SHAPE
ENGINEERING

GREENBROW SOCIAL CLUB

DRAINAGE OPERATION MANAGEMENT AND MAINTENANCE

REVISION A

22ND SEPTEMBER 2025

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SHAPE ENGINEERING

DOCUMENT HISTORY

REV	DATE	DESCRIPTION	PREPARED BY	CHECKED BY	APPROVED BY
A	03.11.2025	First Issue	Louie Regan Civil Engineer	Nick McGorry Director	Nick McGorry Director

CONTACT LIST

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1.0 INTRODUCTION

Background

1.1 Shape Engineering have been instructed to prepare a Drainage Operation and Maintenance Manual for a proposed development at the former Greenbrow Social Club, Manchester.

1.2 This report is based on current best practice guidance as outlined within Ciria 753 The SuDS Manual.

Scope of Report

1.3 This manual is intended to give an overview of the operation, maintenance and management for the range of drainage features included with the design and in relation to typical details only.

1.4 Where proprietary products are specified the manufacturer's instructions and recommendations should be followed in priority to this document unless specifically noted otherwise due to project constraints.

1.5 The recommended operations and frequencies are typical only and should be more frequent initially to ensure that there are no unforeseen issues with the operation and then adjusted to suit the site requirements.

2.0 DRAINAGE STRATEGY

2.1 A proposed Drainage Strategy drawing has been provided in Appendix A.

2.2 The surface water drainage strategy for the development will be to collect and convey the surface water runoff and mitigate flood risk to the site, surrounding areas and downstream, by restricting the discharge rate into the existing combined sewer on Greenbrow Road. Suitable attenuation within the development boundary is to be provided to accommodate this strategy.

2.3 The drainage network is to remain under private ownership and maintained by a management company.

2.4 The surface water drainage will discharge at greenfield discharge rates limited to 5.0l/s into a new combined sewer located to on the western boundary that will outfall west into the existing public combined sewer network within Crow Wood Lane.

2.5 Attenuation will be provided in the form of an online cellular storage tank.

2.6 As the flows are generally being attenuated on site, there will be a period of time after storm events where the network is still partially or fully surcharged and is draining down. Where this surcharging is still present after 48hrs appropriate action should be taken as in Section 3.

2.7 Foul water from dwellings will be collected via a traditional piped network to convey the flows via gravity to the existing public combined sewer network located to on the eastern boundary on Greenbrow Road.

3.0 SURFACE WATER DRAINAGE

3.1 This section is intended to give an overview of the operation and maintenance for the drainage features included with the drainage strategy and in relation to typical details. Where proprietary products are specified, the manufacturer's instructions and recommendations should be followed in priority to this document unless specifically noted otherwise due to project constraints.

3.2 The recommended operations and frequencies are typical only and should be more frequent initially to ensure that there are no unforeseen issues with the operation and then adjusted to suit the site requirements.

3.3 The surface water network is to be designed to accommodate from 1 in 100 year storm rainfall event plus an allowance for 45% climate change. As the flows are to be attenuated on site there will be a period of time after storm events where the network is still partially or fully surcharged and is draining down. Where this surcharging is still present after 48hrs appropriate action should be taken as noted in this section.

Components

3.4 The following components have been included within the drainage design for the development:

- Drainage Channels and Gullies
- Pipes
- Inspection, Manhole and Catchpit Chambers
- Cellular Storage Tank
- Flow Control Units

3.5 A suitable maintenance strategy should be adopted to ensure the drainage network is cleaned regularly and the routine maintenance and cleansing regime should be documented.

3.6 The maintenance of the drainage network will be the responsibility of a private maintenance company.

3.7 A copy of the final drainage layout is provided in Appendix A.

3.8 It is recommended that the drainage system is inspected as a minimum twice a year, with the system also being inspected after any major storm event.

3.9 Significant sediment deposition is likely in areas used for storage, so a post clean-up operation may be required including the removal of litter, vegetation, sewerage debris and larger objects.

3.10 Long-term management practices include monthly sweeping of external paved areas. The sweeping program will remove sand and contaminants directly from paved surfaces before they become mobilised during storm events and transported to the drainage system.

3.11 During the winter months, drainage features such as gullies and channels should be cleared of ice, snow, debris, leaves, litter etc.

3.12 Sediment/material removal should be undertaken in consultation with the environmental regulator to confirm appropriate protocols.

4.0 DRAINAGE CHANNELS AND GULLIES

Location and description

4.1 The locations of the gullies are to be indicated on the drainage drawing in Appendix A, upon completion of the detailed design.

4.2 It is the responsibility of the management company to maintain private gullies within access road, parking/shared parking areas and hard landscaping areas across the development

4.3 The features have been designed in accordance with all relevant standards, plus Statutory Bodies' and manufacturers requirements.

