

Our reference: 077873-CUR-00-XX-RP-GE-042-P01

29th November 2022

Mr Aidan Jackson
The Guinness Partnership
Bower House
1 Stable Street
Hollinwood
Oldham
OL9 7LH

Dear Aidan,

Re: Phase 2 Geo-Environmental and Geotechnical Report for Dam Head Drive, Project 500 Sites

1.0 Introduction

Curtins Consulting were instructed by The Guinness Partnership to undertake a ground investigation of Dam Head Drive, Manchester, M9 6DH. The site is currently an L shaped vacant, and one of six sites currently being investigated on behalf of The Guinness Partnership, as part of Project 500, Manchester City Councils program to deliver affordable homes in the northeast of the city.

Dam Head Drive and Lindsides Walk form the sites western boundary, to the north is woodland beyond which are the gardens associated with residential properties located on Cullen Grove. Immediately east and south are undeveloped areas of public open space with some sparse tree coverage.

The ground investigation was undertaken to determine the ground conditions pertaining to the development site, and to aid contamination risk assessment and foundation design. It is understood that the site is to be developed for residential dwellings. At the time of writing, no proposed site plans were available for review.

2.0 Background Information

Residential development is proposed at a plot of land located east of Dam Head Drive. The site location plan is presented in *Figure 1*, overleaf.

Earliest mapping (1848) shows that the site was undeveloped and surrounded by farmland. Dam Head Farm is 48m northwest of the site (demolished by 1975) and Bailey's Wood is 10m east, extending towards the south. A small watercourse runs through the woods, flowing towards the west-southwest. The Booth Hall Infirmary/Hospital was constructed in 1909 and extended over the years.

From 1932 the surrounding area becomes populated with residential developments to the north of site, expanding across the north and northeast by 1956, and in all directions by 1975. Several ponds are noted 250m of the site, all of which appear to be infilled by the 1980s. A spring is recorded from 1893-1932 on the western boundary of the site. The site remains unchanged until the 1980s when residential developments are constructed across the site. These are later demolished in phases, first the centre by 2000, then to the north by 2006 and finally in the south by 2016.



Figure 1 – Site Location Plan (site boundary shown in pink and 250m buffer in purple).
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BGS mapping indicates the site is underlain by Glacial Till Deposits (interbedded granular and cohesive soils), Manchester Marls Formation bedrock (majority of site) and Chester Formation bedrock (southwest of site). The BGS borehole scans recorded Made Ground overlying alternating layers of sand and clay.

The site is located within a Coal Mining reporting area. A review of available online data indicates the site is not located within the vicinity of recorded mine entries, or a development high risk area. The risk from shallow coal mining is therefore considered to be **Low**.

The site is underlain by a Secondary B Aquifer associated with the bedrock formations. The superficial deposits are designated as a Secondary Undifferentiated Aquifer. The site is not located within a Source Protection Zone (SPZ). There are no potable groundwater abstraction points within 1km. The nearest surface water feature is an issue 94m southeast of the site. There are no surface water abstractions within 1km. There are no discharge consents in the last 20 years within 500m of the site.

The risk from UXO associated with WWII bombing to future intrusive investigation works is considered to be **Low**.

The risk from Made Ground posed to the site's end-users and groundwater is considered to be **Moderate / Low** due to the anticipated lack of contamination in the Made Ground. The risk posed to controlled water is considered to be **Very Low** due to anticipated low permeability superficial soils and distance to the nearest surface water feature. The risk posed to site end-users from ground generating gases arising from organic / putrescible soils near the site is considered to be **Moderate**.



3.0 Curtins Ground Investigation (2022)

Ground investigation works were undertaken on the 10th August 2022.

The scope of the ground investigation was designed in general accordance with current UK guidance including:

- Land Contamination Risk Management (Ref. 1);
- British Standard (BS) 10175:2011 + A2:2017 Investigation of Potentially Contaminated Sites (Ref. 2);
- BS 5930:2015 + A1:2020 Code of Practice for Ground Investigations (Ref. 3); and
- BS EN1997: Part 2:2007 Eurocode 7 (Ref. 4).

3.1 Scope of Works

The site works consisted of six window sample boreholes, WS01, WS02, WS02A, WS02B, WS03 and WS04, advanced to between 0.35 and 3.45m bgl. In-situ testing, including Standard Penetration Tests (SPTs) were undertaken at, generally, 1m intervals within the superficial soils.

An exploratory hole plan is included at the end of this report, Ref. 077873-CUR-00-XX-DR-GE-80001-P02.

4 No. of soil samples, all from the Made Ground were taken and underwent environmental chemical testing, as detailed in Table 3.1. Analysis was undertaken to assess the contamination risk presented to site end users and construction works in this area of the site.

Table 3.1 - Environmental testing suite (Soils)

Suite Ref.	Analyte	LOD
Soils Suite A	Asbestos Screen	-
	pH Value	0.01 pH Units
	2:1 Water Soluble Sulphate	0.01 g/l
	Soil Organic Matter (SOM)	0.1 % ^{w/w}
	Arsenic, Chromium, Chromium VI, Copper, Lead, Selenium, Nickel	1 mg/kg
	Zinc	5 mg/kg
	Boron (water soluble)	1 mg/kg
	Cadmium	0.5 mg/kg
	Mercury	0.17 mg/kg
	Cyanide (total)	1 mg/kg
	Phenols (screen)	0.2 mg/kg
	PAHs (USEPA 16)	0.01 to 0.08 mg/kg
	TPH (Aro/Alk Split)	0.01 to 1.0 mg/kg

The following laboratory geotechnical testing was undertaken in accordance with methods outlines in BS1377: Part 1:2016 (Ref. 5), to enable a foundation assessment for the proposed residential development:

- 2 No. 4pt Atterberg Tests
- 1 No. Particle Size Distribution Tests



- 3 No. 2:1 Water Soluble Sulphate & pH

3.2 Ground Conditions

A summary of the ground conditions encountered during the site investigation works is presented in *Table 3.2*, below, with detailed information presented in the exploratory hole logs included at the end of this report. A summary of each strata is presented below.

Table 3.2 - Summary of encountered ground conditions:

Stratum	Depth to Top of Strata m bgl (m AOD)	Thickness (m)		Description
		Min	Max	
MADE GROUND	0.00	0.35	2.60	MADE GROUND: Grass over loose dark brown slightly clayey gravelly fine to medium sand. Gravel is angular to subrounded, fine to coarse of sandstone, brick, plastic and concrete. Overlying MADE GROUND: Soft to firm brown gravelly very sandy clay. Gravel is angular to rounded fine to coarse of brick, concrete, sandstone and coal. Or MADE GROUND: Grey sandy gravel of fine to coarse subangular of brick, concrete and sandstone. Or MADE GROUND: Concrete
GLACIAL TILL (Cohesive)	1.40 – 2.60 (94.54 – 90.50)	0.80	0.85 ¹	Firm to stiff slightly sandy slightly gravelly CLAY. Gravel is fine to coarse subangular to subrounded of sandstone and coal.
GLACIAL TILL (Granular)	2.20 (93.74)	1.15 ¹		Brown gravelly SAND. Sand is fine to medium. Gravel is fine to coarse subangular to subrounded of sandstone, coal and mudstone.

¹ - Base of strata not encountered.

3.3 Groundwater

Groundwater strikes were recorded in two of the six boreholes during the site investigation. A summary is detailed below within *Table 3.3a*.

Table 3.3a – Water Strikes Noted During Site Investigation

Borehole Ref.	Water Strikes		
	Strike Depth m bgl (m AOD)	Strata	Notes
WS01	1.35 (78.48)	Made Ground	N/A
	2.30 (77.53)	Glacial Till (granular)	N/A
WS04	0.90 (80.19)	Made Ground	N/A

Groundwater, likely representing perched water within Made Ground and granular layers in the Glacial Till, was recorded during gas monitoring visits. The borehole response zones, and recorded water levels are detailed in *Table 3.3b*.



Table 3.3b – Monitoring Well Response Zones

Borehole Ref.	Installation Strata	Response Zone m bgl (m AOD)	Depth to water level m bgl (m AOD)					
			24/08/22	01/09/22	21/09/22	28/09/22	14/10/22	28/10/22
WS01	Made Ground and Glacial Till	1.00 – 2.50 (94.94 – 93.44)	1.25 (94.69)	1.40 (94.54)	1.24 (94.70)	1.23 (94.71)	1.19 (94.75)	1.19 (94.75)
WS03	Made Ground	1.00 – 1.70 (92.71 – 92.01)	Dry	Dry	Dry	Dry	Dry	Dry

3.4 Obstructions

Obstructions were noted at WS02, WS02A, WS02B and WS03 during the site investigation. All terminations were due to the presence of buried concrete obstructions at varying depths: WS02 and WS02A terminated at 0.35m bgl, WS02B terminated at 0.5m bgl and WS03 terminated at 1.75m bgl. Based on the historical mapping it is likely that the concrete is relic foundations from the previous residential dwellings occupying the site which may not have been grubbed out when the buildings were demolished in the late 1990s.

3.5 Observed Potential Contamination

No evidence of visual or olfactory contamination was observed during the ground investigation works.

4.0 Ground Contamination Assessment

Four soil samples from the Made Ground were taken and analysed according to the testing Suite A outlined in Table 3.1. The laboratory chosen by Curtins to undertake the analysis was Envirolab, a UKAS and MCERTS accredited laboratory.

Soil samples were obtained to provide adequate and representative coverage for the proposed end use, accounting for the site's history.

Considering the sites proposed end-use for residential dwellings were compared against adopted 'Residential with homegrown' Generic Assessment Criteria (GACs).

Soil Organic Matter (SOM) content has an influence on the volatilisation of organic contaminants (i.e. the higher the SOM, the lower the volatilisation rate and therefore inhalation exposure risk). Soil organic matter (SOM) value for the Made Ground soils was 0.64% w/w to 3.42% w/w . With consideration for a calculated geometric average, Made Ground soil features determine an SOM of 2.34% w/w . As such, a comparison against Tier 1 thresholds for SOM of 1% w/w for Made Ground soils for a 'Residential with homegrown' end use has been adopted.

The full chemical testing results and adopted GAC's are listed in at the end of this report.

Exceedances were noted within the soil samples for Metals and PAHs. Exceedances are detailed below within Table 4.0.



Table 4.0 – Exceedances within Made Ground

Exploratory Hole I.D.	Depth m bgl (m AOD)	Strata	Determinant	Concentration (mg/kg)	GAC (mg/kg)
WS02	0.35 (79.86)	Made Ground	Mercury	1.36	1
WS03	0.60 (79.85)	Made Ground	Mercury	1.41	1
			Benzo(a)pyrene	7.05	2.2
			Benzo(b) fluoranthene	6.73	2,6
			Dibenzo(ah) anthracene	1.28	0.24
WS04	0.60 (80.49)	Made Ground	Mercury	1.48	1
			Benzo(a)pyrene	5.9	2.2
			Benzo(b) fluoranthene	5.51	2,6
			Dibenzo(ah) anthracene	1.14	0.24

No suspected asbestos containing materials (AMCs) were identified in the exploratory hole logs and no asbestos fibres were positively identified during laboratory screening.

5.0 Geotechnical Laboratory Analysis

A programme of geotechnical testing was scheduled by Curtins to determine the ground model for the site, based on the ground conditions, detailed in Section 3.1.

5.1 Geotechnical Testing and Engineering Properties

The program of geotechnical testing undertaken during this investigation is detailed in Section 3.1.

Where necessary, determination of characteristic parameters has been based on appropriate assessment of results derived from in-situ tests, laboratory analysis and published correlations, complemented with engineering judgement and consideration of the relevant limit state. The parameters are not considered to be absolute and should be referenced with the specific strata text in this section and reviewed when considering a specific area of the site.

5.1.1 Made Ground

A surface cover of grass was underlain by loose dark brown slightly clayey gravelly fine to medium grass cover over slightly gravelly fine to medium sand. Gravel is angular to rounded fine to coarse of concrete, brick, sandstone and timber.

The underlying Made Ground soils comprise either soft to firm brown gravelly very sandy clay, or grey sandy gravel. of fine to coarse subangular of brick, concrete and sandstone.

The gravel fraction of the granular soils includes fine to coarse subangular of brick, concrete and sandstone.



The geotechnical test results recorded within the Made Ground are summarised in *Table 5.1.2*.

Table 5.1.2 – Summary of Made Ground geotechnical test results

Parameter	No. of tests	Minimum	Maximum	Average
pH	4	7.95	9.52	8.55
Water Soluble Sulphate	4	40	130	87
SPT 'N' Value	5	8	50	-

Due to inherent variability in the Made Ground soils, and its compressible nature, it is not considered to be a suitable bearing stratum for the proposed residential development.

5.1.2 Glacial Till

Glacial Till was recorded underlying the Made Ground in WS01 and WS04.

