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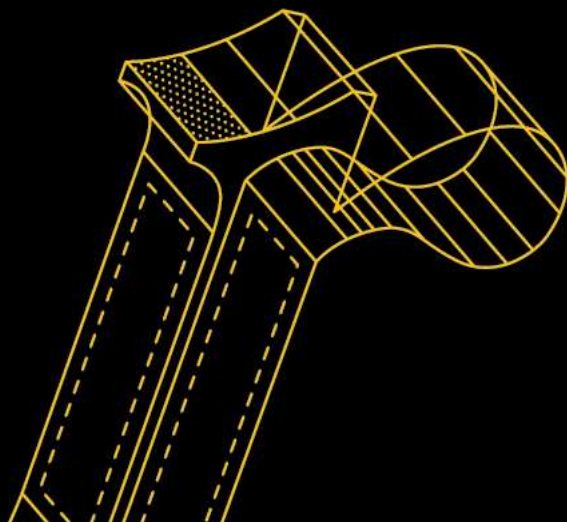
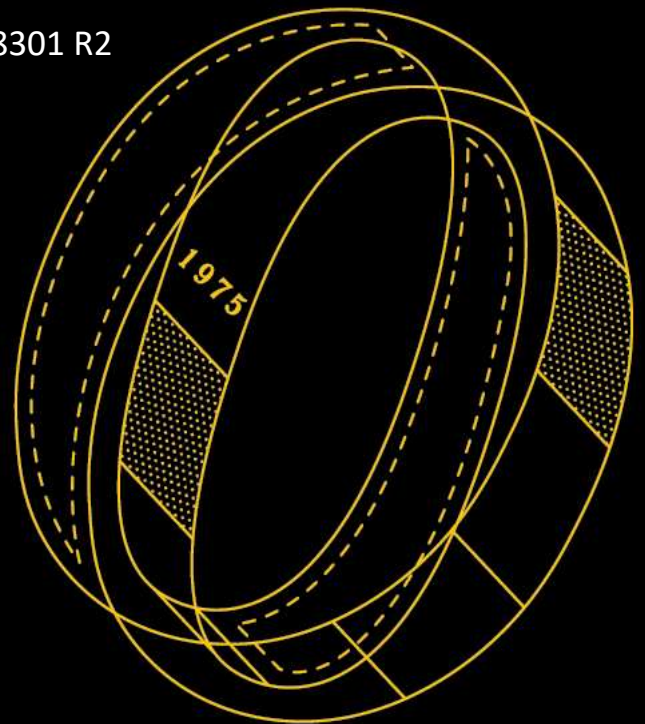
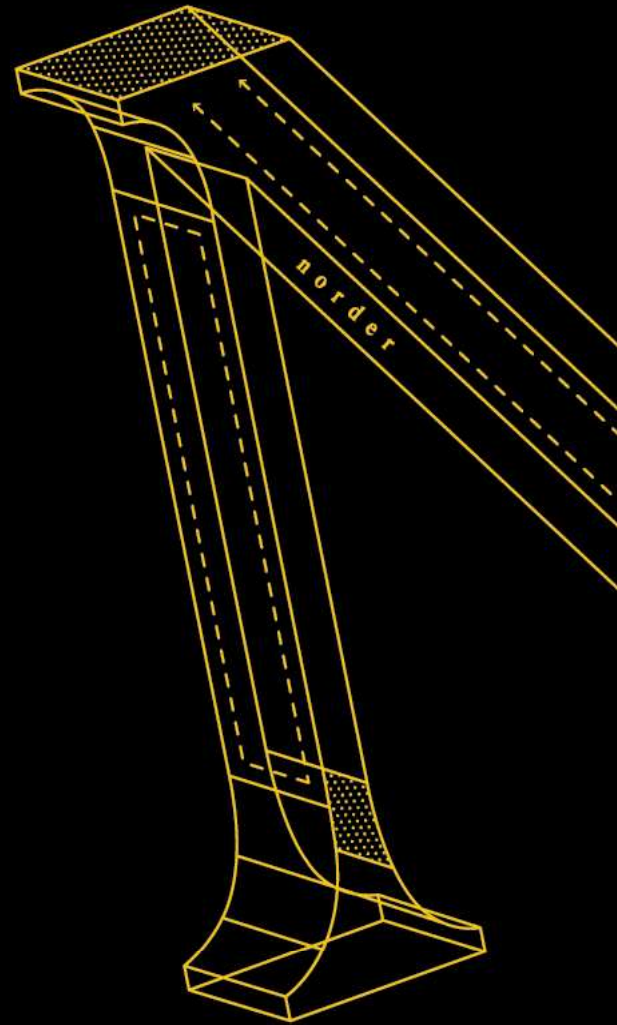
Engineers. Architects. Innovators.