4.4 Drainage gullies should be inspected and cleaned in accordance with the manufacturer's details.

4.5 Locking bolts should be replaced and sufficiently tightened, taking care that the bolt heads do not stand above the top surface of the cover or grate. If covers are allowed to move within their frame, this may cause damage to the frame or seating.

4.6 Sediment/material removal should be undertaken in consultation with the environmental regulator to confirm appropriate protocols.

Inspection and Maintenance Regime

4.7 Regular inspection and maintenance are important for the effective operation of the features. A regular inspection regime is very important to ensure the correct functionality of the surface water drainage network.

4.8 Sediment/material removal should be undertaken in consultation with the environmental regulator to confirm appropriate protocols.

Drainage Gullies Schedule

Maintenance Schedule	Required Action	Frequency
Monitoring (to be undertaken more regularly within the first year of operation and adjusted as required).	Initial Inspection	Half yearly and after large storms.
Regular maintenance / inspection	Litter and debris removal	Monthly or as required.
	Check and remove large vegetation growth near channel runs.	Monthly or as required
	Inspect for evidence of poor operation and/or weed growth. If required, take remedial action. Inspect silt accumulation rates and establish appropriate brushing frequencies. Silt can also be caused by adjacent landscaping areas which should be reprofiled to provide a flat area or berm adjacent to the paving.	3-monthly, 48 hours after large storms.
Remedial Action	Inspect gullies and rod through poorly performing gullies as initial remediation.	Monthly or as required

5.0 PIPES

Location and Description

5.1 The locations of the pipes are indicated on the drainage drawing provided in Appendix A.

5.2 The features have been designed in accordance with all relevant standards, plus Statutory Bodies' and manufacturers requirements.

5.3 Pipes are proprietary products, and the materials can vary across the site and as such where used the manufacture's recommendations should be followed.

5.4 It is the responsibility of the management company to maintain the pipes within the shared ownership areas and public open space areas, which covers the entirety of the development

Operation

5.5 Pipes are intended to be the main conveyance across the development and where oversized they form the attenuation volume required by the limitation of the discharge rate. They are intended to be dry except for during rainfall events.

5.6 These have been designed to be self-cleansing where possible for smaller diameter pipes and for larger diameters the risk is reduced due to the overall pipe size.

5.7 Access for maintenance will be provided through access chambers, manholes and rodding eyes.

Inspection and Maintenance Regime

5.8 Regular inspection and maintenance are important to identify areas which may have been obstructed/clogged and may not be drain correctly thus exposing the development to a greater level of flood risk.

5.9 Sediment/material removal should be undertaken in consultation with the environmental regulator to confirm appropriate protocols.

Pipes Schedule

Maintenance Schedule	Required Action	Frequency
Monitoring (to be undertaken more regularly within the first year of operation and adjusted as required)	Initial inspection should be provided as post construction CCTV survey.	N/A
	Inspect for evidence of poor operation via water level in chambers. If required, take remedial action.	3-monthly, 48 hours after large storms.
Occasional maintenance	Check and remove large vegetation growth near pipe runs.	6 monthly.
Remedial actions	Rod through poorly performing runs as initial remediation.	As required.
	If continued poor performance jet and CCTV survey poorly performing runs.	As required.
	Seek advice as to remediation techniques suitable for the type of performance issue and location.	As required If above does not improve performance.

6.0 INSPECTION, MANHOLE AND CATCHPIT CHAMBERS

Location and Description

6.1 The locations of the inspection, manhole and catchpit chambers are as indicated on the drainage drawing within Appendix A.

6.2 The features have been designed in accordance with all relevant standards, plus Statutory Bodies' and manufacturers requirements.

6.3 Inspection, manhole and catchpit chambers are proprietary products, and the materials can vary across the site and as such where used the manufacture's recommendations should be followed.

6.4 The appropriate health and safety equipment must be used when accessing manholes. Confined space certificates must be held by any personnel entering a manhole.

6.5 It is the responsibility of the management company to maintain the inspection chambers, manholes and catchpits within the shared ownership areas and public open space areas, which covers the entirety of the development

Inspection and Maintenance Regime

6.6 Regular inspection and maintenance are important for the effective operation of the features. A regular inspection regime is very important to ensure the correct functionality of the surface water drainage network.

6.7 Sediment/material removal should be undertaken in consultation with the environmental regulator to confirm appropriate protocols.