The Glacial Till encountered comprises predominantly a cohesive deposit described typically as a firm to stiff sandy clay and brown gravelly sand.

The geotechnical test results recorded within the superficial deposits are summarised in *Table 5.1.2*.

Table 5.1.2 – Summary of Glacial Till geotechnical test results

Parameter		No. of tests	Minimum	Maximum	Average
pH		3	8.3	8.5	8.4
Water Soluble Sulphate (mg/l)		3	15	40	27
SPT 'N' Value		4	9	50	-
Moisture Content (%)		2	18	23	20.5
Atterberg Test	Liquid Limit (%)	2	32	35	33.5
	Plastic Limit (%)	2	16	18	17
	Plasticity Index (%)	2	16	17	16.5
	Modified Plasticity Index (%)	2	12.64	17	14.82
Particle Size Distribution		1	Very Coarse - 0 Gravel - 68 Sand - 15 Fines (<0.063mm) - 17		

Cohesive Soils

A total of two SPT 'N' values were recorded in cohesive Glacial Till recording N values of 9 and 21, where full penetration was met.

Two Atterberg tests were undertaken during the ground investigation, indicating the soils behave as low plasticity clays with a low volume change potential. The volume change potential of the cohesive soils has been determined in accordance with guidance from NHBC Standards Chapter 4.2 (Ref. 6).

The results of SPT can be correlated to an Undrained Shear Strength (c_u) when undertaken within cohesive deposits. For overconsolidated clays, Stroud (1974) reported good correlations between N and c_u , where:

$$c_u = f1 \times \text{SPT 'N' Value}$$



where values of f_1 are the conversion factor based on the plasticity of the clay.

Based on an average plasticity index of 16% and utilising an f_1 factor of 6 undrained shear strengths of 54kPa and 126kPa (medium to high strength) are derived.

Moderately conservative characteristic unit weights of 17-20kN/m³ for a medium strength clay above and below the water table, are deemed appropriate in accordance with BS8002 (Ref. 7).

The results of SPT can be correlated to the modulus of volume compressibility (m_v) when undertaken within cohesive deposits. For overconsolidated clays, Stroud (1974) reported correlations between N and m_v , where:

$$m_v = \frac{1}{f_2 \times SPT \ N}$$

where values of f_2 are a factor of the plasticity of the clay.

Based on an average plasticity index of 16% and utilising an f_2 factor of 0.62, m_v values of 0.077 and 0.128 m²/MN (low to medium compressibility) are derived.

Granular Soils

Granular Glacial Till was recorded within one exploratory hole (WS01) at a depth of 2.20m bgl to the base of the borehole at 3.45m. The thickness was not determined. One SPT at 3.0m refused with an N value >50 suggesting dense to very dense sand.

6.0 Gas Risk Assessment

The conceptual site model concluded there was a Moderate risk from ground gas. There was potential for Made Ground, containing organic /putrescible content as a potential source of ground gas generation. Noxious and asphyxiate gases were attributed to include, but not limited to, carbon dioxide, carbon monoxide, methane and hydrogen sulphide.

Two boreholes were installed with monitoring wells as detailed in *Table 3.3b*.

Six rounds of gas monitoring were undertaken between 24th August 2022 and 28th October 2022. The results log sheets are contained within Appendix A6 and summarised in *Table 6.0*.

Barometric pressures ranging between 995mb and 1012mb were recorded during the monitoring period.

Table 6.0 – Summary of Ground Gas Monitoring Results

Exploratory Hole ID	Maximum Methane Concentration (%)	Maximum Carbon Dioxide Concentration (%)	Minimum Steady Oxygen Concentration (%)	Maximum Peak Gas Flow (l/hr)	Steady State Gas Flow (l/hr)
WS01	<0.1	2.2	16.2	<0.1	<0.1
WS03	<0.1	2.6	15.1	2.0	<0.1

Methane (CH₄) was recorded below the instrument's detection limit within all the monitoring wells.

Carbon Dioxide (CO₂) recorded a maximum concentration of 2.6% %v within WS03 during the fifth visit.

All monitoring visits for all wells recorded hydrogen sulphide (H₂S) and Carbon monoxide (CO) below the limit of detection of the monitoring instrument.



The maximum peak air flow rate was recorded in WS03, at 2.0 l/hr on the fourth visit, reducing to a steady state air flow rate of 0.0 l/hr. For the purposes of this assessment, the negative values are taken as absolute values, and it is assumed that a positive air flow rate of the same value may be achieved in natural ground conditions.

Two gas risk assessments have been undertaken, to account for the end use of the site.

Private residential dwellings, assumed to be built in accordance with the 'model' low-rise housing development and NHBC guidelines (Ref. 8), will be assessed in accordance with the NHBC Traffic Light Classification.

Hazardous gas flow rates (Q_{hg}) can be evaluated using peak air flow rates and maximum methane concentrations, and steady state air flow rates and steady state carbon dioxide concentrations. On the basis that methane concentrations were recorded below the limit of detection of the monitoring instrument throughout the monitoring period, the gas screening value (GSV) will be determined by the carbon dioxide concentrations recorded.

For carbon dioxide, the greatest steady state air flow rate was recorded as <0.1 l/hr in all locations on all visits. The greatest steady state carbon dioxide concentration was recorded in WS03 on the fifth visit (2.6%v/v). On this basis, the calculated Q_{hg} for the site is taken as 0.003 l/hr.

The assessment indicates the site is classified as Green, where designs confirm with the NHBC 'model' low rise house.

Developments which differ significantly from the 'model' construction will be assessed in accordance with BS8485:2019. This includes low rise apartments.

The dataset obtained is considered to be representative of site conditions beneath the proposed development footprint. A range of data has been collated which captures significant temporal variation including barometric variability and various ground saturation states. As such, the gas risk assessment will utilise steady state flow rates and maximum gas concentrations when determining the maximum Q_{hg} .

On the basis that methane concentrations were recorded below the limit of detection of the monitoring instrument throughout the monitoring period, the gas screening value (GSV) will be determined by the carbon dioxide concentrations recorded.

Evaluation of the 'borehole' scenario (highest hazardous gas flow rate that is realistically possible, i.e. in a single borehole) records a maximum Q_{hg} of 0.003 l/hr for carbon dioxide in WS03 on the fifth visit, where maximum steady state flow reading of <0.1 l/hr is used, and maximum gas concentrations of 2.6% v/v recorded.

Evaluation of the 'worst case' check (highest hazardous gas flow rate irrespective of when the readings were taken within a single strata/zone), records a maximum Q_{hg} of 0.003 l/hr for carbon dioxide.

The 'borehole' scenario is considered appropriate to determine the characteristic gas regime for the site, which is consistent with a Characteristic Situation 1 (CS-1) gas regime, where gas protection measures will not be required.

The potential risks from ground gases to the excavations are to be dealt with by the Contractor, in accordance with the Confined Space Regulations, during construction. Appropriate ground gas monitoring and safety measures should be in place during the works, as the potential presence of ground gases could pose a risk in any confined spaces of deep excavations.



7.0 Geotechnical Assessment and Recommendations

The recommendations provided within this section are based on a review of the encountered ground conditions across the site.

The ground conditions at site generally comprised Made Ground to a maximum depth of 2.60m bgl where proven, underlain by Glacial Till. The Glacial Till was encountered at depths of 1.40m (WS01) and 2.60m (WS04). The thickness of the Glacial Till was not proven by this investigation.

7.2 Foundation Assessment

It is understood the proposed development comprises several two storey residential houses and residential apartments. The loadings of the proposed houses and apartments were not available at the time of writing this report. The loads will pre-determine the suitability of foundations adopted, though are not anticipated to be high.

There is uncertainty as to whether shallow foundations will be feasible for all of the plots as Glacial Till was only encountered in two locations (WS01 and WS04) at depths of 1.40m and 2.60m bgl. It is recommended that a supplementary ground investigation is undertaken once a site turnover has been completed to grub out the buried concrete obstructions and determine the depth to a suitable founding stratum in other parts of the site.

Based on the depth to the Glacial Till in WS01 and WS04 only, shallow strip or trench fill foundations are likely to be feasible in these locations. Foundations will be required to be taken down below any Made Ground and be placed in the firm to stiff cohesive Glacial Till that exhibits an undrained shear strength in excess of 50kPa and cast at a minimum depth of 1.5m bgl. Localised over excavation may be required where there is deep Made Ground and to ensure the minimum undrained shear strength criteria of 50kPa is achieved. To prevent differential settlement foundations should be placed wholly in one material i.e. clay and over excavation through granular sand layers in the Glacial Till may be required to achieve this.

Assuming a 0.6m wide strip foundation at a depth of 1.5m bgl, an allowable bearing capacity of 80kPa is likely to be achievable (limited to less than 25mm settlement). Any changes to the foundation width, depth or founding strata will change this capacity. Should excavation depths become excessive and uneconomical to meet the minimum undrained shear strength of 50kPa, a ground improvement (e.g. vibro) or mini-pile solution may be required.

Specialist contractors should be consulted with regards to the design of an appropriate ground improvement or piling scheme to identify the most feasible and economic technique and options.

A qualified experienced engineer should inspect the founding stratum prior to casting.

Groundwater ingress into shallow excavations is not anticipated, however if it does occur sump pumping is likely to be an effective dewatering technique.

Excavation instability associated with the Made Ground and superficial deposits cannot be discounted, especially in granular layers in the Glacial Till and in conjunction with groundwater ingress.

7.3 Ground Floor Slab

In accordance with the NHBC Standards (2022) ground bearing floor slabs may only be used where the depth of Made Ground is less than 600mm deep and properly compacted. Any unsuitable or soft soils should be excavated out and replaced with a granular fill, and the formation thoroughly proof rolled prior to casting. Where Made Ground is greater than 600mm thick, excavating and replacing the Made Ground to an engineering specification may enable a ground bearing solution, however approval from the warranty provider should be



obtained in advance. Alternatively ground improvement such as vibro would enable a ground bearing solution. A suspended floor slab would be required in conjunction with a piled solution.

7.3 Chemical Attack on Buried Concrete

The classification of the site in terms of concrete in aggressive ground is based on the guidance provided in Table C2 of the Building Research Establishment (BRE) Special Digest 1, 3rd edition of 2005. A summary of the results obtained during are summarised in *Table 7.3a*, and *Table 7.3b* details the classification.

Table 7.3a – Aggressive Chemical Environment for Concrete (ACEC) Site Classification

Stratum	Test Type	No. of Tests	Range
Made Ground	pH	4	7.95 – 9.52
	Water Soluble Sulphate (mg/l)	4	40 – 130
Glacial Till	pH	3	8.3 – 8.5
	Water Soluble Sulphate (mg/l)	3	15 – 40

Table 7.3b – Aggressive Chemical Environment for Concrete (ACEC) Site Classification

Stratum	Design Sulphate Class	ACEC Class ⁽¹⁾
Made Ground	DS-1	AC-1
Glacial Till	DS-1	AC-1

(1) ACEC assessment was based on the *mobile* groundwater condition in a *greenfield* scheme area

In these conditions it is considered that deterioration of buried concrete due to sulphate or acid attack is unlikely to occur. The preliminary design of buried concrete according to Table C2 of BRE Special Digest 1:2005 should be in accordance with Class DS-1 (AC-1) conditions assuming mobile groundwater.

8.0 Closure

A **Moderate/Low** risk has been identified to site end-users from soil and dust inhalation, direct contact and ingestion of shallow site soils beneath the site due to the mercury and multiple PAH exceedances recorded. A suitable thickness of clean and inert capping should be installed in all private gardens and areas of soft landscaping, detailed in a Remediation Strategy.

Construction workers should be provided with appropriate personal protection equipment (PPE) with consideration of the environmental testing results presented.

No gas protection measures will be required for the proposed residential development.

There is uncertainty as to whether shallow foundations will be feasible for all of the plots as Glacial Till was only encountered in two locations (WS01 and WS04) at depths of 1.40m and 2.60m bgl. It is recommended that a supplementary ground investigation is undertaken once a site turnover has been completed to grub out the buried concrete obstructions and determine the depth to a suitable founding stratum in other parts of the site.

Based on the depth to the Glacial Till in WS01 and WS04 only, shallow strip or trench fill foundations are likely to be feasible in these locations. Foundations will be required to be taken down below any Made Ground and be placed in the firm to stiff cohesive Glacial Till that exhibits an undrained shear strength in excess of 50kPa and cast at a minimum depth of 1.5m bgl.



Assuming a 0.6m wide strip foundation at a depth of 1.5m bgl, an allowable bearing capacity of 80kPa is likely to be achievable (limited to less than 25mm settlement).

A Design Sulphate class of DS-1 and ACEC Class of AC-1 are deemed suitable for the proposed development for Made Ground and Glacial Till.

