvement technique is to be determined by the contractor to suit the ground conditions.

6.4 Ground improvement should be designed to provide the specified net allowable bearing pressure across the whole area of the proposed foundations/slabs as specified on the drawings.

6.5 Layout drawings, schedules and supporting calculations are to be submitted for comment at least two week prior to commencement of works on site.

6.6 All piling/ground improvement should be undertaken from a suitably prepared formation level. The main contractor is responsible for the design of the working platform to suit the ground conditions and loading requirements specified by the piling/ground improvement contractor.

6.7 The main contractor shall be responsible for all setting out and shall provide the specialist subcontractor with all necessary lines and datum levels and set out each pile. The subcontractor shall check and agree in writing the main contractor's setting out of the piles.

6.8 All piles are to be installed within the permissible tolerances specified in the ICE Specification for Piling and Embedded Retaining Walls. The positions of any piles installed outside of these tolerances are to be reported immediately to the engineer.

6.9 The main contractor is to agree and record the lengths of all piles prior to placing of concrete.

6.10 Concrete for piles should be to the piling contractors' specification to suit the design requirements and ground conditions, but a C28/35 DS-2 mix should be provided as a minimum.

6.11 All piles are to be integrity tested and the results submitted to the engineer. Where load tests are specified the locations of the test piles/areas are to be agreed with the engineer. All piles/ground improvement must be designed to comply with the specified load test settlement criteria.

6.12 All piles should be taken to platform level by the piling contractor. Piles are to be carefully broken down to cut off level by the main contractor to leave a 75mm pile projection into the underside of the foundation.

6.13 All pile reinforcement is to project a minimum of 600mm or 32 times the bar diameter, whichever is greater, into the foundation structure UNO. De-bonding foam is to be applied to reinforcement within cast-in situ piles.

6.14 Provide 50mm lean mix concrete blinding below all pile caps, ground beams, capping beams, and foundations constructed in improved ground.

6.15 The piling/ground improvement contractor is to advise the main contractor of any facilities and attendance to be provided in advance of any contract agreement.

6.16 Pile testing requirements to be determined by the piling designer and appropriate design factors in accordance with EC 1997-1 applied

7.0 Concrete

7.1 To be read in conjunction with the Structural Concrete Specification

7.2 Structural concrete is to be constructed in accordance with the National Structural Concrete Specification (NSCS) 4th edition, except as amended by the project Concrete Specification and as noted on the drawings.

7.3 Refer to the ROC Structural Concrete Specification for concrete grades.

7.4 All reinforcement loose bars and mesh shall be minimum grade B500B deformed type 2 bars and scheduled in accordance with BS 8666:2005 UNO on the structural drawings.

7.5 Cover to reinforcement and all laps to be as shown on the reinforcement drawings. Welding of reinforcement is not permitted without prior approval.

7.6 Refer to Architect's drawings and specification for concrete finishes. It is assumed where columns are visable thy are to recieve a special finish. Refer to architects details for specification of the finish and extent over which the finish is required

7.7 Floor surfaces in plant rooms, exposed surfaces in car parks and concrete surfaces forming the internal sides of air shafts are to be dust sealed. Refer to Architect's details for sealant specification.

7.8 50mm blinding concrete is to be placed under all reinforced concrete in contact with the ground, UNO.

7.9 Openings shown on the structural drawings are to be checked by the Contractor against relevant service builder's work drawings prior to construction. Any discrepancies must be drawn to the attention of CA.

7.10 All holes in reinforced concrete are to be formed. Reinforcement shall be displaced around services holes 250 x 250mm or smaller refer to Services Engineer's and Architect's drawings for setting out of service holes 250 x 250mm or smaller. Only holes larger than 250 x 250mm are specifically shown on ROC drawings

7.11 No cutting or removal of placed concrete is permitted without prior acceptance by the engineer. Holes/chases etc. other than those shown on the structural drawings are not to be made without prior approval.

7.12 The positions and details of all construction joints not shown on the drawings are to be agreed with the CA before work commences.

7.13 Starter kickers to all walls, columns, etc. to be 75mm high UNO on reinforcement drawings. Starter sections in watertight construction to be 150mm high UNO.

7.14 For details of lightning protection requirements, refer to Service Engineer's drawings.

7.16 Allowance should be made by the contractor for camber in the formwork such that the alignment of the completed structure conforms to that shown on the construction drawings. Similarly allowance should be made in the lengths of columns and walls such that the floors achieve the specified level (within tolerance) allowing for the axial shortening of the columns/walls due to the both the short term and long term axial load effects.

8.0 Steelwork – in addition, refer to Specification for Steelwork

8.1 To be read in conjunction with the Structural Steelwork Specification

8.2 The design detailing fabrication and erection of all steelwork shall comply with the 'The National Structural Steelwork Specification for Building Construction' 5th edition CE Marking version, published by BCSA/SCI, except as modified by the project steelwork specification. In addition all work shall comply with the requirements of BS EN 1090 as applicable to the Execution Class specified (ref note 3.11).