Chambers Schedule

Maintenance Schedule	Required Action	Frequency
Monitoring (to be undertaken more regularly within the first year of operation and adjusted as required)	Inspect inlets, outlets and overflows for blockages, and clear if required. If faults persist jetting and CCTV survey may be required.	Monthly and after large storms.
	Check penstocks and other mechanical devices (if present).	Half yearly.
	Inspect ventilation cowl (if present)	Monthly and after large storms.
Regular maintenance \ inspection	Inspect and identify any areas that are not operating correctly. If required, take remedial action.	Monthly for 3 months, then six monthly.
	Debris removal from catchment surface (where may cause risks to performance)	Monthly
	Where rainfall infiltrates into blocks from above, check surface of filter for blockage by silt, algae or other matter. Remove and replace surface infiltration medium as necessary.	Monthly (and after large storms)
	Remove sediment from pre-treatment structures	Annually (or as required after heavy rainfall events)
Remedial actions	Repair/rehabilitation of inlets, outlet, overflows and vents.	As required.

7.0 FLOW CONTROL UNITS

Location and Description

7.1 The location of the flow control unit is as indicated on the drainage drawing within Appendix A.

7.2 The manufacturer details and specification of the flow control unit is included in Appendix B.

7.3 The features will be designed in accordance with all relevant standards, plus Statutory Bodies' and manufacturers requirements.

7.4 It is the responsibility of the management company to maintain the flow control units.

Operation

7.5 The flow control units are intended for downstream flow control and to restrict the discharge rate of surface water leaving the site.

7.6 The flow control installed is a JFC Hydro-Valve Vortex Flow Control with a 1.0m design head and 5.0l/s design flow. This is a proprietary product; therefore, manufacturer's recommendations should also be taken into consideration.

Inspection and Maintenance Regime

7.7 Access for maintenance will be provided by locating within a manhole chamber.

7.8 The appropriate health and safety equipment must be used when accessing manholes. Confined space certificates must be held by any personnel entering a manhole and the appropriate permits should be obtained from water authority prior to any access.

Flow Control Schedule

Maintenance Schedule	Required Action	Frequency
Monitoring (to be undertaken more regularly within the first year of operation and adjusted as required).	Inspect inlets for blockages, and clear if required. If faults persist jetting and CCTV survey may be required.	Monthly and after large storms.
Regular maintenance / inspection	Inspect and identify any areas that are not operating correctly. If required, take remedial action.	Monthly for 3 months, then six monthly.
	Debris removal from catchment surface (where may cause risks to performance).	Monthly
	Remove sediment from pre-treatment structures and flow control chambers.	Annually (or as required after heavy rainfall events)
Remedial Actions	Repair/rehabilitation of inlets.	As required.

8.0 ATTENUATION TANK

Location and Description

8.1 There is 1No. cellular attenuation tank proposed for the development, the location of cellular attenuation tanks are as indicated on the drainage drawing as provided in Appendix A.

8.2 The manufacturers details and specification for the attenuation tank are included in Appendix C.

8.3 The feature will be designed in accordance with all relevant standards, plus statutory Bodies' and manufacturers requirements.

8.4 Attenuation tanks are proprietary products and therefore manufacturer's recommendations should also be taken into consideration. Additionally, different manufacturers may have different connection types and arrangements which will need to be taken into consideration.

8.5 It is the responsibility of the management company to maintain the cellular attenuation tanks.

Operation

8.6 The attenuation tank is designed to be the primary surface water storage feature to for the development site to support the restricted discharge.

8.7 The tank is designed to be in use during all storm events up to and including the 1 in 100 year plus climate change event.

8.8 A catchpit manhole is also positioned prior to and downstream of the tank inlets to reduce the risk of sedimentation entering the storage units during regular conveyance and during surcharging events.

8.9 Access for maintenance will be provided from the manhole chambers upstream and downstream of the tank.

Inspection And Maintenance Regime

8.10 Regular inspection and maintenance are important for the effective operation of the features. As the feature is an open accessible feature of the development, a regular inspection regime is very important to ensure the correct functionality of the surface water drainage network.

8.11 Sediment/material removal should be undertaken in consultation with the environmental regulator to confirm appropriate protocols.

8.12 The Cellular attenuation tank is a proprietary product and manufacturers guidance should be adhered too.

Cellular Attenuation Tank Schedule

Maintenance Schedule	Required Action	Frequency
Monitoring	Inspect/check all inlets, outlets, vents, and overflows to ensure that they are in good condition and operating as designed	Annually
	Survey inside of tank for sediment build-up and remove if necessary	Every 5 years or as required
Regular maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action	Monthly for 3 months, then annually
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly
	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter, remove and replace surface infiltration medium as necessary	Annually
	Remove sediment from pre-treatment structures and/ or internal forebays	Annually, or as required
Remedial Actions	Repair/rehabilitate inlets, outlet, overflows and vents	As required