Client: Heineken

Project Title: Finished Goods Warehouse
Heineken, Manchester

Document Title: Flood Risk Assessment and
Drainage Design Report

Document No.: 8901-NDA-XX-XX-RP-D-8301 R2



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Appendix A – Site Location Plan

Appendix B – Existing Topographical Survey

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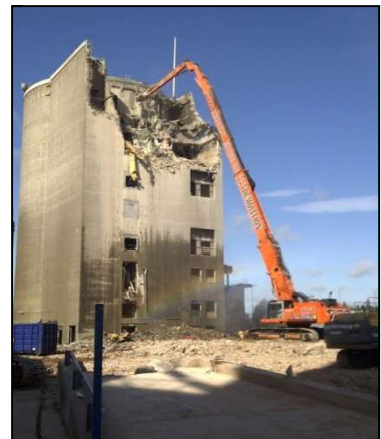
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1 Introduction

This Flood Risk Assessment (FRA) has been produced in accordance with the 2021 National Planning Policy Framework (NPPF) and the associated Technical Guidance (TG), both contained within the 2021 Planning Practice Guidance (PPG).

The FRA has been produced on behalf of Heineken in respect of the proposed new Finished Goods Warehouse at their existing site in Manchester. The building is intended to be a temporary structure.

The comments and opinions contained in this report, including the conclusions and recommendations, are based on the following information:

- (i) Existing OS plans.
- (ii) Topographical survey of existing levels.
- (iii) Government and Environment Agency flood mapping.
- (iv) United Utilities sewer record plans.

This report is prepared solely for the use of Heineken and their advisers.

2 Site Information

2.1 Existing Site

The proposed development is located within the existing Heineken Brewery site off Denmark Road, Manchester, M15 6LD. Grid reference 383945 E, 396176 N. A site location plan is attached under Appendix A.

The Heineken brewery is bound by Princess Road to the west with a supermarket and retail park beyond. To the north, the site is bound by Greenheys Lane with a residential area beyond. Charles Halle Road and Denmark Road run adjacent to the eastern boundary with residential housing beyond, along with a primary school and commercial unit. To the south the site is bound by Moss Lane East, with a number of churches and social clubs and commercial units beyond.

The Bridgewater Canal runs approximately 1.75Km to the north west of the site and the River Irwell runs approximately 1.85Km to the north west of the site.

Both vehicular and pedestrian access can be taken into the site via Denmark Road.

The Heineken site is approximately 7.5Ha in area and accommodates the main factory and production buildings along with the main office building. The remainder of the site is used as car parking, lorry parking and storage. The site is covered in impermeable surfacing, being a mixture of concrete and asphalt pavements, with the exception of a relatively small area of landscaping at the site entrance and to some boundaries.

The proposed Finished Goods Warehouse occupies an area of approximately 2655m² and is located to the north side of the main Heineken building. The area is currently occupied by a lorry yard which is positively drained to the surface water drainage network. The existing levels in the proposed development area have a general fall from the north, having an approximate level of 33.50m AOD, to the south having a level of approximately 33.30m AOD.

An existing topographical survey plan showing development areas and existing ground levels, is attached under **Appendix B**.

2.2 Proposed Development

The proposed development consists of a new single storey Finished Goods Warehouse building, occupying an area of 2,655m². The building is intended to be a temporary structure. The access road and yard areas are to remain in-situ and both vehicular and pedestrian access to the site will continue to be taken from Denmark Road. Both vehicle and pedestrian movements around the site will remain as present.

The development will remain as fully impermeable surfacing. Proposed site layout plans are attached under **Appendix C**.

2.3 Flood Zone Classification

Flood mapping for Rivers and Seas has been obtained from the Environment Agency website and a copy of the Flood Map is attached under **Appendix D**.

The Environment Agency Flood Map for Planning shows that the majority of the proposed development area is located within Flood Zone 3 which has an annual probability of flooding from rivers of 1 in 100 or greater (1% or greater). The south west corner of the proposed development area is located within Flood Zone 2 which has an annual probability of flooding from rivers of between 1 in 100 and 1 in 1000 (0.1% and 1.0%).

There are no allocated flood storage areas within the vicinity of the site area.

Product 4 data has also been supplied by the Environment Agency and this confirms the above flood zones.