8.3 Beam reference given on ROC drawings shall be used by the Contractor on the fabrication drawings.

8.4 UNO all plates, flats and rolled sections shall be grade shall be S355 to BS EN 10025, of appropriate sub-grade to suit element location, fabrication detail and thickness (contractor to determine) and all hollow sections shall be hot finished grade S355J2H to BS EN 10210. For plates and sections in welded joints where there is a high risk of lamellar tearing, Z-quality material is to be used.

8.5 All connections, including baseplates and holding down bolts, are to be designed by the Contractor for the forces shown on the drawings and schedules. Forces are specified at the ultimate limit state. Loads given relate only to the permanent works. All loading conditions resulting from temporary works or the construction sequence shall be determined by and designed for by the Contractor. At tender, in the absence of specified loads, the contractor should assume all connections are designed for a minimum shear load of 0.5Pv plus a nominal axial load +/- 25kN

8.6 Where connections are detailed on the drawings these are provided to indicate design intent only. Final connection design is still by the contractor and the contractor is to confirm his acceptance of the proposed details in writing prior to start of fabrication. Connections of members will be located centrally about the respective axis of the members unless specifically noted otherwise.

8.7 In addition to the specified design loads all connections should be designed for the minimum tie forces for the avoidance of disproportionate collapse as required by the relevant design code.

8.8 All structural connections are to have a minimum of 4 no. M20 grade 8.8 bolts generally or 2 no. M20 grade 8.8 bolts for bracing members.

8.9 All structural welds are to be a minimum of 6mm fillet weld. For PPBW the specified penetration should be to suit the applied loads but not less than 2√t where t is the thickness of the thinner part joined. Single sided PPBW's that are asymmetric relative to the parts joined should not be used where the joint is in tension or compression.

8.10 All contractor designed work, drawings and calculations, are to be submitted for review by ROC at least two weeks prior to the commencement of fabrication.

8.11 All fastener assemblies shall be manufactured to harmonised BS EN standards and supplied complete with CE markings as follows:
Ordinary bolts to BS EN 15048-1
Pre-loaded bolts to BS EN 14399-1

8.12 Grade 8.8 bolts shall be supplied either sherardized or spun galvanised. Bolts of higher strengths are to be supplied without any applied zinc coatings.

8.13 Foundation bolts shall be in accordance with BS 7419. Where grouted sleeves are used no reliance should be placed on the grout to resist horizontal shear. The Contractor's design should ensure such horizontal forces can be transmitted by either base friction or directly by setting the baseplate into a shallow pocket, or in the case of high shear forces by the provision of a shear key fixed to the underside of the baseplate.

8.14 Grout around foundation bolts and under base plates is to be non-shrink and have a minimum characteristic strength at 28 days of 40N/mm².

8.15 Note Removed

8.16 Note Removed

8.17 Shear Studs, Welding and Testing

8.17.1 Welders to be certified by the equipment manufacturer or other recognised authority as competent for the work.

8.17.2 Trial welding: before starting demonstrate suitability of the materials, equipment and welding system proposed. Fix not less than 10 studs for testing. During the work, at the beginning of each shift each weld is to fix not less than two studs for testing.

8.17.3 Testing trial welds: each weld is to exhibit a full 360° flash. Subject each test stud to a 30° bend test, welds must show no sign of cracking.

8.17.4 Production welds are to be visually free from cracks and lack of fusion and be capable of developing at least the ultimate strength of the stud.

8.18 All new steelwork to be shot blasted to BS EN ISO 8501-1, preparation grade Sa 2½ including all additional fitments.

8.19 Immediately after shot blasting all steelwork is to be coated with one coat of an approved pre-fabrication primer. After fabrication all steelwork is to be finished in accordance with the specification.

8.20 Where sprayed fire protection is to be applied, the Contractor is to ensure compatibility of the fire protection with the applied paint system.

8.21 Steel columns noted as concrete encased to have all mill scale removed during fabrication and left unpainted. Concrete casing to be minimum 50mm all round and reinforced with D49 wrapping fabric UNO.

8.22 All steelwork below ground to have a minimum 100mm C28/35 concrete encasement all round.

8.23 After erection all damaged areas of paintwork are to be touched up with an approved primer to the satisfaction of the engineer.

8.24 Exposed steelwork is to be left in a primed state for decoration by the main contractor to the architect's specification.

8.25 Primed steelwork in cavities or encased in brickwork is to be further protected with two coats of bitumen paint.

8.26 All galvanised steelwork to be shot blasted to BS EN ISO 8501-1, preparation grade Sa 2½ using chilled angular iron grit grade G24 to give a coarse surface profile, followed by chemical cleaning and galvanising to give a mean coating thickness of 140microns.

8.27 Site welding shall only be carried out with the prior written consent of ROC and after submission and acceptance of fabricator's welding procedure.

8.28 All steelwork considered as visually exposed to have regards to the high quality and appearance of connections, welds, members etc. required throughout the steel frame. Welds, edges and surface imperfections are to be prepared to preparation grade P2 to BS EN ISO 8501.

8.29 The Contractor is to supply and install all lift steelwork as required by the lift manufacturer's design and drawings.