9.0 APPENDIX A – DRAINAGE LAYOUT

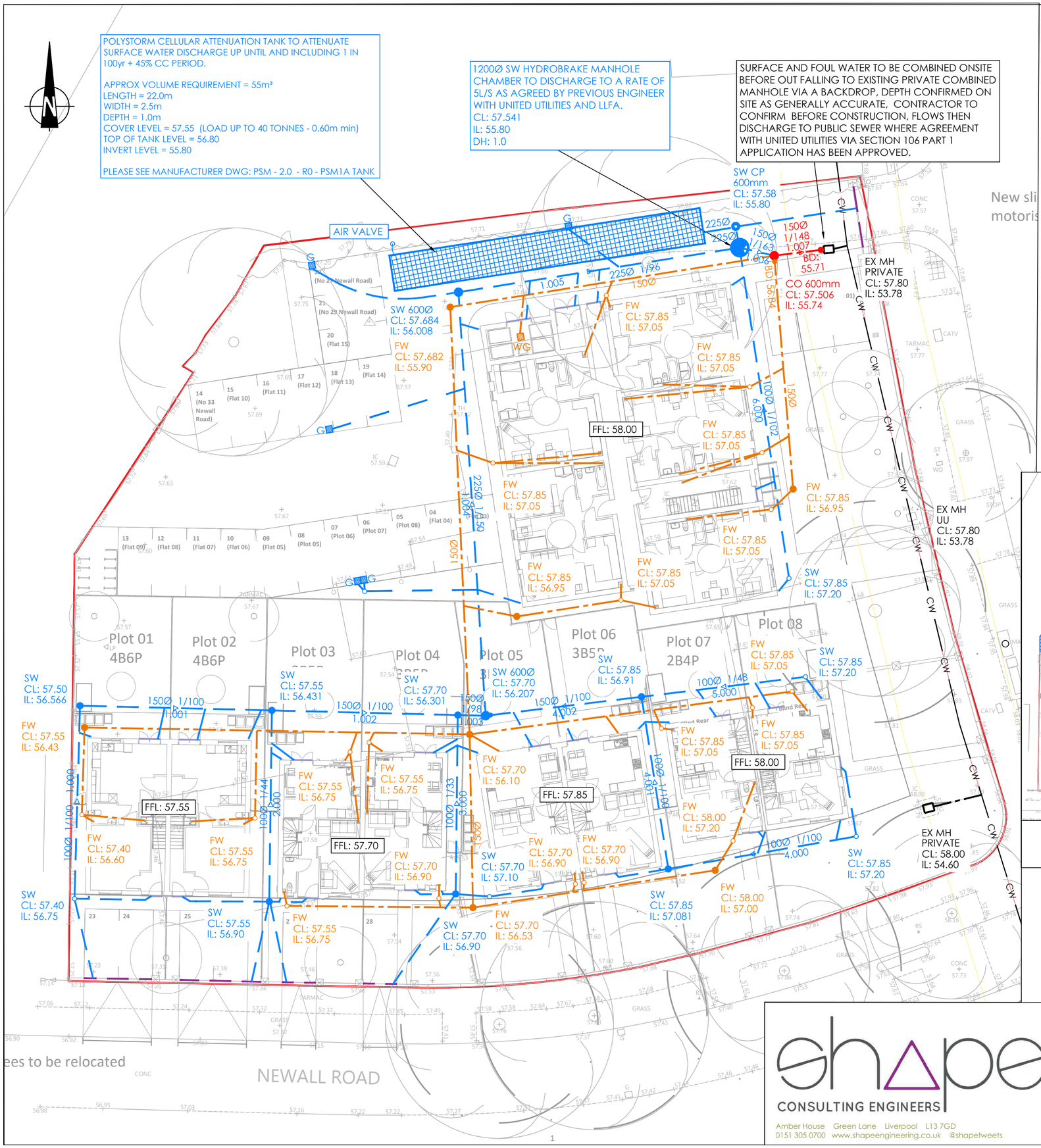
POLYSTORM CELLULAR ATTENUATION TANK TO ATTENUATE SURFACE WATER DISCHARGE UP UNTIL AND INCLUDING 1 IN 100yr + 45% CC PERIOD.

APPROX VOLUME REQUIREMENT = 55m³
LENGTH = 22.0m
WIDTH = 2.5m
DEPTH = 1.0m
COVER LEVEL = 57.55 (LOAD UP TO 40 TONNES - 0.60m min)
TOP OF TANK LEVEL = 56.80
INVERT LEVEL = 55.80