If you have any queries with regard to this letter report, please do not hesitate to contact us.

Yours sincerely,

J Lord
Senior Geo-Environmental Engineer
For and on behalf of
Curtins Consulting Ltd



Author	Signature	Date
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Checked	Signature	Date
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R Stringer BSc (Hons) CGeol EurGeol FGS Principal Geotechnical Engineer		29 November 2022

Rev	Description	Issued by	Checked	Date



References

1. Environmental Agency (2020) Land Contamination Risk Management (LCRM)
2. British Standards Institution (2011) Investigation of Potentially Contaminated Sites: Code of Practice (report no. BS10175:2011+A2:2017).
3. British Standards Institution BSI (2015+2000) Code of Practice for Site Investigations (report no. BS5930).
4. British Standards Institution (2006) Eurocode 7: Geotechnical Design (report no. BSEN1997).
5. British Standard (2016) Methods of test for soils for civil engineering purposes. General requirements and sample preparation (report no. BS1377-1).
6. NHBC Standards (2021) Chapter 4.2
7. British Standards Institution (BS 8002:2015) Code of Practice for Earth Retaining Structures.
8. NHBC (2007) Guidance of Evaluation of Development Proposals on sites where methane and carbon dioxide are present. Report Edition No. 4

Appendices

Appendix A1 – Drawings

Appendix A2 – Exploratory Hole Logs

Appendix A3 – Geo-Environmental Laboratory Testing Results

Appendix A4 – Generic Assessment Criteria (GACs)

Appendix A5 – Geotechnical Laboratory Testing Results

Appendix A6 – Ground Gas Monitoring Results



Appendix A1 – Drawings



SITE INVESTIGATION

 BOREHOLE LOCATIONS

Rev:	Description:	Date:	By:	Chkd:
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Structures - Civils - Environmental - Infrastructure - Transport Planning - Sustainability - Expert Advisory Services
Birmingham - Bristol - Cardiff - Douglas - Edinburgh - Kendal - Leeds - Liverpool - London - Manchester - Nottingham

Status:

INFORMATION

Project:

DAM HEAD DRIVE, BLACKLEY

Drg Title:

EXPLORATORY HOLE LOCATION PLAN

Scale:	Size:	First Issue:	Drawn:	Checked:
NTS	A3	16/08/2022	JL	RA

Drg No:	Rev:
077873-CUR-00-XX-DR-GE-80001	P02

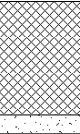
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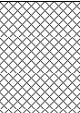


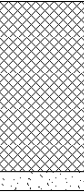
Appendix A2 – Exploratory Hole Logs

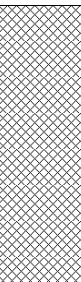
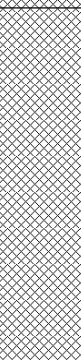
				Borehole Log			Borehole No. WS01 Sheet 1 of 1		
Project Name: Dam Head Drive				Project No. 077873.03		Co-ords: -		Hole Type WS	
Location: Dam Head Drive, Blackley, M9 6DH				Level: 95.94		Scale 1:20			
Client: Guinness Partnership				Dates: 10/08/2022 - 10/08/2022		Logged By JL			
Well	Water Strikes	Samples and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
		0.25			95.69		MADE GROUND: Grass over dark brown slightly clayey gravelly SAND. Sand is fine to medium. Gravel is fine to coarse angular to subrounded of brick, concrete, sandstone and plastic	1 2 3 4	
	0.50	ES					MADE GROUND: Soft to firm brown gravelly very sandy CLAY. Gravel is fine to medium, angular to rounded of brick, concrete, sandstone and coal		
			0.95			94.99			MADE GROUND: Grey sandy GRAVEL of fine to coarse angular to subangular of brick, concrete and sandstone
	1.20		N=19 (3,3/3,5,6,5)						
			1.40			94.54			Firm to stiff brown slightly sandy slightly gravelly CLAY. Gravel is fine to medium subangular to subrounded of sandstone and coal
	1.80	D							
		2.00						2	
		2.20			93.74		Brown gravelly SAND. Sand is medium to coarse. Gravel is fine to coarse subangular to subrounded of sandstone, coal and mudstone.		
2.50	D								
		3.00		50 (25 for 115mm/50 for 190mm)				3	
		3.45			92.49		End of borehole at 3.45 m	4	

Remarks
Service inspection pit hand excavated from GL to 1.20m. Groundwater encountered at 1.35m and 2.30m bgl.

					Borehole Log			Borehole No. WS02 Sheet 1 of 1			
Project Name: Dam Head Drive					Project No. 077873.03		Co-ords: -		Hole Type WS		
Location: Dam Head Drive, Blackley, M9 6DH					Level: 95.43		Scale 1:20		Logged By JL		
Client: Guinness Partnership					Dates: 10/08/2022 - 10/08/2022						
Well	Water Strikes	Samples and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description			
		Depth (m)	Type	Results							
					0.30	95.13		MADE GROUND: Grass over slightly gravelly SAND. Sand is fine to coarse. Gravel is fine to coarse angular to rounded of concrete, brick sandstone and timber		1	
					0.35	95.08		MADE GROUND: CONCRETE End of borehole at 0.35 m			
										2	
										3	
										4	
Remarks Service inspection pit hand excavated from GL to 1.20m. No groundwater encountered. Terminated at 0.35m bgl on concrete obstruction. Relocated to WS02A											

					Borehole Log			Borehole No. WS02A Sheet 1 of 1	
Project Name: Dam Head Drive					Project No. 077873.03		Co-ords: -		Hole Type WS
Location: Dam Head Drive, Blackley, M9 6DH					Level: 95.00		Scale 1:20		
Client: Guinness Partnership					Dates: 10/08/2022 - 10/08/2022		Logged By JL		
Well	Water Strikes	Samples and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
					0.30	94.70		MADE GROUND: Grass over slightly gravelly SAND. Sand is fine to coarse. Gravel is fine to coarse angular to rounded of concrete, brick sandstone and timber	1
					0.35	94.65		MADE GROUND: CONCRETE ----- End of borehole at 0.35 m	
									2
									3
									4
Remarks Service inspection pit hand excavated from GL to 1.20m. No groundwater encountered. Terminated at 0.35m bgl on concrete obstruction. Relocated to WS02B									

					Borehole Log			Borehole No. WS02B Sheet 1 of 1	
Project Name: Dam Head Drive					Project No. 077873.03		Co-ords: -		Hole Type WS
Location: Dam Head Drive, Blackley, M9 6DH					Level: 95.60		Scale 1:20		
Client: Guinness Partnership					Dates: 10/08/2022 - 10/08/2022		Logged By JL		
Well	Water Strikes	Samples and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description	
		Depth (m)	Type	Results					
		0.35	ES		0.45 0.50	95.15 95.10		MADE GROUND: Grass over slightly gravelly SAND. Sand is fine to coarse. Gravel is fine to coarse angular to rounded of concrete, brick sandstone and timber	1
								MADE GROUND: CONCRETE End of borehole at 0.50 m	
									2
									3
									4
Remarks Service inspection pit hand excavated from GL to 1.20m. No groundwater encountered. Terminated at 0.50m bgl on concrete obstruction. Relocated to WS02B									
									

					Borehole Log			Borehole No. WS03 Sheet 1 of 1			
Project Name: Dam Head Drive					Project No. 077873.03		Co-ords: -		Hole Type WS		
Location: Dam Head Drive, Blackley, M9 6DH					Level: 93.71		Scale 1:20		Logged By JL		
Client: Guinness Partnership					Dates: 10/08/2022 - 10/08/2022						
Well	Water Strikes	Samples and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description			
		Depth (m)	Type	Results							
		0.60	ES		0.75	92.96		MADE GROUND: Grass over greyish brown slightly clayey gravelly SAND. Sand is fine to coarse. Gravel is fine to coarse angular to subrounded of brick, metal, concrete, sandstone, plastic and coal	1		
		1.20		N=9 (3,2/3,2,2,2)				MADE GROUND: Brownish grey very sandy GRAVEL, with frequent cobbles. Gravel is angular to subangular, fine to coarse of brick, metal and concrete. Cobbles are angular of brick.			
		1.70		50 (25 for 30mm/50 for 20mm)	1.70 1.75	92.01 91.96		MADE GROUND: CONCRETE End of borehole at 1.75 m			
Remarks Service inspection pit hand excavated from GL to 1.20m. No groundwater encountered. Terminated at 1.75m bgl on concrete obstruction.											

					Borehole Log		Borehole No. WS04 Sheet 1 of 1	
Project Name: Dam Head Drive				Project No. 077873.03		Co-ords: -		Hole Type WS
Location: Dam Head Drive, Blackley, M9 6DH				Level: 93.10		Scale 1:20		
Client: Guinness Partnership				Dates: 10/08/2022 - 10/08/2022		Logged By JL		
Well	Water Strikes	Samples and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.20	92.90		MADE GROUND: Grass over dark brown slightly gravelly SAND. Sand is fine to medium. Gravel is fine to coarse subangular of brick
		0.40	ES					MADE GROUND: Brown slightly clayey gravelly SAND. Sand is fine to coarse. Gravel is fine to coarse angular to subrounded of brick, metal, glass, wood, timber, concrete and sandstone.
					0.85	92.25		MADE GROUND: Brownish grey very sandy GRAVEL, with occasional cobbles. Gravel is fine to coarse angular to subrounded of brick, concrete, sandstone. Cobbles are angular to subangular of brick.
		1.20		N=9 (3,3/2,2,2,3)				
		2.00		N=8 (2,2/2,2,2,2)				
					2.40	90.70		MADE GROUND: Grey GRAVEL of fine to coarse angular to subangular of sandstone aggregate
			2.60	90.50		Firm to stiff brown gravelly CLAY. Gravel is fine to coarse subangular to subrounded of sandstone.		
		2.80	D					
		3.00		N=9 (3,2/3,2,2,2)				
					3.45	89.65	End of borehole at 3.45 m	

Remarks

Service inspection pit hand excavated from GL to 1.20m. Groundwater encountered at 0.90m bgl. Terminated at 3.45m bgl due to continual collapse of granular deposits.



Appendix A3 – Geo-Environmental Laboratory Testing Results

FINAL ANALYTICAL TEST REPORT

Envirolab Job Number: 22/07885
Issue Number: 1

Date: 19 August, 2022

Client: Curtins Consulting (Manchester)
Merchant Exchange
17-19 Whitworth Street
Manchester
UK
M1 5WG

Project Manager: Jonathan Lord
Project Name: P500 - Dam Head Drive
Project Ref: 077873.002
Order No: EBMA3483
Date Samples Received: 11/08/22
Date Instructions Received: 12/08/22
Date Analysis Completed: 19/08/22

Approved by:



Richard Wong
Client Manager

Envirolab Job Number: 22/07885

Client Project Name: P500 - Dam Head Drive

Client Project Ref: 077873.002

Lab Sample ID	22/07885/1	22/07885/2	22/07885/3	22/07885/4				Units	Limit of Detection	Method ref
Client Sample No										
Client Sample ID	WS01	WS02	WS03	WS04						
Depth to Top	0.50	0.35	0.60	0.40						
Depth To Bottom										
Date Sampled	10-Aug-22	10-Aug-22	10-Aug-22	10-Aug-22						
Sample Type	Soil - ES	Soil - ES	Soil - ES	Soil - ES						
Sample Matrix Code	4AB	4AE	4ABE	4ABE						
% Stones >10mm _A	<0.1	<0.1	<0.1	<0.1				% w/w	0.1	A-T-044
pH _D ^{M#}	8.56	9.52	8.18	7.95				pH	0.01	A-T-031s
Sulphate (water sol 2:1) _D ^{M#}	0.05	0.13	0.13	0.04				g/l	0.01	A-T-026s
Cyanide (total) _A ^{M#}	<1	<1	<1	<1				mg/kg	1	A-T-042sTCN
Phenols - Total by HPLC _A	<0.2	<0.2	<0.2	<0.2				mg/kg	0.2	A-T-050s
Organic Matter _D ^{M#}	1.1	4.2	5.0	5.9				% w/w	0.1	A-T-032 OM
Arsenic _D ^{M#}	2	8	10	7				mg/kg	1	A-T-024s
Boron (water soluble) _D	<1.0	<1.0	<1.0	<1.0				mg/kg	1	A-T-027s
Cadmium _D ^{M#}	1.9	1.0	1.3	1.2				mg/kg	0.5	A-T-024s
Copper _D ^{M#}	22	42	57	51				mg/kg	1	A-T-024s
Chromium _D ^{M#}	25	26	23	21				mg/kg	1	A-T-024s
Chromium (hexavalent) _D	<1	<1	<1	<1				mg/kg	1	A-T-040s
Lead _D ^{M#}	30	157	188	151				mg/kg	1	A-T-024s
Mercury _D	0.23	1.36	1.41	1.48				mg/kg	0.17	A-T-024s
Nickel _D ^{M#}	32	17	22	27				mg/kg	1	A-T-024s
Selenium _D ^{M#}	<1	<1	<1	<1				mg/kg	1	A-T-024s
Zinc _D ^{M#}	69	146	145	121				mg/kg	5	A-T-024s