8.30 No drilling, cutting or modification of steelwork is permitted without prior acceptance by ROC.

8.31 The Contractor is to allow for co-ordination with other contractors whose work interfaces with the steel frame. This co-ordination may include the design, fabrication, and erection of steel elements, brackets and fixings specified by other contractors. Work to be specified by other contractors is not shown on ROC drawings.

8.32 Where steelwork is to be erected within an existing building or is required to connect to an existing structure, it is the responsibility of the steelwork contractor to survey the existing building to confirm the length and setting out of the new steelwork elements prior to fabrication.

8.33 Temporary stability of the steel frame is to be addressed by the steelwork sub-contractor's method statement. Refer to the drawings, steelwork specification and CDM risk assessments for any sequence constraints or temporary works required by the design. Notwithstanding the information provided, the Contractor is responsible for assessing the requirement for and the design of all temporary works to suit the proposed construction sequence and method.

8.34 For plate assemblies which are cast into walls, the Contractor shall survey the as-built locations of the cast-in plates prior to commencement of fabrication and allow suitable tolerances in the design and detailing of the connections.

8.35 Allowance should be made by the contractor for pre-cambers in beams and pre-sets in trusses and cantilevers such that the alignment of the completed structure conforms to that shown on the construction drawings. Similarly allowance should be made in the lengths of columns such that the floors achieve the specified level (within tolerance) allowing for the axial shortening of the columns due to axial load effects.

8.36 Fire protection to steelwork to be in accordance with the architect's drawings and specification.

8.37 For orientation of columns refer to ROC drawings

8.38 The Contractor shall clearly identify the location of site connections on the fabrication drawings together with details of and calculations for those connections.

8.39 Where cold formed steel sections are shown on the drawings, the steel contractor is to provide all necessary struts, anti-sag rods, diagonal ties, etc, as recommended by the manufacturer.

			Cover (mm)			
Element	**Spec Ref.**	**Concrete Grade**	**T**	**B**	**NF**	**FF**
Suspended ground floor slab & retaining wall/upstands	Type A	C35/45	40	75	40	75
Suspended internal slabs	Type B	C35/45	30	30	-	-
Suspended external slabs (podium & roofs)	Type C	C35/45	50	30	-	-
In-situ columns	Type D.1	C50/60	-	-	40	40
In-situ columns (below ground)	Type D.2	C50/60	-	-	40	40
External columns	Type D.3	C50/60	-	-	40	40
In-situ walls	Type E.1	C50/60	-	-	40	40
In-situ walls (below ground)	Type E.2	C50/60	-	-	40	40
Substructure (core base, pilecaps & ground beams)	Type F	C35/45	75	75	75	75
Transfer Beam (L01)	Type G	C55/67	30	30	40	40

Note FF relates to ground facing side of any walls/upstands

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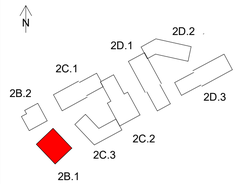
WORK IN PROGRESS

The information on this drawing is an optimised solution based upon external information as of 14/07/2023 It may be subject to change through external influences such as client brief and scope changes, services BWIC, sub-contractor requirements etc. Therefore a suitable contingent approach must be applied where the information is used for strategic decision making, procurement or construction in advance of final design requirements from other designers and sub-contractors.

NOTES

1. RoC drawings are to be read in conjunction with relevant documents, specifications, architectural and services drawings, including approved builders work drawings. The contractor should notify CA of any discrepancies between the structural drawings and specifications or other drawings.
2. Do not scale from the drawings or the computer digital date. Only figured dimensions are to be used.
3. It is the responsibility of the lead consultant (architect) to check the setting out provided on these drawings and the co-ordination of the frame with the envelope & other elements of the building.
4. All dimensions are in millimetres unless noted otherwise.
5. All levels are in metres above Ordnance Datum unless noted otherwise.
6. For Concrete Grades and RC Estimates see general notes drg 8050.
7. This drawing is to be read in conjunction with drg's 8050, 9050, 9051 & 9052.
8. For Loading Information refer to Drg 8080+

VOLUME / BLOCK PLAN



REV	DATE	DETAILS	DRN	CHK
P03	20.12.2024	FOR APPROVAL	SJB	DC
P02	13.12.2024	FOR APPROVAL	SJB	DC
P01	31.07.2023	STAGE 3B ISSUE	SJB	DC

DRAWING STATUS
FOR APPROVAL

RIBA STAGE
STAGE 4

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DRAWN SJB	DATE 11.2022	SCALES As indicated
CHECKED MJN	DATE 11.2022	THE ABOVE SCALES APPLY WHEN PLOTTED AT A1 SIZE -DO NOT SCALE-
APPRVD MJN	DATE 11.2022	EMAIL FOR DRAWINGS: info@rocconsulting.com

CLIENT
FEC

PROJECT TITLE
RED BANK DEVELOPMENT

DRAWING TITLE
BLOCK 2B1 GENERAL NOTES

DRAWING No. RDB-ROC-2B1-XX-DR-S-DR-8050
REV. P03