PLEASE SEE MANUFACTURER DWG: PSM - 2.0 - R0 - PSM1A TANK

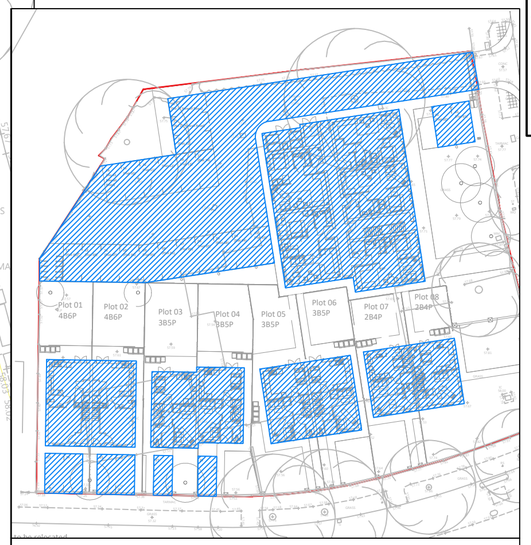
1200Ø SW HYDROBRAKE MANHOLE CHAMBER TO DISCHARGE TO A RATE OF 5L/S AS AGREED BY PREVIOUS ENGINEER WITH UNITED UTILITIES AND LLFA.
CL: 57.541
IL: 55.80
DH: 1.0

SURFACE AND FOUL WATER TO BE COMBINED ONSITE BEFORE OUT FALLING TO EXISTING PRIVATE COMBINED MANHOLE VIA A BACKDROP, DEPTH CONFIRMED ON SITE AS GENERALLY ACCURATE, CONTRACTOR TO CONFIRM BEFORE CONSTRUCTION, FLOWS THEN DISCHARGE TO PUBLIC SEWER WHERE AGREEMENT WITH UNITED UTILITIES VIA SECTION 106 PART 1 APPLICATION HAS BEEN APPROVED.



DRAINAGE KEY

- CW — EXISTING UU COMBINED WATER SEWER
- EXISTING PRIVATE COMBINED WATER SEWER
- PROPOSED PRIVATE FOUL WATER SEWER
- PROPOSED PRIVATE SURFACE WATER SEWER
- PROPOSED 300Ø PRIVATE FW PPIC
- PROPOSED 300Ø PRIVATE SW PPIC
- PROPOSED 450Ø PRIVATE FW PPIC UNLESS OTHERWISE STATED
- PROPOSED 450Ø PRIVATE SW PPIC UNLESS OTHERWISE STATED
- PROPOSED PRIVATE CATCH PIT
- PROPOSED SW HYDROBRAKE MANHOLE
- PROPOSED COMBINED SEWER
- PROPOSED SW GULLY
- PROPOSED SW ACO CHANNEL
- PROPOSED THRESHOLD SLOT CHANNEL
- FOUL WASH DOWN GULLY



PROPOSED IMPERMEABLE AREA 1 = 1540m²

GENERAL NOTES

THIS DRAWING TO READ IN CONJUNCTION WITH ALL RELEVANT STRUCTURAL AND ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.

ALL DIMENSIONS TO BE CHECKED ON SITE BY THE CONTRACTOR / FABRICATOR PRIOR TO COMMENCEMENT OF WORKS.

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS STATED OTHERWISE.

ALL WORKS TO BE CARRIED OUT IN STRICT ACCORDANCE WITH THE ENGINEER'S SPECIFICATIONS, RELEVANT BRITISH STANDARDS AND WHERE APPLICABLE LOCAL AUTHORITIES REQUIREMENTS.

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY SERVICE/UTILITY APPARATUS IN THE VICINITY OF THE WORKS BEFORE DRAINAGE INSTALLATION HAS COMMENCED. SHAPE WILL ACCEPT NO CLAIMS WHATSOEVER IN RESPECT OF ANY LOSSES OR DAMAGE CAUSED IN RESPECT OF SUCH APPARATUS.

NOTE:

FOR INFORMATION PURPOSES THE FOLLOWING SHOULD BE ASSUMED:

ALL INDICATED SURFACE WATER SEWERS ARE 100mm IN DIAMETER AND HAVE A MINIMUM GRADIENT OF 1:100 UNLESS OTHERWISE STATED.

ALL FOUL WATER SEWERS ARE 100mm IN DIAMETER AND HAVE A MINIMUM GRADIENT OF 1:80 UNLESS OTHERWISE STATED.

ALL FOUL AND SURFACE WATER PPIC'S TO BE 450 IN DIAMETER UNLESS OTHERWISE STATED.

ALL MANHOLE AND SEWER TRENCH DEPTH SHOULD PROVIDE A MINIMUM COVER OF 1.2m TO THE SOFFIT OF THE PIPE. FURTHER LEVEL INFORMATION WILL BE CALCULATED IN DUE COURSE.