The proposed Finished Goods Warehouse is classed as general industry and would therefore be classed as a Less Vulnerable development in accordance with Table 2 : Flood Risk Vulnerability Classification, of the PPG, associated with the NPPF. Table 3 : Flood Risk Vulnerability and Flood Zone Compatibility, of the PPG, associated with the NPPF shows that, an exceptions test is not required for a less vulnerable classification Flood Zone 3a and therefore the proposed development is appropriate within Flood Zone 3a.

3 Possible Sources of Flooding

3.1 Fluvial (Rivers)

Fluvial sources of flooding include the inundation of floodplains from rivers and watercourses, inundation of areas outside the floodplain due to the influence of bridges, embankments and other features which artificially raise water levels, overtopping or breaching of defences, blockages of culverts, flood channels or flood corridors.

The Bridgewater Canal runs approximately 1.75Km to the north west of the site and the River Irwell runs approximately 1.85Km to the north west of the site.

The Environment Agency Flood Map for Planning shows that the majority of the proposed development area is located within Flood Zone 3 which has an annual probability of flooding from rivers of 1 in 100 or greater (1% or greater). The south west corner of the proposed development area is located within Flood Zone 2 which has an annual probability of flooding from rivers of between 1 in 100 and 1 in 1000 (0.1% and 1.0%). There are no allocated flood storage areas within the vicinity of the site area.

Product 4 data has also been supplied by the Environment Agency and this confirms the above flood zones. This data also provides modelled data in the vicinity of the Heineken site, where node location 3 sits closest to the development area. Node 3 shows a flood level of 33.47m AOD for a 1 in 100 year storm event, a flood level of 33.75m AOD for a 1 in 100 year storm event plus 30% climate change and a flood level of 33.79m AOD for a 1 in 100 year storm event plus 35% climate change.

The proposed Finished Goods Warehouse has a finished floor level of 33.55m AOD, to match the adjacent main factory building to facilitate forklift movements between the two buildings. The finished floor level is therefore slightly higher than the modelled 1 in 100 year flood level of 33.47mAOD, indicating that the warehouse will not flood in this storm event.

The warehouse is however considered to be at risk of flooding from an event greater than 1%. As such, it is recommended that flood resilient measures are incorporated into the detailed design process. Refer to Section 4 for floor resilient measure options.

The Finished Goods Warehouse will replace existing concrete service yard and the proposals provide a low percentage amendment to the current site arrangement and it is considered that this will have a negligible effect on the site and surrounding areas, during a flood event.

A copy of the Flood Mapping and Environment Agency correspondence is attached under **Appendix D**.

3.2 Tidal

The area and associated Rivers are not subject to tidal events.

3.3 Surface Water

Flooding from surface water can include sheet run off from adjacent land and surcharged sewers being combined, foul or surface water sewers.

The Government long term flood risk information shows that the whole of the development is located within an area having a low to medium risk of flooding due to surface water. A low risk of flooding means that it has a chance of flooding of between 0.1% and 1% each year and a medium risk of flooding means that it has a chance of flooding between 1% and 3.33% each year. The medium risk areas match the drainage gully lines serving the current lorry yard.

It is noted that the government website providing this information states that it is difficult to predict flooding from surface water as rainfall location and volume are difficult to forecast. In addition, local features can greatly affect the chance and severity of flooding.

The Finished Goods Warehouse will replace existing positively drained concrete service yard and the proposals will have a negligible effect on the site and surrounding areas due to risk of flooding from surface water.

The development area is considered to be at a potential risk of flooding from surface water of between 1% and 3.3%, however, the Finished Goods Warehouse is considered as a less vulnerable development and as such, it is recommended that flood resilient measures are incorporated into the detailed design process. Refer to Section 4 for floor resilient measure options.

A copy of the long term flood risk information is attached under **Appendix D**.

3.4 Ground Water

Flooding from groundwater can include the water table rising above ground level after prolonged rainfall, remote from a watercourse, low lying areas underlain by permeable rock (aquifers), seepage directly into properties, and groundwater recovery after pumping has ceased from mining or industry.

There is no evidence of groundwater issues on the site.

3.5 Infrastructure Failure

Flooding from the failure of reservoirs, canals, industrial processes, burst water mains, blocked sewers or failed pumping stations.

United Utilities sewer records show a 2450mm brick combined sewer and a 750mm by 1020mm brick combined sewer run in a westerly direction within the carriageway of Moss Lane East. These sewers are shown to have a depth to invert of approximately 8m and 3.5m respectively.