Envirolab Job Number: 22/07885

Client Project Name: P500 - Dam Head Drive

Client Project Ref: 077873.002

Lab Sample ID	22/07885/1	22/07885/2	22/07885/3	22/07885/4				Units	Limit of Detection	Method ref
Client Sample No										
Client Sample ID	WS01	WS02	WS03	WS04						
Depth to Top	0.50	0.35	0.60	0.40						
Depth To Bottom										
Date Sampled	10-Aug-22	10-Aug-22	10-Aug-22	10-Aug-22						
Sample Type	Soil - ES	Soil - ES	Soil - ES	Soil - ES						
Sample Matrix Code	4AB	4AE	4ABE	4ABE						
Asbestos in Soil (inc. matrix)										
Asbestos in soil _D [#]	NAD	NAD	NAD	NAD						A-T-045
Asbestos Matrix (visual) _D	-	-	-	-						A-T-045
Asbestos Matrix (microscope) _D	-	-	-	-						A-T-045
Asbestos ACM - Suitable for Water Absorption Test? _D	N/A	N/A	N/A	N/A						A-T-045

Envirolab Job Number: 22/07885

Client Project Name: P500 - Dam Head Drive

Client Project Ref: 077873.002

Lab Sample ID	22/07885/1	22/07885/2	22/07885/3	22/07885/4				Units	Limit of Detection	Method ref
Client Sample No										
Client Sample ID	WS01	WS02	WS03	WS04						
Depth to Top	0.50	0.35	0.60	0.40						
Depth To Bottom										
Date Sampled	10-Aug-22	10-Aug-22	10-Aug-22	10-Aug-22						
Sample Type	Soil - ES	Soil - ES	Soil - ES	Soil - ES						
Sample Matrix Code	4AB	4AE	4ABE	4ABE						
PAH-16MS										
Acenaphthene _A ^{M#}	<0.01	0.22	2.27	0.71				mg/kg	0.01	A-T-019s
Acenaphthylene _A ^{M#}	<0.01	0.06	0.15	0.14				mg/kg	0.01	A-T-019s
Anthracene _A ^{M#}	<0.02	0.56	3.88	1.69				mg/kg	0.02	A-T-019s
Benzo(a)anthracene _A ^{M#}	<0.04	1.60	7.66	5.39				mg/kg	0.04	A-T-019s
Benzo(a)pyrene _A ^{M#}	<0.04	1.55	7.05	5.90				mg/kg	0.04	A-T-019s
Benzo(b)fluoranthene _A ^{M#}	<0.05	1.66	6.73	5.51				mg/kg	0.05	A-T-019s
Benzo(ghi)perylene _A ^{M#}	<0.05	0.92	3.94	3.75				mg/kg	0.05	A-T-019s
Benzo(k)fluoranthene _A ^{M#}	<0.07	0.63	2.59	2.16				mg/kg	0.07	A-T-019s
Chrysene _A ^{M#}	<0.06	1.85	7.76	5.39				mg/kg	0.06	A-T-019s
Dibenzo(ah)anthracene _A ^{M#}	<0.04	0.25	1.28	1.14				mg/kg	0.04	A-T-019s
Fluoranthene _A ^{M#}	<0.08	3.53	15.6	10.7				mg/kg	0.08	A-T-019s
Fluorene _A ^{M#}	<0.01	0.17	1.91	0.50				mg/kg	0.01	A-T-019s
Indeno(123-cd)pyrene _A ^{M#}	<0.03	1.05	4.97	4.79				mg/kg	0.03	A-T-019s
Naphthalene _A ^{M#}	<0.03	0.06	0.95	0.21				mg/kg	0.03	A-T-019s
Phenanthrene _A ^{M#}	0.04	2.05	14.3	5.54				mg/kg	0.03	A-T-019s
Pyrene _A ^{M#}	<0.07	3.25	14.2	9.77				mg/kg	0.07	A-T-019s
Total PAH-16MS _A ^{M#}	<0.08	19.4	95.2	63.3				mg/kg	0.01	A-T-019s

Envirolab Job Number: 22/07885

Client Project Name: P500 - Dam Head Drive

Client Project Ref: 077873.002

Lab Sample ID	22/07885/1	22/07885/2	22/07885/3	22/07885/4				Units	Limit of Detection	Method ref
Client Sample No										
Client Sample ID	WS01	WS02	WS03	WS04						
Depth to Top	0.50	0.35	0.60	0.40						
Depth To Bottom										
Date Sampled	10-Aug-22	10-Aug-22	10-Aug-22	10-Aug-22						
Sample Type	Soil - ES	Soil - ES	Soil - ES	Soil - ES						
Sample Matrix Code	4AB	4AE	4ABE	4ABE						
TPH CWG with Clean Up										
Ali >C5-C6 _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
Ali >C6-C8 _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
Ali >C8-C10 _A	<1	<1	1	<1				mg/kg	1	A-T-055s
Ali >C10-C12 _A ^{M#}	<1	<1	2	1				mg/kg	1	A-T-055s
Ali >C12-C16 _A ^{M#}	<1	1	8	3				mg/kg	1	A-T-055s
Ali >C16-C21 _A ^{M#}	<1	3	14	6				mg/kg	1	A-T-055s
Ali >C21-C35 _A ^{M#}	16	123	234	181				mg/kg	1	A-T-055s
Total Aliphatics _A	15	115	236	177				mg/kg	1	Calc-As Recd
Aro >C5-C7 _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
Aro >C7-C8 _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
Aro >C8-C10 _A	<1	<1	2	1				mg/kg	1	A-T-055s
Aro >C10-C12 _A	<1	1	8	3				mg/kg	1	A-T-055s
Aro >C12-C16 _A	2	7	40	28				mg/kg	1	A-T-055s
Aro >C16-C21 _A ^{M#}	1	61	163	164				mg/kg	1	A-T-055s
Aro >C21-C35 _A ^{M#}	4	143	510	471				mg/kg	1	A-T-055s
Total Aromatics _A	7	192	657	618				mg/kg	1	Calc-As Recd
TPH (Ali & Aro >C5-C35) _A	22	307	893	795				mg/kg	1	Calc-As Recd
BTEX - Benzene _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
BTEX - Toluene _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
BTEX - Ethyl Benzene _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
BTEX - m & p Xylene _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
BTEX - o Xylene _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s
MTBE _A [#]	<0.01	<0.01	<0.01	<0.01				mg/kg	0.01	A-T-022s

REPORT NOTES

General

This report shall not be reproduced, except in full, without written approval from Envirolab.

The results reported herein relate only to the material supplied to the laboratory.

The residue of any samples contained within this report, and any received with the same delivery, will be disposed of six weeks after initial scheduling. For samples tested for Asbestos we will retain a portion of the dried sample for a minimum of six months after the initial Asbestos testing is completed.

Analytical results reflect the quality of the sample at the time of analysis only.

Opinions and interpretations expressed are outside the scope of our accreditation.

If results are in italic font they are associated with an AQC failure, these are not accredited and are unreliable.

A deviating samples report is appended and will indicate if samples or tests have been found to be deviating. Any test results affected may not be an accurate record of the concentration at the time of sampling and, as a result, may be invalid.

The Client Sample No, Client Sample ID, Depth to Top, Depth to Bottom and Date Sampled were all provided by the client.

Soil chemical analysis:

All results are reported as dry weight (<40°C).

For samples with Matrix Codes 1 - 6 natural stones, brick and concrete fragments >10mm and any extraneous material (visible glass, metal or twigs) are removed and excluded from the sample prior to analysis and reported results corrected to a whole sample basis. This is reported as '% stones >10mm'.

For samples with Matrix Code 7 the whole sample is dried and crushed prior to analysis and this supersedes any "A" subscripts

All analysis is performed on the sample as received for soil samples which are positive for asbestos or the client has informed asbestos may be present and/or if they are from outside the European Union and this supersedes any "D" subscripts.

TPH analysis of water by method A-T-007:

Free and visible oils are excluded from the sample used for analysis so that the reported result represents the dissolved phase only.

Electrical Conductivity of water by Method A-T-037:

Results greater than 12900µS/cm @ 25°C / 11550µS/cm @ 20°C fall outside the calibration range and as such are unaccredited.

Asbestos:

Asbestos in soil analysis is performed on a dried aliquot of the submitted sample and cannot guarantee to identify asbestos if only present in small numbers as discrete fibres/fragments in the original sample.

Stones etc. are not removed from the sample prior to analysis.

Quantification of asbestos is a 3 stage process including visual identification, hand picking and weighing and fibre counting by sedimentation/phase contrast optical microscopy if required. If asbestos is identified as being present but is not in a form that is suitable for analysis by hand picking and weighing (normally if the asbestos is present as free fibres) quantification by sedimentation is performed. Where ACMs are found a percentage asbestos is assigned to each with reference to 'HSG264, Asbestos: The survey guide' and the calculated asbestos content is expressed as a percentage of the dried soil sample aliquot used.

Predominant Matrix Codes:

1 = SAND, 2 = LOAM, 3 = CLAY, 4 = LOAM/SAND, 5 = SAND/CLAY, 6 = CLAY/LOAM, 7 = OTHER, 8 = Asbestos bulk ID sample, 9 = INCINERATOR ASH.

Samples with Matrix Code 7 & 8 are not predominantly a SAND/LOAM/CLAY mix and are not covered by our BSEN 17025 or MCERTS accreditations, with the exception of bulk asbestos which are BSEN 17025 accredited.

Secondary Matrix Codes:

A = contains stones, B = contains construction rubble, C = contains visible hydrocarbons, D = contains glass/metal,

E = contains roots/twigs.

Key:

IS indicates Insufficient Sample for analysis.

US indicates Unsuitable Sample for analysis.

NDP indicates No Determination Possible.

NAD indicates No Asbestos Detected.

N/A indicates Not Applicable.

Superscript # indicates method accredited to ISO 17025.

Superscript "M" indicates method accredited to MCERTS.

Subscript "A" indicates analysis performed on the sample as received.

Subscript "D" indicates analysis performed on the dried sample, crushed to pass a 2mm sieve

Subscript "A" indicates analysis has dependant options against results. Testing dependant on results appear in the comments area of your sample receipt.

EPH CWG results have humics mathematically subtracted through instrument calculation

TPH results "with Cleanup" indicates results cleaned up with Silica during extraction

EPH CWG GCxGC ID from TPH CWG

Where we have identified humic substances in any ID's from TPH CWG with Clean Up please note that the concentration of these

humic substances is not included in the quantified results and are included in the ID for information.

Please contact us if you need any further information.

Envirolab Deviating Samples Report

Units 7&8 Sandpits Business Park, Mottram Road, Hyde, SK14 3AR
Tel. 0161 368 4921 email. ask@envlab.co.uk

Client:	Curtins Consulting (Manchester), Merchant Exchange, 17-19 Whitworth Street , Manchester, UK, M1 5WG	Project No:	22/07885
Project:	P500 - Dam Head Drive	Date Received:	12/08/2022 (am)
Clients Project No:	077873.002	Cool Box Temperatures (°C):	19.7

NO DEVIATIONS IDENTIFIED

If, at any point before reaching the laboratory, the temperature of the samples has breached those set in published standards, e.g. BS-EN 5667-3, ISO 18400-102:2017, then the concentration of any affected analytes may differ from that at the time of sampling.

Envirolab Analysis Dates

Lab Sample ID	22/07885/1	22/07885/2	22/07885/3	22/07885/4
Client Sample No				
Client Sample ID/Depth	WS01 0.50m	WS02 0.35m	WS03 0.60m	WS04 0.40m
Date Sampled	10/08/22	10/08/22	10/08/22	10/08/22
A-T-019s	18/08/2022	18/08/2022	18/08/2022	18/08/2022
A-T-022s	17/08/2022	17/08/2022	17/08/2022	17/08/2022
A-T-024s	18/08/2022	18/08/2022	18/08/2022	18/08/2022
A-T-026s	18/08/2022	18/08/2022	18/08/2022	18/08/2022
A-T-027s	17/08/2022	17/08/2022	17/08/2022	17/08/2022
A-T-031s	18/08/2022	18/08/2022	18/08/2022	18/08/2022
A-T-032 OM	17/08/2022	17/08/2022	17/08/2022	17/08/2022
A-T-040s	18/08/2022	18/08/2022	18/08/2022	18/08/2022
A-T-042sTCN	19/08/2022	19/08/2022	19/08/2022	19/08/2022
A-T-044	17/08/2022	17/08/2022	17/08/2022	17/08/2022
A-T-045	16/08/2022	16/08/2022	16/08/2022	16/08/2022
A-T-050s	16/08/2022	16/08/2022	16/08/2022	16/08/2022
A-T-055s	18/08/2022	18/08/2022	18/08/2022	18/08/2022
Calc-As Recd	18/08/2022	18/08/2022	18/08/2022	18/08/2022

The above dates are the analysis completion dates, please note that these are not necessarily the date that the analysis was weighed/extracted.