Rev	By	Description	Date
AB1	NM	As Built	03.11.2025

Revision Schedule

shape CONSULTING ENGINEERS
Amber House Green Lane Liverpool L13 7GD
0151 305 0700 www.shapeengineering.co.uk @shapetweets

PROJECT: Greenbrow Social Club, Manchester City Council

TITLE: Drainage Strategy Plan

Client	Casey Group
Status	-
Purpose of Issue	As Built
Designed By	GP
Approved By	EC
Drawn By	NM
Project No.	2639
Sheet No.	C-0001
Scale (@ A2)	1:200
DATE	01.02.2024
Rev	AB1

10.0 APPENDIX B – HYDROBRAKE DETAILS

Registered Office
JFC PLASTICS LTD.
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Banbury Road, Goldicote
Stratford-Upon-Avon, CV37 7NB,
United Kingdom

Sales Office
JFC CIVILS U.K.
Units 320/321,
25 Goodlass Road, Speke,
Liverpool, L24 9HJ,
United Kingdom
T. + 44 1928 583 391
E. infouk@jfccivils.com

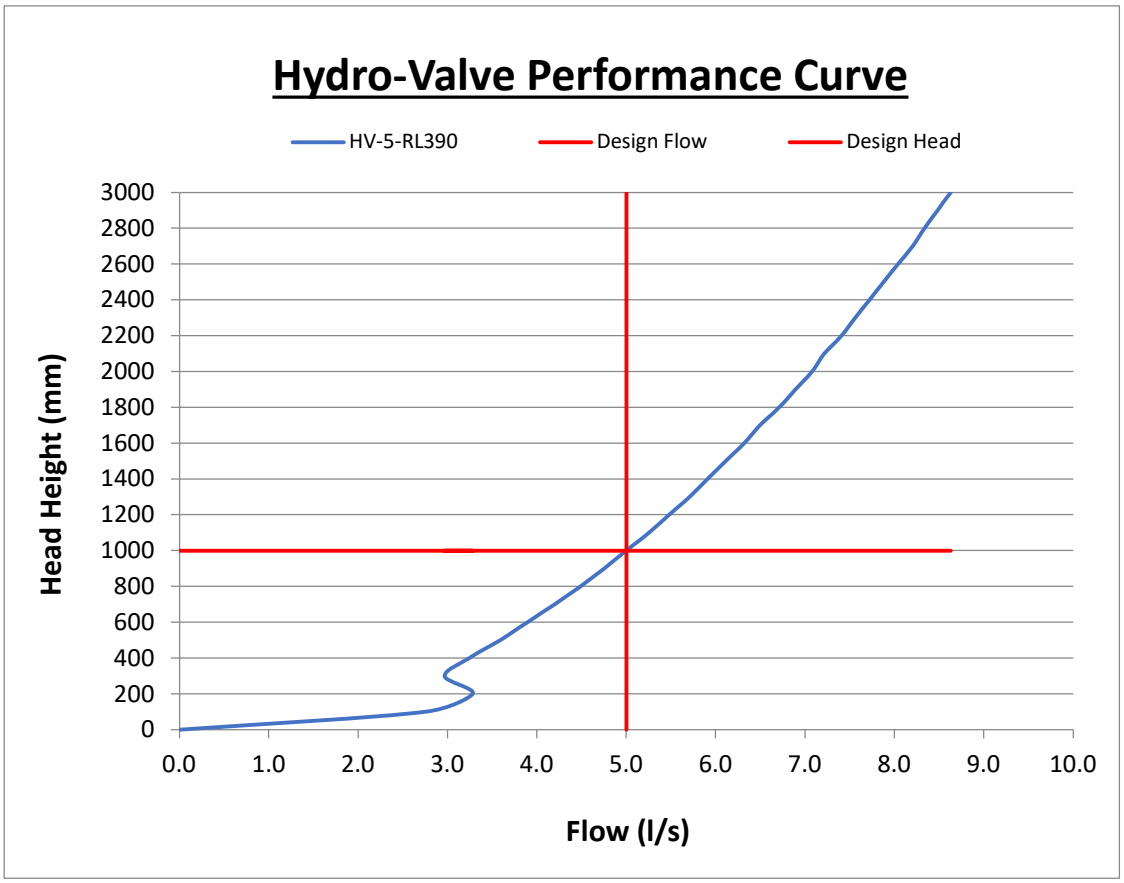
Hydro-Valve

Vortex Flow Control

JFC Reference:	HVS-U2799
Date:	23/04/2025
Client	JDP
Address:	
Site Address:	Greenbrow Social Club, Wythenshaw, Manchester
Application:	Stormwater Attenuation Control

Specification

Code:	HV-5-RL390
Design Flow:	5 l/s
Design Head:	1000 mm
Outlet Pipe Size:	110 mm (assumed)
Outlet Manhole Type:	Ø1.05m (assumed)
Minimum Sump:	300 mm
Manual By-Pass:	Yes
Overall Dimensions:	HV-II-SMBF or HV-II-SMBC