A 620mm by 860mm brick culvert combined sewer runs in a southerly direction within Princess Road. The sewer records do not provide a depth for the sewer. The sewer connects onto the 2450mm combined sewer indicated above to continue to run in a westerly direction away from the site. A 610mm by 930mm combined sewer runs in a southerly direction within the site and under the site service road to the south. This sewer is shown to have a depth to invert of approximately 3m and runs into the 750mm by 1020mm brick combined sewer indicated above. This sewer also serves the adjacent residential development located to the east of the site.

A 680mm brick combined sewer, shown to increase to a 620mm by 770mm sewer, runs in a northerly direction within the carriageway of Denmark Road, commencing at the site entrance. This sewer is shown to have a depth to invert of approximately 2.25m. This sewer is shown to have a 750mm combined spur running into the site which stops at a dead end.

A 300mm diameter, 560mm by 710mm, and 600mm diameter combined sewers run into the site from the Police Station to its eastern boundary. These sewers are shown to connect to each other and run in a westerly direction, but are shown to end within the site boundary. A 1805mm by 3300mm surface water sewer is shown to run in a southerly direction adjacent to the western side of Denmark Road. The sewer is shown to have a depth to invert of approximately 4m and is shown to end at the site entrance.

As these drainage systems are adopted and owned by United Utilities, it is considered that these systems are likely to be adequately maintained and monitored.

A copy of the sewer records are attached under **Appendix E**.

Two surface water drains, commencing as 150mm diameter and increasing to 300mm diameter, run in an easterly direction within the existing lorry yard picking up the yard drainage. These two systems connect together to run in a southerly direction down the eastern side of the site, then run west to connect onto the United Utilities combined sewer network. These two surface water drains will need to be diverted around the proposed Finished Good Warehouse.

As these private drainage systems are owned and maintained by the brewery as part of their site management system, they are considered to be adequately maintained.

As such, the site is considered not to be at significant risk from infrastructure failure or flooding from existing sewers.

The Government flood mapping for reservoirs shows that the whole site will potentially be affected due to the failure of reservoirs when river levels are normal. It is noted by the Environment Agency that the failure of a reservoir is very unlikely and

that the maps provided reflect a credible worst case scenario for the purposes of emergency planning.

It is therefore recommended that a Flood Emergency Plan is produced for this scenario. Refer to Section 4 for flood resilient measure options.

A copy of the Government flood mapping for reservoirs is attached under **Appendix E**.

3.6 Development Drainage

An increase in formally drained impermeable area could increase flood risk to a wider catchment.

The existing development area which is to accommodate the proposed Finished Goods Warehouse is covered by impermeable concrete surfacing, which is formally drained and discharged into the surrounding site drainage network. Therefore there is no increase in formally drained impermeable area and no increase in flood risk to a wider catchment.

The surface water discharging from the Finished Goods Warehouse will need to be designed in accordance with the NPPF Non-statutory Technical Standards for sustainable drainage systems.

4 Flood Resilience Measures

The provision of flood resilience measures are recommended to be considered in the design and implementation of the Finished Goods Warehouse, along with the production of an emergency plan to facilitate the safe movement of personnel should flooding occur due to the risk of flooding from rivers and the failure of a reservoir.

Flood resilience measures would include:

- i) Production and implementation of a Flood Emergency Plan in the event of flooding from rivers.
- ii) Production and implementation of a Flood Emergency Plan in the event of flooding from reservoirs.
- iii) Sockets, meters and other electrical items to be installed at a height of 1.2m above FFL.
- iv) Flooring material to be considered, with use of concrete and tiling, sacrificial flooring, or similar, to allow easier cleaning and return to use.
- v) Consideration for internal wall units, furniture etc to be provided in water resistant materials or as sacrificial items.
- vi) Flood barrier provision independent or integral, at effected doorways.

5 SW Drainage Design

5.1 Drainage Hierarchy and Principles

The NPPF Non-statutory Technical Standards for sustainable drainage systems, and Part H of the Building Regulations, requires the following order of priority to be considered when designing the surface water drainage for a development :

- a) an adequate soakaway or some other adequate infiltration system, or where that is not reasonably practicable,
- b) a watercourse, or where that is not reasonably practicable,
- c) a surface water sewer, highway drain or another drainage system,
- d) a combined sewer.

Discharging Via Infiltration

A Geotechnical and Geo Environmental Investigation Report, reference MJH/J1834/D3/1 and dated November 2019, has been produced by Emcus. The report shows that the site consists of made ground, underlain by clay.