End of Report



Appendix A4 – Generic Assessment Criteria (GACs)

Adopted Soil Generic Assessment Criteria
Sandy loam with 6% SOM



Contaminants	Residential <u>with</u> home grown produce	Residential <u>without</u> home grown produce	Allotments	Commercial	Public open space near residential housing POS _{resi}	Public park POS _{park}
Metals						
Beryllium	1.7	1.7	35	12	2.2	63
Boron	290	11,000	45	240,000	21,000	46,000
Cadmium	10⁽¹³⁾ 22	85⁽¹³⁾ 150	1.8 3.9	230 410	120 220	560 880
Chromium III	910	910	18,000	8,600	1,500	33,000
Chromium VI	6 21	6 21	1.8 170	33 49	7.7 21	220 250
Lead	200	310	80	2,300	630	1,300
Mercury (<i>elemental</i>)	1	1	26	26	16	26⁽⁸⁾ [30]
Mercury (<i>inorganic</i>)	170	240	80	3600	120	240
Nickel	130⁽¹⁰⁾	180⁽¹⁰⁾	53⁽¹¹⁾	980⁽¹⁰⁾	230	800
Vanadium	410	1200	91	9000	2000	5000
Copper	2400	7100	520	68000	12000	44000
Zinc	3700	40000	620	730000	81000	170000
Semi-Metals and non-metals						
Arsenic	32⁽¹²⁾ 37	35⁽¹²⁾ 40	43⁽¹²⁾ 49	640⁽¹²⁾ 640	79 79	170 170
Antimony		550		7500	1500	3300
Selenium	350	600	120	13000	1100	1800
Inorganic chemicals						
Cyanide	34	34	34	34	34	34
Organic contaminants						
<i>Aliphatic risk banded hydrocarbons - TPHCWG method</i>						
EC ₅₋₅ - EC ₆	160	160	3900	12000	600000	180000
EC ₅₋₆ - EC ₈	530	530	13000	40000	620000	320000
EC ₅₋₈ - EC ₁₀	150	150	1700	11000	13000	21000
EC ₁₀₋₁₀ - EC ₁₂	760	770	7300	47000	13000	24000
EC ₁₂₋₁₂ - EC ₁₆	4300	4400	13000	90000	13000	26000
EC ₅₋₁₆ - EC ₃₅	110000	110000	270000	1800000	250000	490000
EC ₃₅₋₃₅ - EC ₄₄	110000	110000	270000	1800000	250000	490000
<i>Aromatic risk banded hydrocarbons - TPHCWG method</i>						
EC ₅₋₅ - EC ₇	300	1400	57	86000	56000	92000
EC ₅₋₇ - EC ₈	660	3900	120	180000	56000	100000
EC ₅₋₈ - EC ₁₀	190	270	51	17000	5000	9300
EC ₁₀₋₁₀ - EC ₁₂	380	1200	74	34000	5000	10000
EC ₁₂₋₁₂ - EC ₁₆	660	2500	130	38000	5000	10000
EC ₅₋₁₆ - EC ₂₁	930	1900	260	28000	3800	7800
EC ₂₁₋₂₁ - EC ₃₅	1700	1900	1600	28000	3800	7900
EC ₃₅₋₃₅ - EC ₄₄	1700	1900	1600	28000	3800	7900
Aliph + Arom EC >44-70	1900	1900	3000	28000	3800	7900
Aromatic						
Benzene	0.33 0.87	1.0 3.3	0.07 0.18	95 98	73 140	110 230
Ethyl benzene	350	840	90	2800⁽⁸⁾ [66000]	2800⁽⁸⁾ [25000]	2800⁽⁸⁾ [27000]
Toluene	610	2700	120	4400⁽⁸⁾ [190000]	4400⁽⁸⁾ [56000]	4400⁽⁸⁾ [100000]
Xylene ⁽⁹⁾	230	290	160	2600⁽⁸⁾ [32000]	2600⁽⁸⁾ [43000]	2600⁽⁸⁾ [31000]
Phenol	420	520	280	3200⁽¹⁴⁾ (38000)	3200⁽¹⁴⁾ (10000)	3200⁽¹⁴⁾ (9300)
Polycyclic Aromatic Hydrocarbons (PAH)						
Naphthalene	13	13	24	1100	4900	3000
Acenaphthylene	920	6000	160	100000	15000	30000
Acenaphthene	1100	6000	200	100000	15000	30000
Fluorene	860	4500	160	71000	9900	20000
Phenanthrene	440	1500	90	23000	3100	6300
Anthracene	11000	37000	2200	540000	74000	150000
Fluoranthene	890	1600	290	23000	3100	6400
Pyrene	2000	3800	620	54000	7400	15000
Benz(a)anthracene	13	15	13	180	29	62
Chrysene	27	32	19	350	57	120
Benzo(b)fluoranthene	3.7	4.0	3.9	45	7.2	16.0
Benzo(k)fluoranthene	100	110	130	1200	190	440
Benzo(a)pyrene	3.0 5.0	3.2 5.3	3.5 5.7	36 77	5.7 10	13 21
Indeno(123cd)pyrene	41	46	39	510	82	180
Dibenzo(ah)anthracene	0.3	0.32	0.43	3.6	0.58	1.4
Benzo(ghi)perylene	350	360	640	4000	640	1600
Chlorinated Aliphatic Hydrocarbons						
Vinyl chloride	0.0014	0.0015	0.0018	0.12	3.5	5.4
Trichloroethene (TCE)	0.075	0.08	0.21	5.7	120	120
1,1,1,2 Tetrachlorethane	6.4	8.2	4.4	560	1400	2100
Tetrachlorethane (PCE)	0.90	0.92	3.6	95	1400	1500
1,1,1 Trichlorethane	39	40	240	3000	140000	100000

Notes

- All values above are in mg/kg
- Numbers in bold are SGVs or GAC that are derived based on SGV report input parameters, numbers in italics are S4ULs, numbers in bold-italics are based on EIC/AGS/CL:AIRE numbers & input parameters and underlined numbers are C4SLs**
- Soil organic matter (SOM) is assumed to be 6% - DEFAULT VALUE
- Soil type is assumed to be sandy loam - DEFAULT SOIL TYPE
- For residential, the building type is conservatively assumed to be a small terrace house where the development includes bungalows change to more conservative bungalow setting in computer model
- For commercial, the building type is conservatively assumed to be a pre 1970s office building, where the proposed development comprises houses, flat with living spaces changes setting in model accordingly
- For classrooms consider increasing the dust loading factor in the 'Soil and Building Data' of the CLEA 1.04 model from 50 to 100µg m⁻³
- Based on vapour saturation limit as suggested by EA / [] model value
- Lowest of o-, m- and p-xylene
- Based on comparison of inhalation exposure with inhalation TDI
- Based on comparison of oral, dermal, and inhalation exposure with the oral TDI
- Based on a comparison of oral and dermal soil exposure with oral Index Dose only
- Averaged over and based on lifetime exposure
- Based on critical concentration for skin irritation in humans arising from contact with phenol in aqueous solution (number in brackets based on health effects following long term exposure for illustration)
- NA: Not applicable

Adopted Soil Generic Assessment Criteria
Sandy loam with 2.5% SOM



Contaminants	Residential <u>with</u> home grown produce	Residential <u>without</u> home grown produce	Allotments	Commercial	Public open space near residential housing POS _{resi}	Public park POS _{park}
Metals						
Beryllium	1.7	1.7	35	12	2.2	63
Boron	290	11,000	45	240,000	21,000	46,000
Cadmium	10⁽¹³⁾ 22	85⁽¹³⁾ 150	1.8 3.9	230 410	120 220	560 880
Chromium III	910	910	18,000	8,600	1,500	33,000
Chromium VI	6 <u>21</u>	6 <u>21</u>	1.8 <u>170</u>	33 <u>49</u>	7.7 <u>21</u>	220 <u>250</u>
Lead	<u>200</u>	<u>310</u>	<u>80</u>	<u>2,300</u>	<u>630</u>	<u>1,300</u>
Mercury (elemental)	1	1	26	26	16	26 ⁽⁸⁾ [30]
Mercury (inorganic)	170	240	80	3600	120	240
Nickel	130 ⁽¹⁰⁾	180 ⁽¹⁰⁾	53 ⁽¹¹⁾	980 ⁽¹⁰⁾	230	800
Vanadium	410	1200	91	9000	2000	5000
Copper	2400	7100	520	68000	12000	44000
Zinc	3700	40000	620	730000	81000	170000
Semi-Metals and non-metals						
Arsenic	32⁽¹²⁾ 37	35⁽¹²⁾ 40	43⁽¹²⁾ 49	640⁽¹²⁾ 640	79 <u>79</u>	170 <u>170</u>
Antimony		550		7500	1500	3300
Selenium	350	600	120	13000	1100	1800
Inorganic chemicals						
Cyanide	34	34	34	34	34	34
Organic contaminants						
<i>Aliphatic risk banded hydrocarbons - TPHCWG method</i>						
EC ₅ - EC ₆	78	78	1700	5900	590000	130000
EC ₆ - EC ₈	230	230	5600	17000	610000	220000
EC ₈ - EC ₁₀	65	65	770	4800	13000	18000
EC ₁₀ - EC ₁₂	330	330	4400	23000	13000	23000
EC ₁₂ - EC ₁₆	2400	2400	13000	82000	13000	25000
EC ₁₆ - EC ₃₅	92000	92000	270000	1700000	250000	480000
EC ₃₅ - EC ₄₄	92000	92000	270000	1700000	250000	480000
<i>Aromatic risk banded hydrocarbons - TPHCWG method</i>						
EC ₅ - EC ₇	140	690	27	46000	56000	84000
EC ₇ - EC ₈	290	1800	51	110000	56000	95000
EC ₈ - EC ₁₀	83	110	21	8100	5000	8500
EC ₁₀ - EC ₁₂	180	590	31	28000	5000	9700
EC ₁₂ - EC ₁₆	330	2300	57	37000	5100	10000
EC ₁₆ - EC ₂₁	540	1900	110	28000	3800	7700
EC ₂₁ - EC ₃₅	1500	1900	820	28000	3800	7800
EC ₃₅ - EC ₄₄	1500	1900	820	28000	3800	7800
Aliph + Arom EC >44-70	1800	1900	2100	28000	3800	7800
<i>Aromatic</i>						
Benzene	0.16	0.49	0.035	50	72	100
Ethyl benzene	150	380	39	1200⁽⁸⁾ [35000]	1200⁽⁸⁾ [24000]	1200⁽⁸⁾ [22000]
Toluene	270	1300	51	1900⁽⁸⁾ [110000]	1900⁽⁸⁾ [56000]	1900⁽⁸⁾ [95000]
Xylene ⁽⁹⁾	98	120	70	1200⁽⁸⁾ [14000]	1200⁽⁸⁾ [42000]	1200⁽⁸⁾ [23000]
Phenol	290	420	140	1500⁽¹⁴⁾ (35000)	1500⁽¹⁴⁾ (10000)	1500⁽¹⁴⁾ (8300)
<i>Polycyclic Aromatic Hydrocarbons (PAH)</i>						
Naphthalene	5.6	5.6	10	460	4900	1900
Acenaphthylene	420	4600	69	97000	15000	30000
Acenaphthene	510	4700	85	97000	15000	30000
Fluorene	400	3800	67	68000	9900	20000
Phenanthrene	220	1500	38	22000	3100	6200
Anthracene	5400	35000	950	540000	74000	150000
Fluoranthene	560	1600	130	23000	3100	6300
Pyrene	1200	3800	270	54000	7400	15000
Benz(a)anthracene	11	14	6.5	170	29	56
Chrysene	22	31	9.4	350	57	110
Benzo(b)fluoranthene	3.3	4.0	2.1	44	7.2	15
Benzo(k)fluoranthene	93	110	75	1200	190	410
Benzo(a)pyrene	2.7	3.2	2	35	5.7	12
Indeno(123cd)pyrene	36	46	21	510	82	170
Dibenzo(ah)anthracene	0.28	0.32	0.27	3.6	0.57	1.3
Benzo(ghi)perylene	340	360	470	4000	640	1500
Chlorinated Aliphatic Hydrocarbons						
Vinyl chloride	0.00087	0.001	0.001	0.077	3.5	5
Trichloroethene (TCE)	0.034	0.036	0.091	2.6	120	91
1,1,1,2 Tetrachlorethane	2.8	3.5	1.9	250	1400	1800
Tetrachlorethane (PCE)	0.39	0.4	1.5	42	1400	1100
1,1,1 Trichlorethane	18	18	110	1300	140000	76000