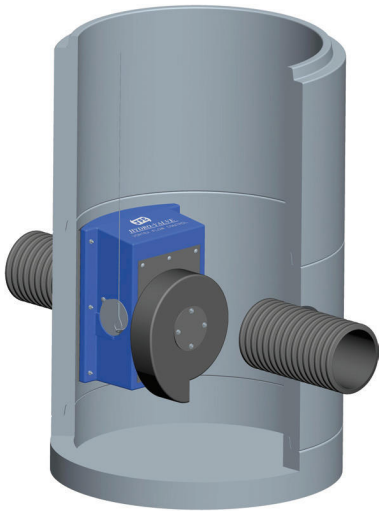
Head (mm)	Flow (l/s)
0	0.00
100	2.75
200	3.28
300	2.97
400	3.24
500	3.59
600	3.89
700	4.19
800	4.48
900	4.75
1000	5.00
1100	5.25
1200	5.48
1300	5.71
1400	5.91
1500	6.11
1600	6.32
1700	6.49
1800	6.71
1900	6.89
2000	7.08
2100	7.22
2200	7.41
2300	7.56
2400	7.72
2500	7.88
2600	8.04
2700	8.20
2800	8.34
2900	8.48
3000	8.63

This unique Hydro-Valve head/discharge curve has been derived from systematic modelling along with laboratory testing and only applies to the unit specified above. Head / discharge curve based on bottom to top modelling.

CIVILS

Quality Plastic Solutions

Hydro-Valve
Vortex Flow Control



The Hydro-Valve is a vortex flow control device for throttling the flow rate of stormwater before discharge into a local storm drain or water course.

It is available in a range of sizes with varying flow rates to suit all specifications. It is used to control the flow of stormwater from the attenuation tank preventing downstream flooding during periods of heavy rainfall. Manufactured to customer specifications in order to achieve a specified flow rate at a given head height. This self-activating device uses vortex principles to control and attenuate stormwater flow without the need for moving parts or the need for an external power source.

Features & Benefits

- Typical flow rate: 1-50L/s*
- Typical head heights: 450-3000mm*
- WRC certification
- Fast installation
- Customised specification
- 3-6 times greater oriface CSA
- Hydraulic data available

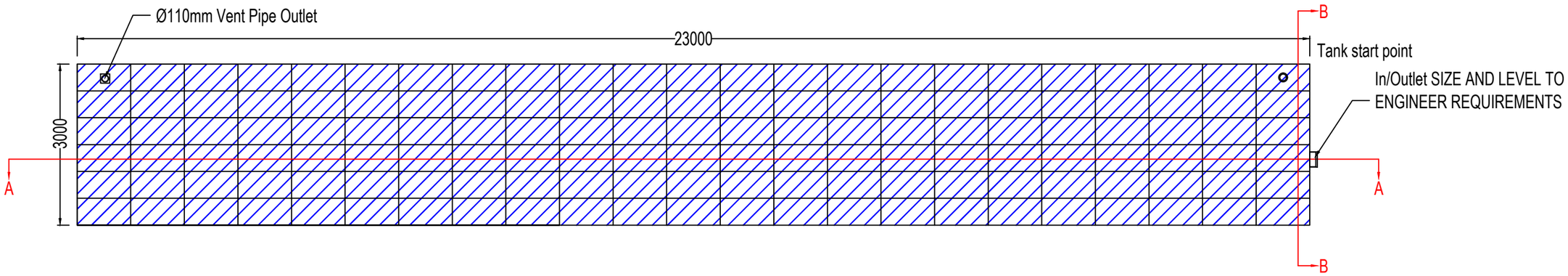
* Contact JFC for further information.

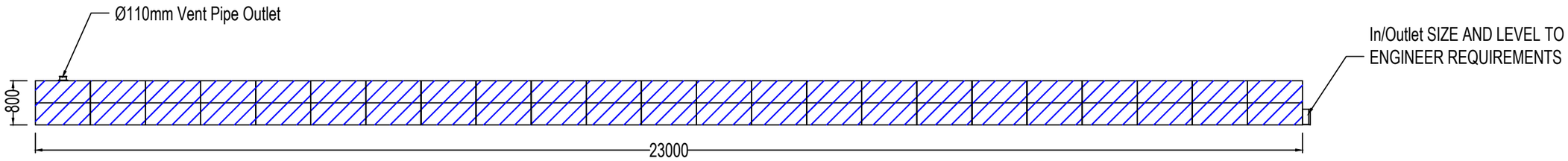
Hydro-Valve Fits The Following Manhole Types

CURVED BACK	FLAT BACK**
1200mm	1050mm
1350mm	1800mm
1500mm	2100mm
	2400mm

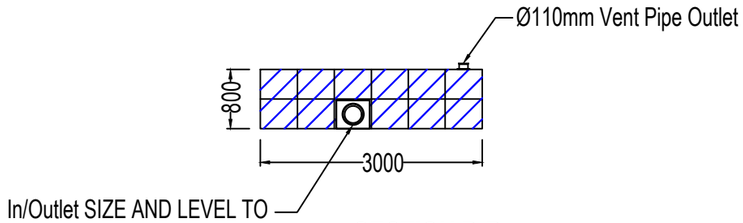
** Requires a concrete shuttered wall on the inside of the manhole chamber.