Emcus have subsequently produced a letter reference J1834_LTR_03, confirming that the cohesive natural strata predominantly encountered immediately beneath the site will not be sufficiently permeable to receive water from soakaways. In addition, it is recommended that water from soakaways, permeable pavements etc, is not discharged into the made ground beneath the site.

On this basis, discharge of surface water via infiltration is not considered a viable option for the discharge of the surface water. A copy of the Emcus letter is attached under **Appendix F**.

Discharge to Watercourse

There are no watercourses within the vicinity of the site boundary therefore this is not considered a viable option for the discharge of surface water.

Discharge to Sewer

Due to the surface water discharge from the site not being feasible via infiltration or to a watercourse, it is proposed to discharge the surface water into the existing site surface water drainage system, running in the vicinity of the proposed warehouse, which ultimately discharges into the United Utilities combined sewer network.

5.2 Potential for Sustainable Drainage Systems

A review of the potential SuDs techniques and applications within the proposed development has been undertaken to determine whether each technique could be viable for the site. The items considered potentially viable for the site are shaded green.

Green Roof	The building has a pitched roof and is therefore not suitable for a green roof solution.
Infiltration systems	Infiltration is not feasible due to the site being underlain by clay.
Proprietary Treatment Systems	There are no new proposed access road or car parking areas being constructed on the site.
Filter Strips, Filter Drains and Swales	There is minimal space available on the site to incorporate this feature.
Bio-Retention Systems and Trees	There is limited space to deliver a suitable system and along with high maintenance costs this system is not favourable.
Pervious pavements	There are no new proposed access road or car parking areas being constructed on the site.
Attenuation Storage Tanks	Attenuation storage can be provided under the existing service yard area

Detention Basins	There is insufficient space available on the site to incorporate this feature.
Ponds and Wetlands	There is insufficient space available on the site to incorporate this feature.

5.3 Drainage Design Criteria

The DEFRA Non-statutory Technical Standards for sustainable drainage systems, in conjunction with the NPPF, requires that for developments that were previously developed, the peak run off rate from the development for the 1 in 1 year rainfall event and the 1 in 100 year rainfall event should be as close as reasonably practical to the green field runoff for the same rainfall event, but should never exceed the rate of discharge from the development prior to redevelopment for that event.

The site is located within the Conurbation Core Critical Drainage Area and any development within this area is identified in the Manchester Trafford and Salford Strategic Flood Risk Assessment as requiring a reduction in surface water run off, of at least 50% compared to existing rates.

The area of the Finished Goods Warehouse is 2,655m² in area, all of which is located over an existing impermeable concrete service yard. The Finished Goods Warehouse is a temporary building.

The greenfield discharge from the Finished Goods Warehouse area has been calculated using the Micro Drainage Rural Runoff Calculator. The discharge from a 1 in 1 year storm event is calculated as 2.7 l/s and the discharge from a 1 in 100 year storm event is calculated as 4.9 l/s. Qbar is shown as 1.1 l/s rural, 3.2 l/s urban.

The site however is a brownfield site containing existing hard standing under the Finished Goods Warehouse area, measuring 2,655m². The discharge rate has been calculated using the Micro Drainage network design package. The discharge from a 1 in 1 year storm event is calculated as 33.2 l/s and the discharge from a 1 in 100 year storm event is calculated as 97.9 l/s.

As the development is located within the Conurbation Core Critical Drainage Area, a reduction in surface water run off, of at least 50% compared to existing rates. As the proposed Finished Goods Warehouse is a temporary structure, a 50% reduction has been applied which results in a proposed surface water discharge strategy with a discharge restricted to 16 l/s and attenuated for a 1 in 100 year storm plus a 40% allowance for climate change.

This provides 50% betterment to the 1 in 1 year discharge from the development area, and an 83% betterment to the 1 in 100 year discharge from the development area. A copy of the Micro Drainage calculations are attached under **Appendix G**.

5.4 Attenuation Volume

The surface water discharge from the proposed building extension is to be restricted to 16 l/s. The excess surface water generated by the proposed flow restriction of 5l/s is to be attenuated to accommodate a 1 in 100 year storm plus a 40% allowance for climate change.

The surface water drainage has been modelled using the Micro Drainage computer programme Network Design. The total contributing area of 2,655m² has been included in the model. A vortex control flow restriction has been provided to limit the flow to a maximum of 16 l/s with surface water attenuation provided in a crate tank having a storage volume of 66m³. This has been simulated and is shown to accommodate a 1 in 100 year storm event plus 40% allowance for climate change and shows the provision is adequate to accommodate the design storm event.