Notes

- All values above are in mg/kg
- Numbers in bold are SGVs or GAC that are derived based on SGV report input parameters, numbers in italics are S4ULs, numbers in bold-italics are based on EIC/AGS/CL:AIRE numbers & input parameters and underlined numbers are C4SLs
- Soil organic matter (SOM) is assumed to be 2.5% - DEFAULT VALUE
- Soil type is assumed to be sandy loam - DEFAULT SOIL TYPE
- For residential, the building type is conservatively assumed to be a small terrace house where the development includes bungalows change to more conservative bungalow setting in computer model
- For commercial, the building type is conservatively assumed to be a pre 1970s office building, where the proposed development comprises houses, flat with living spaces changes setting in model accordingly
- For classrooms consider increasing the dust loading factor in the 'Soil and Building Data' of the CLEA 1.04 model from 50 to 100µg m⁻³
- Based on vapour saturation limit as suggested by EA / [] model value
- Lowest of o-, m- and p-xylene
- Based on comparison of inhalation exposure with inhalation TDI
- Based on comparison of oral, dermal, and inhalation exposure with the oral TDI
- Based on a comparison of oral and dermal soil exposure with oral Index Dose only
- Averaged over and based on lifetime exposure
- Based on critical concentration for skin irritation in humans arising from contact with phenol in aqueous solution (number in brackets based on health effects following long term exposure for illustration)
- NA: Not applicable

Adopted Soil Generic Assessment Criteria
Sandy loam with 1% SOM



Contaminants	Residential <u>with</u> home grown produce	Residential <u>without</u> home grown produce	Allotments	Commercial	Public open space near residential housing POS _{resi}	Public park POS _{park}
Metals						
Beryllium	1.7	1.7	35	12	2.2	63
Boron	290	11,000	45	240,000	21,000	46,000
Cadmium	10⁽¹³⁾ 22	85⁽¹³⁾ 150	1.8 3.9	230 410	120 220	560 880
Chromium III	910	910	18,000	8,600	1,500	33,000
Chromium VI	6 21	6 21	1.8 170	33 49	7.7 21	220 250
Lead	200	310	80	2,300	630	1,300
Mercury (<i>elemental</i>)	1	1	26	26	16	26⁽⁸⁾ [30]
Mercury (<i>inorganic</i>)	170	240	80	3600	120	240
Nickel	130 ⁽¹⁰⁾	180 ⁽¹⁰⁾	53 ⁽¹¹⁾	980 ⁽¹⁰⁾	230	800
Vanadium	410	1200	91	9000	2000	5000
Copper	2400	7100	520	68000	12000	44000
Zinc	3700	40000	620	730000	81000	170000
Semi-Metals and non-metals						
Arsenic	32⁽¹²⁾ 37	35⁽¹²⁾ 40	43⁽¹²⁾ 49	640⁽¹²⁾ 640	79 79	170 170
Antimony		550		7500	1500	3300
Selenium	350	600	120	13000	1100	1800
Inorganic chemicals						
Cyanide	34	34	34	34	34	34
Organic contaminants						
Aliphatic risk banded hydrocarbons - TPHCWG method						
EC ₅₋₅ - EC ₆	42	42	730	3200	570000	95000
EC ₆₋₆ - EC ₈	100	100	2300	7800	600000	150000
EC ₈₋₈ - EC ₁₀	27	27	320	2000	13000	14000
EC ₁₀₋₁₀ - EC ₁₂	130	130	2200	9700	13000	21000
EC ₁₂₋₁₂ - EC ₁₆	1100	1100	11000	59000	13000	25000
EC ₁₆₋₁₆ - EC ₃₅	65000	65000	260000	1600000	250000	450000
EC ₃₅₋₃₅ - EC ₄₄	65000	65000	260000	1600000	250000	450000
Aromatic risk banded hydrocarbons - TPHCWG method						
EC ₅₋₅ - EC ₇	70	370	13	26000	56000	76000
EC ₇₋₇ - EC ₈	130	860	22	56000	56000	87000
EC ₈₋₈ - EC ₁₀	34	47	8.6	3500	5000	7200
EC ₁₀₋₁₀ - EC ₁₂	74	250	13	16000	5000	9200
EC ₁₂₋₁₂ - EC ₁₆	140	1800	23	36000	5100	10000
EC ₁₆₋₁₆ - EC ₂₁	260	1900	46	28000	3800	7600
EC ₂₁₋₂₁ - EC ₃₅	1100	1900	370	28000	3800	7800
EC ₃₅₋₃₅ - EC ₄₄	1100	1900	370	28000	3800	7800
Aliph + Arom EC >44-70	1600	1900	1200	28000	3800	7800
Aromatic						
Benzene	0.08	0.3	0.017	28	72	90
Ethyl benzene	65	170	16	520⁽⁸⁾ [17000]	520⁽⁸⁾ [24000]	520⁽⁸⁾ [17000]
Toluene	120	610	22	860⁽⁸⁾ [59000]	860⁽⁸⁾ [56000]	860⁽⁸⁾ [87000]
Xylene ⁽⁹⁾	41	53	28	480⁽⁸⁾ [69000]	480⁽⁸⁾ [41000]	480⁽⁸⁾ [17000]
Phenol	180	310	66	760⁽¹⁴⁾ (31000)	760⁽¹⁴⁾ (10000)	760⁽¹⁴⁾ (7600)
Polycyclic Aromatic Hydrocarbons (PAH)						
Naphthalene	2.3	2.3	4.1	190	4900	1200
Acenaphthylene	170	2900	28	83000	15000	29000
Acenaphthene	210	3000	34	84000	15000	29000
Fluorene	170	2800	27	63000	9900	20000
Phenanthrene	95	1300	15	22000	3100	6200
Anthracene	2400	31000	380	520000	74000	150000
Fluoranthene	280	1500	52	23000	3100	6300
Pyrene	620	3700	110	54000	7400	15000
Benz(a)anthracene	7.2	11	2.9	170	29	49
Chrysene	15	30	4.1	350	57	93
Benzo(b)fluoranthene	2.6	3.9	0.99	44	7.1	13
Benzo(k)fluoranthene	77	110	37	1200	190	370
Benzo(a)pyrene	2.2	3.2	0.97	35	5.7	11
Indeno(123cd)pyrene	27	45	9.5	500	82	150
Dibenzo(ah)anthracene	0.24	0.31	0.14	3.5	0.57	1.1
Benzo(ghi)perylene	320	360	290	3900	640	1400
Chlorinated Aliphatic Hydrocarbons						
Vinyl chloride	0.00064	0.00077	0.00055	0.059	3.5	4.8
Trichloroethene (TCE)	0.016	0.017	0.041	1.2	120	70
1,1,1,2 Tetrachlorethane	1.2	1.5	0.79	110	1400	1500
Tetrachlorethane (PCE)	0.18	0.18	0.65	19	1400	810
1,1,1 Trichlorethane	8.8	9	48	660	140000	57000

Notes

- All values above are in mg/kg
- Numbers in bold are SGVs or GAC that are derived based on SGV report input parameters, numbers in italics are S4ULs, numbers in bold-italics are based on EIC/AGS/CL:AIRE numbers & input parameters and underlined numbers are C4SLs**
- Soil organic matter (SOM) is assumed to be 1% - DEFAULT VALUE
- Soil type is assumed to be sandy loam - DEFAULT SOIL TYPE
- For residential, the building type is conservatively assumed to be a small terrace house where the development includes bungalows change to more conservative bungalow setting in computer model
- For commercial, the building type is conservatively assumed to be a pre 1970s office building, where the proposed development comprises houses, flat with living spaces changes setting in model accordingly
- For classrooms consider increasing the dust loading factor in the 'Soil and Building Data' of the CLEA 1.04 model from 50 to 100µg m⁻³
- Based on vapour saturation limit as suggested by EA / [] model value
- Lowest of o-, m- and p-xylene
- Based on comparison of inhalation exposure with inhalation TDI
- Based on comparison of oral, dermal, and inhalation exposure with the oral TDI
- Based on a comparison of oral and dermal soil exposure with oral Index Dose only
- Averaged over and based on lifetime exposure
- Based on critical concentration for skin irritation in humans arising from contact with phenol in aqueous solution (number in brackets based on health effects following long term exposure for illustration)
- NA: Not applicable



Appendix A5 – Geotechnical Laboratory Testing Results

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Analytical Report Number : 22-80491

Project / Site name:	P500 Dam Head Drive	Samples received on:	22/08/2022
Your job number:	B077873	Samples instructed on/ Analysis started on:	26/08/2022
Your order number:	EBMA 3526	Analysis completed by:	08/09/2022
Report Issue Number:	1	Report issued on:	09/09/2022
Samples Analysed:	3 soil samples		


Signed:

Joanna Wawrzeczko
Reporting Specialist
For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41 -711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are :

soils	- 4 weeks from reporting
leachates	- 2 weeks from reporting
waters	- 2 weeks from reporting
asbestos	- 6 months from reporting

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Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies. An estimate of measurement uncertainty can be provided on request.

Analytical Report Number: 22-80491
Project / Site name: P500 Dam Head Drive
Your Order No: EBMA 3526

Lab Sample Number				2403868	2403869	2403870
Sample Reference				WS01	WS01	WS04
Sample Number				None Supplied	None Supplied	None Supplied
Depth (m)				1.80	2.50	2.80
Date Sampled				10/08/2022	10/08/2022	10/08/2022
Time Taken				None Supplied	None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status			
Stone Content	%	0.1	NONE	< 0.1	< 0.1	14
Moisture Content	%	0.01	NONE	13	9.1	11
Total mass of sample received	kg	0.001	NONE	0.4	0.4	0.3

General Inorganics

pH - Automated	pH Units	N/A	MCERTS	8.4	8.5	8.3
Total Sulphate as SO ₄	%	0.005	MCERTS	0.012	0.019	0.032
Water Soluble SO ₄ 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.015	0.04	0.026
Water Soluble Chloride (2:1) (leachate equivalent)	mg/l	0.5	MCERTS	1.3	1.4	2.7
Total Sulphur	%	0.005	MCERTS	0.008	0.012	0.064
Water Soluble Nitrate (2:1) as N (leachate equivalent)	mg/l	2	NONE	< 2.0	< 2.0	< 2.0

Heavy Metals / Metalloids

Magnesium (water soluble)	mg/kg	5	NONE	62	10	30
Magnesium (leachate equivalent)	mg/l	2.5	NONE	31	5.1	15

U/S = Unsuitable Sample I/S = Insufficient Sample

Analytical Report Number : 22-80491

Project / Site name: P500 Dam Head Drive

* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
2403868	WS01	None Supplied	1.8	Brown clay and loam.
2403869	WS01	None Supplied	2.5	Brown sandy gravel.**
2403870	WS04	None Supplied	2.8	Brown clay and sand with stones.

**Non MCERTS Matrix

Analytical Report Number : 22-80491

Project / Site name: P500 Dam Head Drive

Water matrix abbreviations:

Surface Water (SW) Potable Water (PW) Ground Water (GW) Process Waters (PrW) Final Sewage Effluent (FSE) Landfill Leachate (LL)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Sulphate, water soluble, in soil (16hr extraction)	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In house method.	L038-PL	D	MCERTS
Magnesium, water soluble, in soil	Determination of water soluble magnesium by extraction with water followed by ICP-OES.	In-house method based on TRL 447	L038-PL	D	NONE
Moisture Content	Moisture content, determined gravimetrically. (30 oC)	In house method.	L019-UK/PL	W	NONE
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement.	In house method.	L099-PL	D	MCERTS
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight.	In-house method based on British Standard Methods and MCERTS requirements.	L019-UK/PL	D	NONE
Total Sulphate in soil as %	Determination of total sulphate in soil by extraction with 10% HCl followed by ICP-OES.	In house method.	L038-PL	D	MCERTS
Total Sulphur in soil as %	Determination of total sulphur in soil by extraction with aqua-regia, potassium bromide/bromate followed by ICP-OES.	In house method.	L038-PL	D	MCERTS
Water Soluble Nitrate (2:1) as N in soil	Determination of nitrate by reaction with sodium salicylate and colorimetry.	In-house method based on Examination of Water and Wastewater & Polish Standard Method PN-82/C-04579.08, 2:1 extraction.	L078-PL	W	NONE
Chloride, water soluble, in soil	Determination of Chloride colorimetrically by discrete analyser.	In house method.	L082-PL	D	MCERTS

For method numbers ending in 'UK' analysis have been carried out in our laboratory in the United Kingdom.