11.0 APPENDIX C – CELLULAR TANK MANAUFACTURER PRODUCT DATA





SECTION A-A

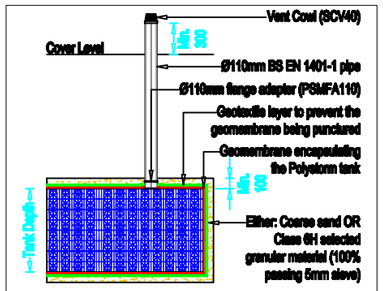


SECTION B-B

PROPOSED POLYSTORM STRUCTURE	
Plan Area	69m²
Tank Volume	55.2m³
Storage	52.44m³
Polystorm-R Units	276 (PSM1A)

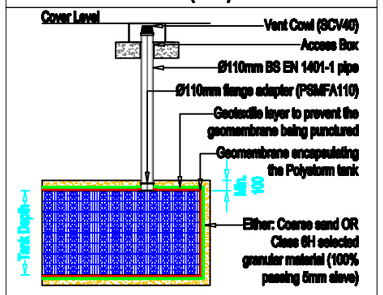
LEGEND:

Polystorm-R Units (PSM1A)



ATTENUATION TANK VENTILATION SYSTEM - OPTION 1)

Scale (N.T.S)



ATTENUATION TANK VENTILATION SYSTEM - OPTION 2)

Scale (N.T.S)

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NOTES:-

1. THIS DRAWING DETAILS A PROPOSED POLYPIPE CIVILS & GREEN URBANISATION PRODUCT GENERAL ARRANGEMENT, REFLECTING OUR INTERPRETATION OF THE INFORMATION PROVIDED.
2. POLYPIPE CIVILS & GREEN URBANISATION (PCGU) PRODUCTS SHALL BE INSTALLED IN ACCORDANCE WITH PCGU RECOMMENDATIONS (REFER TO PCGU TECHNICAL GUIDANCE FOR FURTHER INFORMATION); GIVING DUE CONSIDERATION TO THE SITE SPECIFIC CONDITIONS AND REQUIREMENTS, IN ADDITION TO ANY REQUIREMENTS MANDATED BY THE RELEVANT APPROVING AND ULTIMATE OWNER ORGANISATION(S).
3. DO NOT SCALE, REFER TO FIGURED DIMENSIONS ONLY.
4. ALL DIMENSIONS IN MILLIMETRES, UNLESS OTHERWISE STATED.
5. ALL DIMENSIONS ARE NOMINAL AND MAY VARY WITHIN MANUFACTURING OR CONSTRUCTION TOLERANCES.
6. ALL SITE TEMPORARY AND ENABLING WORKS BY OTHERS.
7. THIS DRAWING IS INTENDED FOR GUIDANCE ONLY. CONFIRMATION OF THE INFORMATION CONTAINED WITHIN THIS DOCUMENT SHOULD BE SOUGHT FROM THE OVERSEEING ENGINEER(S) PRIOR TO THE COMMENCEMENT OF FINAL DESIGN OR CONSTRUCTION ACTIVITIES.
8. PLEASE REFER TO POLYSTORM & POLYSTORM LITE TECHNICAL GUIDE.
9. THE PROPOSED POLYSTORM STRUCTURE GENERAL ARRANGEMENT HAS BEEN BASED ON PROVIDING THE MINIMUM STRUCTURE VOLUME STATED ONLY. NO STRUCTURAL OR HYDRAULIC ASSESSMENT HAS BEEN MADE OF THE SUITABILITY OF THE PROPOSED POLYSTORM STRUCTURE FOR THE PROPOSED APPLICATION, NOR HOW IT WILL INTEGRATES INTO THE PROPOSED DRAINAGE SYSTEM.
10. ALL MEMBRANE JOINTS TO BE WELDED RATHER THAN GLUED.

Notes :- All the Dimensions by Millimeters

KEYTEC INSTALLATION SERVICES

KEYTEC INSTALLATION SERVICES
ROYLE HOUSE
COWM TOP BUSINESS PARK
COWM TOP LANE
OL11 2PU
TEL: 01706 374416
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PROJECT

FORMER GREENBROW SOCIAL CLUB, WYTHENSHAWE

ATTENUATION TANK

TITLE

DETAILS OF ATTENUATION/STORAGE TANK DETAILS USING POLYSTORM PSM1A UNIT.

Scale	Drawn	Checked
N.T.S.	AY	KD
Date	**Drawing No.**	**Revision**
30/08/2024	PSM - 2.0	R0