A copy of the Micro Drainage calculations are attached under **Appendix G**. A copy of the Drainage Layout Drawing and Details are attached under **Appendix H**.

5.5 Flow paths and Flood Exceedance Routes

The drainage has been designed to accommodate the surface water generated from a 1 in 100 year storm plus a 40% allowance for climate change. It is unlikely that this design storm event is exceeded and the system floods under these circumstances.

If the proposed drainage facilities were to fail or their capacity exceeded, the excess surface water would flow towards the south of the yard, within the site boundary, thereby maintaining existing flow paths and flood exceedance routes. Flood exceedance routes are shown on the Drainage Layout Drawing, attached under **Appendix H**.

5.6 Off Site Works and Consents

The proposals will be re-using the existing on site drainage network which ultimately connects onto the United Utilities sewer network, therefore no off site construction works will be required.

Although there will be an overall reduction of surface water being discharged into the United Utilities sewer network via the existing connection, a S106 application is to be submitted for the proposed indirect connection onto the United Utilities sewer network.

5.7 Management and Maintenance

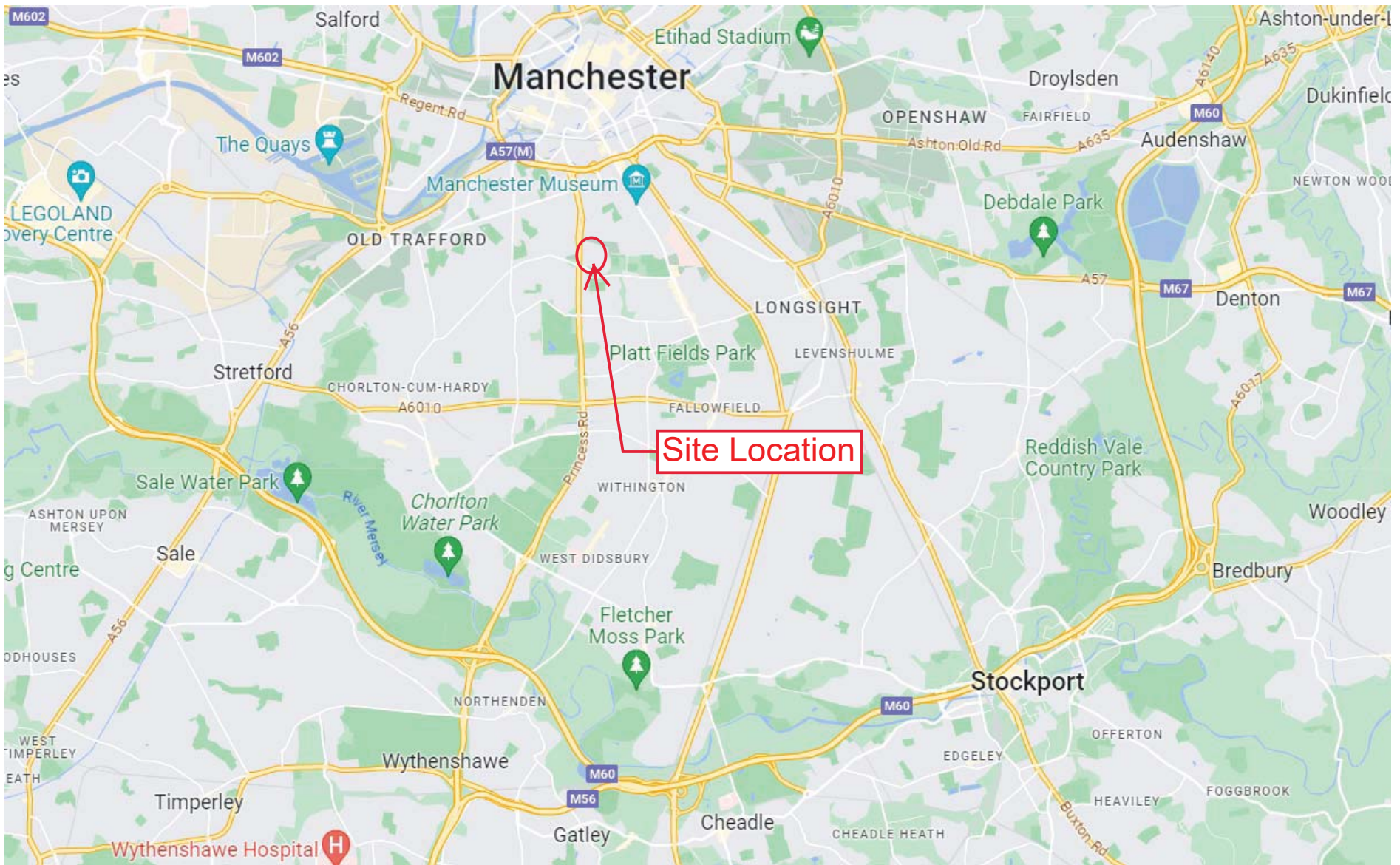
The management and maintenance regime for the drainage systems will be undertaken by the site owner/occupier under a Maintenance and Management Plan.