For method numbers ending in 'PL' analysis have been carried out in our laboratory in Poland.

Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.

Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.

TEST CERTIFICATE

DETERMINATION OF LIQUID AND PLASTIC LIMITS

Tested in Accordance with: BS 1377-2:1990: Clause 4.3 and 5

i2 Analytical Ltd
Unit 8 Harrowden Road
Brackmills Industrial Estate
Northampton NN4 7EB

Client: Curtins
Client Address: 17-19 Whitworth Street West, Manchester,
M1 5WG

Contact: Jonathan Lord
Site Address: P500 Dam Head Drive

Client Reference: B077873
Job Number: 22-80490
Date Sampled: 10/08/2022
Date Received: 22/08/2022
Date Tested: 05/09/2022
Sampled By: Not Given

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

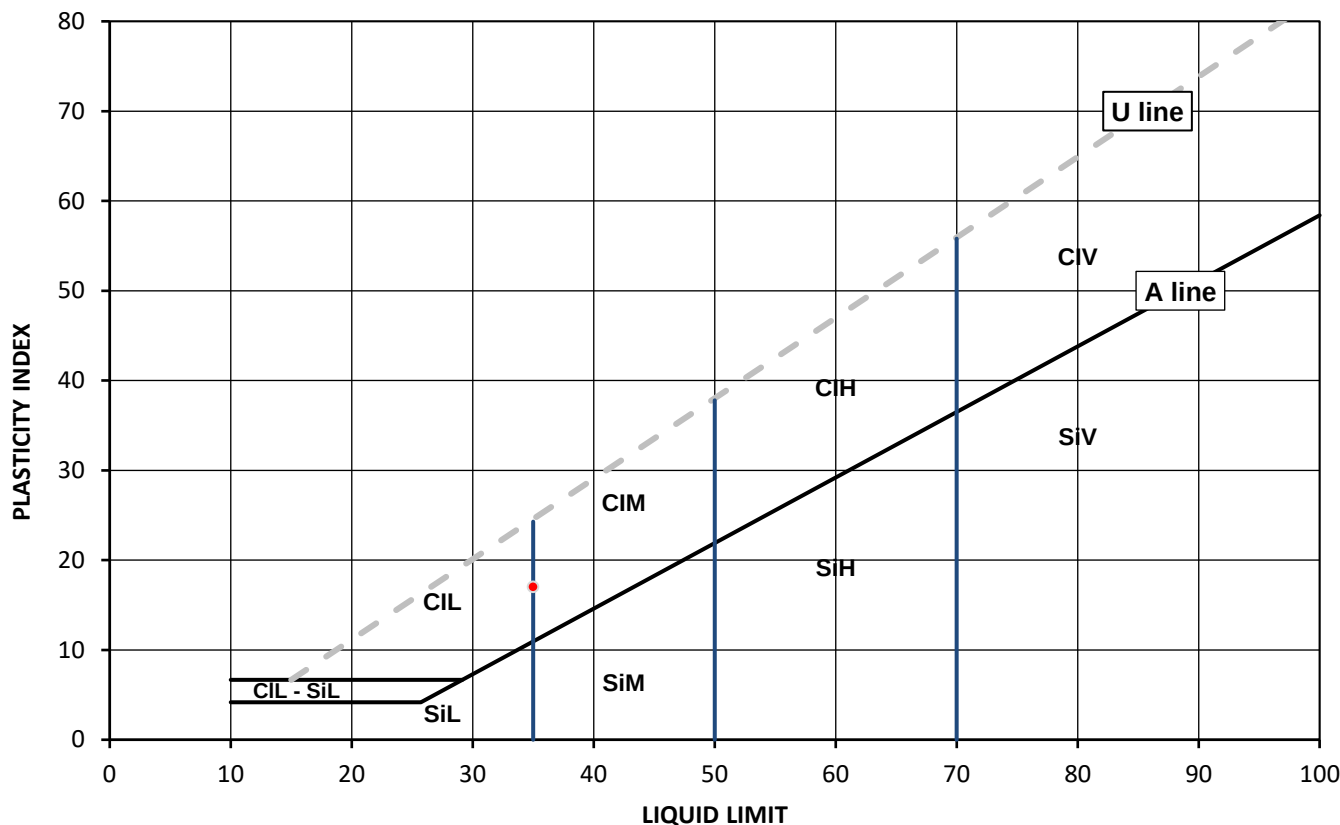
Test Results:

Laboratory Reference: 2403865
Hole No.: WS01
Sample Reference: Not Given
Sample Description: Brown sandy CLAY

Depth Top [m]: 1.80
Depth Base [m]: Not Given
Sample Type: D

Sample Preparation: Tested in natural condition

As Received Water Content [W] %	Liquid Limit [WL] %	Plastic Limit [Wp] %	Plasticity Index [Ip] %	% Passing 425µm BS Test Sieve
23	35	18	17	100



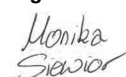
Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing – Identification and classification of soil

	Plasticity	Liquid Limit
Cl	Clay	below 35
Si	Silt	35 to 50
	L Low	50 to 70
	M Medium	exceeding 70
	H High	
	V Very high	
	O Organic	append to classification for organic material (eg CIHO)

Note: Water Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed:



Monika Siewior
Reporting Specialist
for and on behalf of i2 Analytical Ltd

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TEST CERTIFICATE

DETERMINATION OF LIQUID AND PLASTIC LIMITS

Tested in Accordance with: BS 1377-2:1990: Clause 4.3 and 5

i2 Analytical Ltd
Unit 8 Harrowden Road
Brackmills Industrial Estate
Northampton NN4 7EB

Client: Curtins
Client Address: 17-19 Whitworth Street West, Manchester,
M1 5WG

Contact: Jonathan Lord
Site Address: P500 Dam Head Drive

Client Reference: B077873
Job Number: 22-80490
Date Sampled: 10/08/2022
Date Received: 22/08/2022
Date Tested: 05/09/2022
Sampled By: Not Given

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

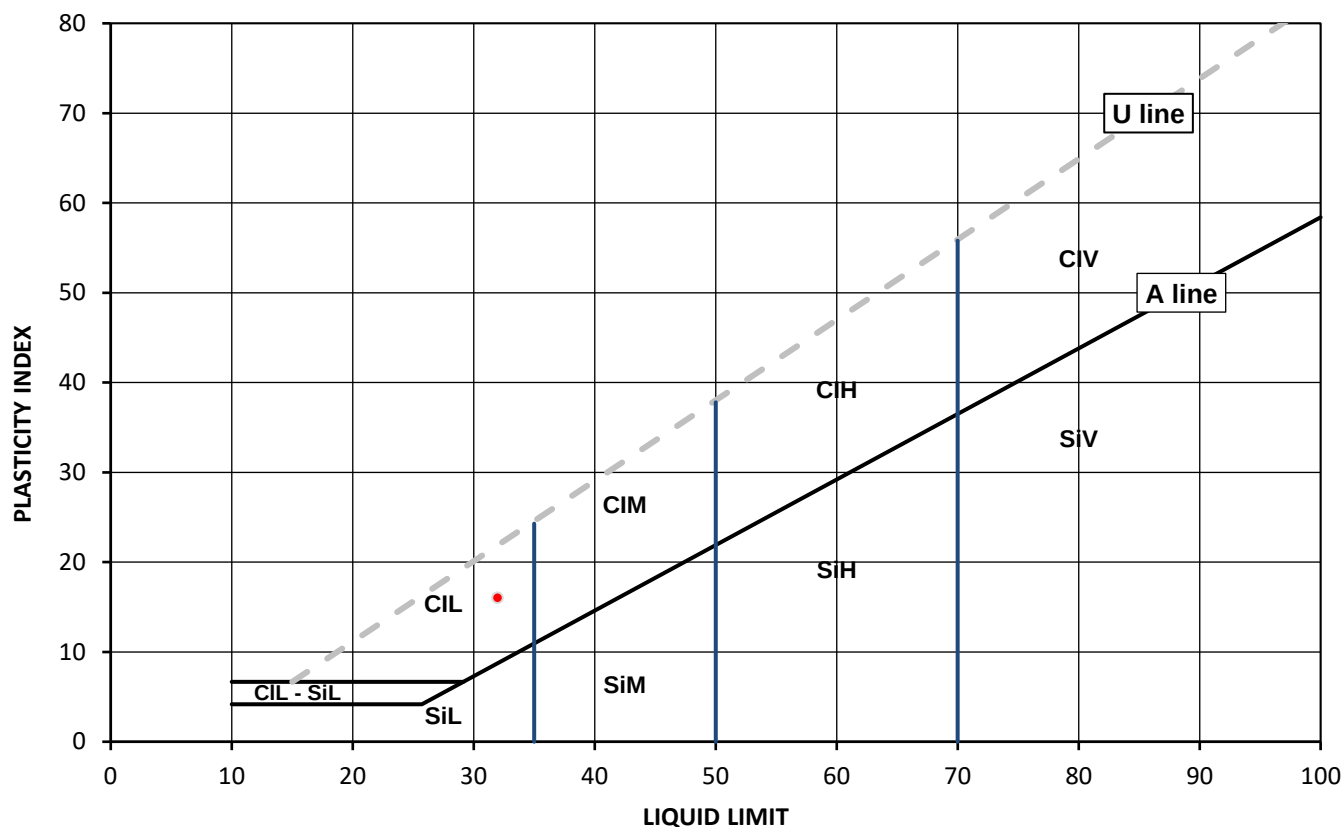
Test Results:

Laboratory Reference: 2403867
Hole No.: WS04
Sample Reference: Not Given
Sample Description: Brown slightly gravelly very sandy CLAY

Depth Top [m]: 2.80
Depth Base [m]: Not Given
Sample Type: D

Sample Preparation: Tested after washing to remove >425um

As Received Water Content [W] %	Liquid Limit [WL] %	Plastic Limit [Wp] %	Plasticity Index [Ip] %	% Passing 425µm BS Test Sieve
18	32	16	16	79



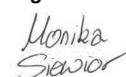
Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing – Identification and classification of soil

	Plasticity	Liquid Limit
Cl Clay	L Low	below 35
Si Silt	M Medium	35 to 50
	H High	50 to 70
	V Very high	exceeding 70
	O Organic	append to classification for organic material (eg CIHO)

Note: Water Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed:



Monika Siewior
Reporting Specialist
for and on behalf of i2 Analytical Ltd

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4041

Client: Curtins

Client Address: 17-19 Whitworth Street West, Manchester,
M1 5WG

Contact: Jonathan Lord

Site Address: P500 Dam Head Drive

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

SUMMARY REPORT**SUMMARY OF CLASSIFICATION TEST RESULTS**

Tested in Accordance with:

Water Content by BS 1377-2:1990: Clause 3.2; Atterberg by BS 1377-2: 1990:
Clause 4.3 (4 Point Test), Clause 4.4 (1 Point Test) and 5; PD by BS 1377-2:
1990: Clause 8.2i2 Analytical Ltd
Unit 8 Harrowden Road
Brackmills Industrial Estate
Northampton NN4 7EB

Environmental Science

Client Reference: B077873

Job Number: 22-80490

Date Sampled: 10/08/2022

Date Received: 22/08/2022

Date Tested: 05/09/2022

Sampled By: Not Given

Test results

Laboratory Reference	Hole No.	Sample				Description	Remarks	Water Content BS 1377-2 [W] %	Water Content BS EN ISO 17892-2 [W] %	Atterberg				Density			Total Porosity# %		
		Reference	Depth Top m	Depth Base m	Type					% Passing 425um %	WL %	Wp %	Ip %	bulk Mg/m3	dry Mg/m3	PD Mg/m3			
2403865	WS01	Not Given	1.80	Not Given	D	Brown sandy CLAY	Atterberg 4 Point	23		100	35	18	17						
2403867	WS04	Not Given	2.80	Not Given	D	Brown slightly gravelly very sandy CLAY	Atterberg 4 Point	18		79	32	16	16						

Note: # Non accredited; NP - Non plastic

Comments:

Signed:

*Monika Siewior*Monika Siewior
Reporting Specialist
for and on behalf of i2 Analytical Ltd

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4041

Client: Curtins

Client Address: 17-19 Whitworth Street West, Manchester,
M1 5WG

Contact: Jonathan Lord

Site Address: P500 Dam Head Drive

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

SUMMARY REPORT

DETERMINATION OF WATER CONTENT

Tested in Accordance with: BS 1377-2: 1990: Clause 3.2

i2 Analytical Ltd
Unit 8 Harrowden Road
Brackmills Industrial Estate
Northampton NN4 7EB



Environmental Science

Client Reference: B077873

Job Number: 22-80490

Date Sampled: 10/08/2022

Date Received: 22/08/2022

Date Tested: 05/09/2022

Sampled By: Not Given

Test results

Laboratory Reference	Hole No.	Sample				Description	Remarks	WC	Sample preparation / Oven temperature at the time of testing			
		Reference	Depth Top m	Depth Base m	Type							
2403865	WS01	Not Given	1.80	Not Given	D	Brown sandy CLAY		23	Sample was quartered, oven dried at 109 °C			
2403867	WS04	Not Given	2.80	Not Given	D	Brown slightly gravelly very sandy CLAY		18	Sample was quartered, oven dried at 109 °C			

Comments:

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Signed:

Monika Siewior

Monika Siewior
Reporting Specialist
for and on behalf of i2 Analytical Ltd

TEST CERTIFICATE

DETERMINATION OF PARTICLE SIZE DISTRIBUTION

Tested in Accordance with: BS 1377-2: 1990

i2 Analytical Ltd
Unit 8 Harrowden Road
Brackmills Industrial Estate
Northampton NN4 7EB

Client: Curtins
Client Address: 17-19 Whitworth Street West, Manchester,
M1 5WG

Contact: Jonathan Lord
Site Address: P500 Dam Head Drive

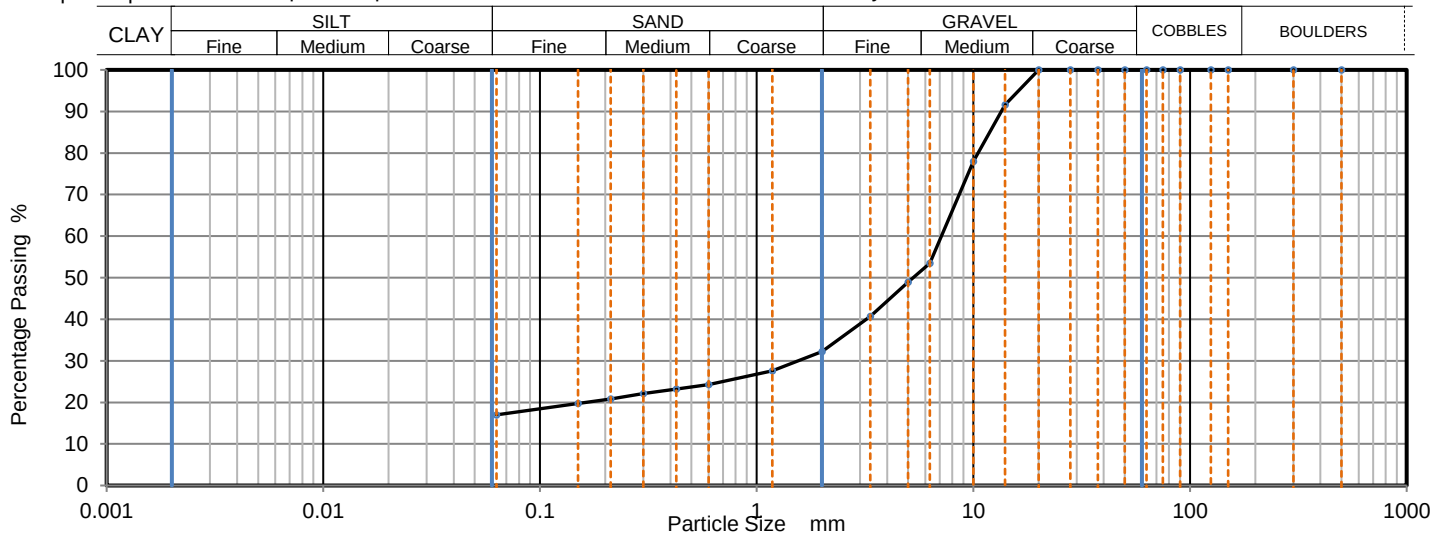
Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: B077873
Job Number: 22-80490
Date Sampled: 10/08/2022
Date Received: 22/08/2022
Date Tested: 05/09/2022
Sampled By: Not Given

Test Results:

Laboratory Reference: 2403866
Hole No.: WS01
Sample Reference: Not Given
Sample Description: Brown sandy clayey GRAVEL
Sample Preparation: Sample was quartered, oven dried at 106.0 °C and broken down by hand.

Depth Top [m]: 2.50
Depth Base [m]: Not Given
Sample Type: D



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
500	100		
300	100		
150	100		
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	92		
10	78		
6.3	54		
5	49		
3.35	41		
2	32		
1.18	28		
0.6	24		
0.425	23		
0.3	22		
0.212	21		
0.15	20		
0.063	17		

Sample Proportions	% dry mass
Very coarse	0
Gravel	68
Sand	15
Fines <0.063mm	17

Grading Analysis	
D100	mm
D60	mm
D30	mm
D10	mm
Uniformity Coefficient	> 110
Curvature Coefficient	

Uniformity Coefficient calculated in accordance with BS EN ISO 14688-2:2018

Note: Tested in Accordance with BS1377:Part 2:1990, clause 9.2

Remarks:

Signed:

Monika Siewior

Monika Siewior
Reporting Specialist
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Appendix A6 – Ground Gas Monitoring Results

Curtins

Merchant Exchange, 17-19 Whitworth Street West, Manchester, M1 5WG

Tel: 0161 236 2394

Fax: 0161 228 7902

**GAS MONITORING LOG SHEET****Project:** Dam Head Drive, P500**Date:** 24/08/2022**Job Number:** B077873**Visit:** 1**Client:** Guinness Partnership**Weather:** Overcast**Barometric State:** Steady**Ground Conditions:** Dry

Borehole Reference	Barometric Pressure mb	Flow l/hr		Methane %		Carbon Dioxide %		Oxygen %	Hydrogen Sulphide ppm	Carbon Monoxide ppm	Water Level m bgl	Borehole Base m bgl	PID	Note
		Max	SS	Max	SS	Max	SS							
WS01	1001	0.0	0.0	0.0	0.0	1.1	1.1	19.5	0	0	1.25	1.50	1.00	
WS03	1001	0.0	0.0	0.0	0.0	1.1	1.1	19.2	0	0	Dry	1.46	1.00	
MAX		0.0	0.0	0.0	0.0	1.1	1.1	19.5	0	0				

Notes**Logged by**

RBJ

*1% gas volume = 10,000 ppm**Flow rate, methane and carbon dioxide reported as 'maximum' (max) and 'steady state' (SS) readings.**All other gases recorded at 'steady state' unless otherwise stated*

Curtins

Merchant Exchange, 17-19 Whitworth Street West, Manchester, M1 5WG

Tel: 0161 236 2394

Fax: 0161 228 7902

**GAS MONITORING LOG SHEET****Project:** Dam Head Drive, P500 **Date:** 01/09/2022**Job Number:** B077873 **Visit:** 2**Client:** Guinness Partnership **Weather:** Sunny**Barometric State:** Steady **Ground Conditions:** Dry

Borehole Reference	Barometric Pressure mb	Flow l/hr		Methane %		Carbon Dioxide %		Oxygen %	Hydrogen Sulphide ppm	Carbon Monoxide ppm	Water Level m bgl	Borehole Base m bgl	PID	Note
		Max	SS	Max	SS	Max	SS							
WS01	1010	0.0	0.0	0.0	0.0	1.4	1.4	18.20	0	0	1.4	1.44	1.00	
WS03	1010	0.0	0.0	0.0	0.0	0.7	0.7	20.00	0	0	Dry	1.46	1.00	
MAX		0.0	0.0	0.0	0.0	1.4	1.4	20.0	0	0				

Notes**Logged by**

JP

*1% gas volume = 10,000 ppm**Flow rate, methane and carbon dioxide reported as 'maximum' (max) and 'steady state' (SS) readings.**All other gases recorded at 'steady state' unless otherwise stated*

Curtins

Merchant Exchange, 17-19 Whitworth Street West, Manchester, M1 5WG

Tel: 0161 236 2394

Fax: 0161 228 7902

**GAS MONITORING LOG SHEET****Project:** Dam Head Drive, P500**Date:** 21/09/22**Job Number:** B077873**Visit:** 3**Client:** Guinness Partnership**Weather:** Sunny**Barometric State:** Steady**Ground Conditions:** Dry

Borehole Reference	Barometric Pressure mb	Flow		Methane		Carbon Dioxide		Oxygen %	Hydrogen Sulphide ppm	Carbon Monoxide ppm	Water Level m bgl	Borehole Base m bgl	PID	Note
		l/hr		%		%								
		Max	SS	Max	SS	Max	SS							
WS01	1012	0.0	0.0	0.0	0.0	1.5	1.5	16.4	0	0	1.24	1.35	1.00	
WS03	1012	0.0	0.0	0.0	0.0	0.9	0.9	19.1	0	0	Dry	1.46	1.00	
MAX		0.0	0.0	0.0	0.0	1.5	1.5	19.1	0	0				

Notes**Logged by**

JP

*1% gas volume = 10,000 ppm**Flow rate, methane and carbon dioxide reported as 'maximum' (max) and 'steady state' (SS) readings.**All other gases recorded at 'steady state' unless otherwise stated*

Curtins

Merchant Exchange, 17-19 Whitworth Street West, Manchester, M1 5WG

Tel: 0161 236 2394

Fax: 0161 228 7902

**GAS MONITORING LOG SHEET**

Project: Dam Head Drive, P500 **Date:** 28/09/22
Job Number: B077873 **Visit:** 4
Client: Guinness Partnership **Weather:** Sunny
Barometric State: Falling **Ground Conditions:** Wet

Borehole Reference	Barometric Pressure mb	Flow l/hr		Methane %		Carbon Dioxide %		Oxygen %	Hydrogen Sulphide ppm	Carbon Monoxide ppm	Water Level m bgl	Borehole Base m bgl	PID	Note
		Max	SS	Max	SS	Max	SS							
WS01	1001	0.0	0.0	0.0	0.0	1.5	1.5	16.2	0	0	1.23	1.35	1.00	1
WS03	1001	2.0	0.0	0.0	0.0	0.7	0.7	19.9	0	0	Dry	1.46	1.00	
MAX		2.0	0.0	0.0	0.0	1.5	1.5	19.9	0	0				

Notes

1 Steady state gas flow rate reached after 2 seconds.

Logged by

TW

*1% gas volume = 10,000 ppm**Flow rate, methane and carbon dioxide reported as 'maximum' (max) and 'steady state' (SS) readings.**All other gases recorded at 'steady state' unless otherwise stated*

Curtins

Merchant Exchange, 17-19 Whitworth Street West, Manchester, M1 5WG

Tel: 0161 236 2394

Fax: 0161 228 7902

**GAS MONITORING LOG SHEET****Project:** Dam Head Drive, P500**Date:** 14/10/22**Job Number:** B077873**Visit:** 5**Client:** Guinness Partnership**Weather:** Sunny**Barometric State:** Falling**Ground Conditions:** Damp

Borehole Reference	Barometric Pressure mb	Flow l/hr		Methane %		Carbon Dioxide %		Oxygen %	Hydrogen Sulphide ppm	Carbon Monoxide ppm	Water Level m bgl	Borehole Base m bgl	Note
		Max	SS	Max	SS	Max	SS						
WS01	995	0.0	0.0	0.0	0.0	2.2	2.2	16.6	0	0	1.19	1.37	
WS03	996	0.0	0.0	0.0	0.0	2.6	2.6	15.1	0	0	Dry	1.47	
MAX		0.0	0.0	0.0	0.0	2.6	2.6	16.6	0	0			

Notes**Logged by** JP*1% gas volume = 10,000 ppm**Flow rate, methane and carbon dioxide reported as 'maximum' (max) and 'steady state' (SS) readings.**All other gases recorded at 'steady state' unless otherwise stated*

Curtins

Merchant Exchange, 17-19 Whitworth Street West, Manchester, M1 5WG

Tel: 0161 236 2394

Fax: 0161 228 7902

**GAS MONITORING LOG SHEET****Project:** Dam Head Drive, P500**Date:** 28/10/22**Job Number:** B077873**Visit:** 6**Client:** Guinness Partnership**Weather:** Sunny and overcast**Barometric State:** Steady**Ground Conditions:** Damp

Borehole Reference	Barometric Pressure mb	Flow l/hr		Methane %		Carbon Dioxide %		Oxygen %	Hydrogen Sulphide ppm	Carbon Monoxide ppm	Water Level m bgl	Borehole Base m bgl	Note
		Max	SS	Max	SS	Max	SS						
WS01	1000	0.0	0.0	0.0	0.0	0.9	0.9	17.8	0	0	1.19	1.36	
WS03	1000	0.0	0.0	0.0	0.0	0.8	0.8	19.6	0	0	Dry	1.47	
MAX		0.0	0.0	0.0	0.0	0.9	0.9	19.6	0	0			

Notes**Logged by**

JP

*1% gas volume = 10,000 ppm**Flow rate, methane and carbon dioxide reported as 'maximum' (max) and 'steady state' (SS) readings.**All other gases recorded at 'steady state' unless otherwise stated*