The Maintenance and Management Plan is provided as a separate document.

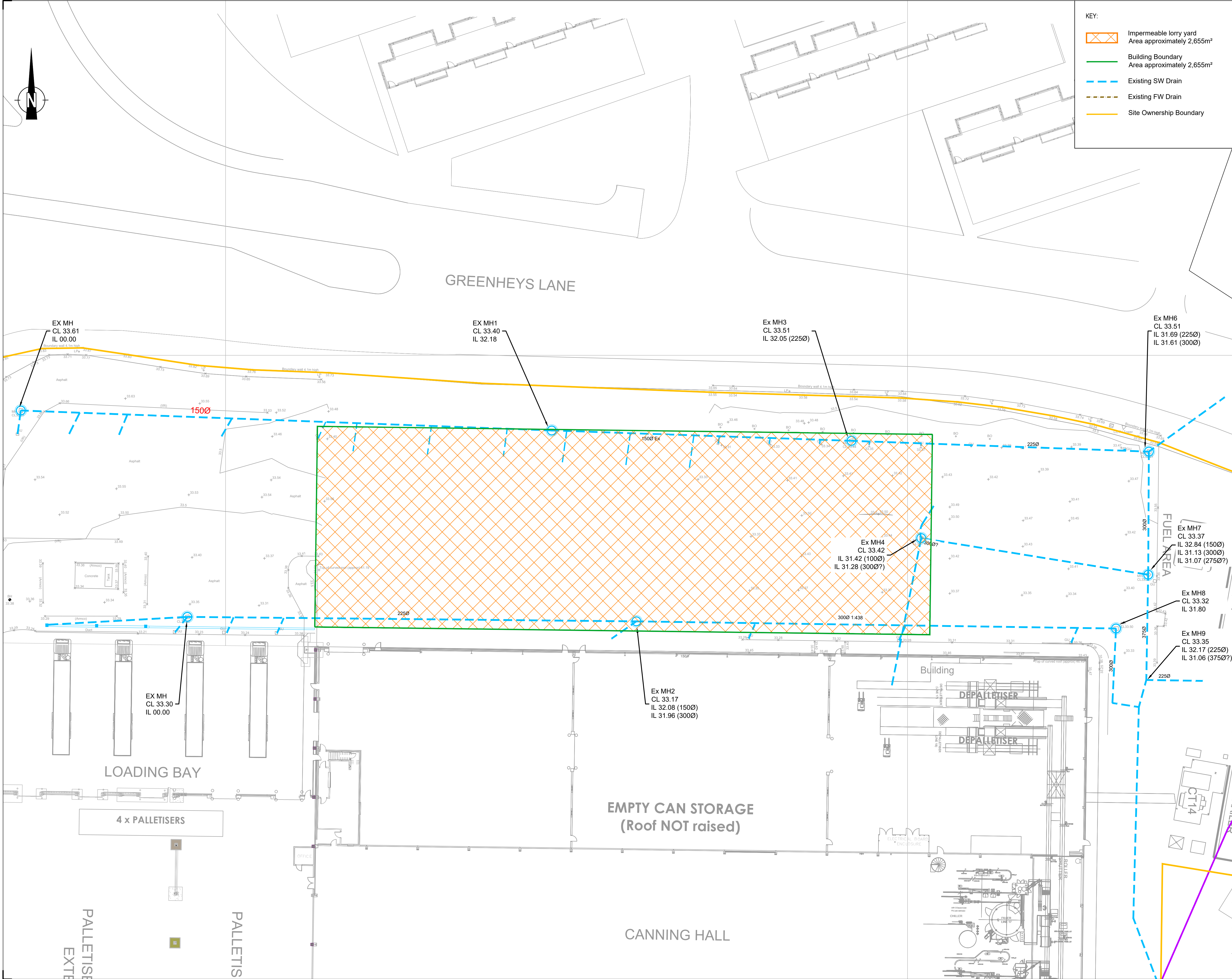
Appendix A – Site Location Plan

Finished Goods Warehouse Heineken Brewery, Manchester

Site Location Plan



Appendix B – Existing Topographical Survey



General Notes:

1. The drawings, design and all information contained therein are the sole copyright of Norder Design Associates (NDA) and reproduction in any form is forbidden unless permission is obtained in writing.
2. This drawing is to be read in conjunction with all other relevant Architects, Engineers and specialists drawings, details and specifications.
3. Conflicting information shown on the NDA drawings or discrepancies between the information given by NDA and that provided by others must be referred to NDA before work proceeds.
4. Dimensions must not be scaled from this drawing.
5. Dimensions marked 'dimension to be site checked' are subject to confirmation by site measurement before construction proceeds.
6. Subcontractor surveys, such as for the purposes of steelwork installation, take precedence over 'site dimensions' indicated on this drawing. All surveys should ensure an accurate fit of components.
7. Temporary works associated with the construction of the project shall be the responsibility of the Principal Contractor.
8. Where any temporary works or scaffolding are required the Principal Contractor shall appoint a competent and suitably experienced Temporary Works Coordinator (TWC). Refer to BS 5975 for guidance on temporary works and the role of a TWC.
9. Calculations and drawings of all Contractor designed structural items associated with NDA's design responsibilities shall be submitted to the Engineer for comment at least 10 days prior to manufacture. Any comments made by NDA do not absolve the Contractor of his design responsibilities.
10. Contractor to refer to the Health and Safety File for identification of hazards, including the existence of underground and overhead utilities.
11. The contractor is to work within any planning restriction including working hours, noise levels etc.
12. Design and drawing information is based on the MSE topographical survey drawing number NDA3789-1 Heineken Manchester. NDA does not accept any liability for any inaccuracies this information may contain.

KEY:

- Impermeable lorry yard
Area approximately 2,655m²
- Building Boundary
Area approximately 2,655m²
- Existing SW Drain
- Existing FW Drain
- Site Ownership Boundary

MANCHESTER BREWERY
FINISHED GOODS WAREHOUSE

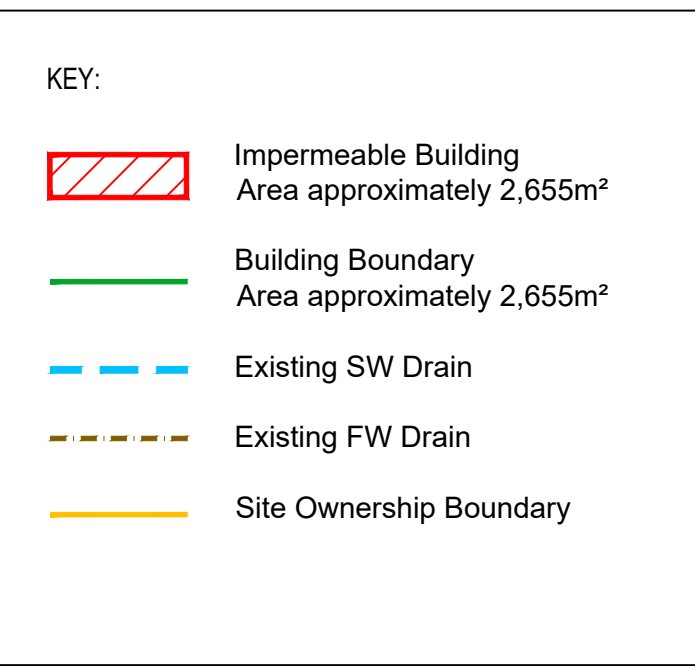
EXISTING TOPOGRAPHICAL SURVEY
AND HARD STANDING AREAS

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SCALE: 1 : 250
SHEET SIZE: A1
SUITABILITY: S2 - Suitable for Information
DRAWING NO.: 8901-NDA-XX-XX-DR-C-1011
NDA REF: 8901
REV: P01

Appendix C – Proposed Development Layout



P01	23/06/23	For information	AF	ML
REV	DATE	AMENDMENT	DRW	CHK
CLIENT				



PROJECT

Manchester Brewery
Finished Goods Warehouse

DRAWING TITLE

Proposed Development Layout
and Hard Standing Area

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SCALE	SHEET SIZE	NDA REF
Scale 1 : 250	A1	8901

SUITABILITY

S2 - Suitable for Information

DRAWING NO.	REV
8901-NDA-XX-XX-DR-C-1012	P01

Appendix D – Flood Mapping Information

Flood map for planning

Your reference
Heineken, Man

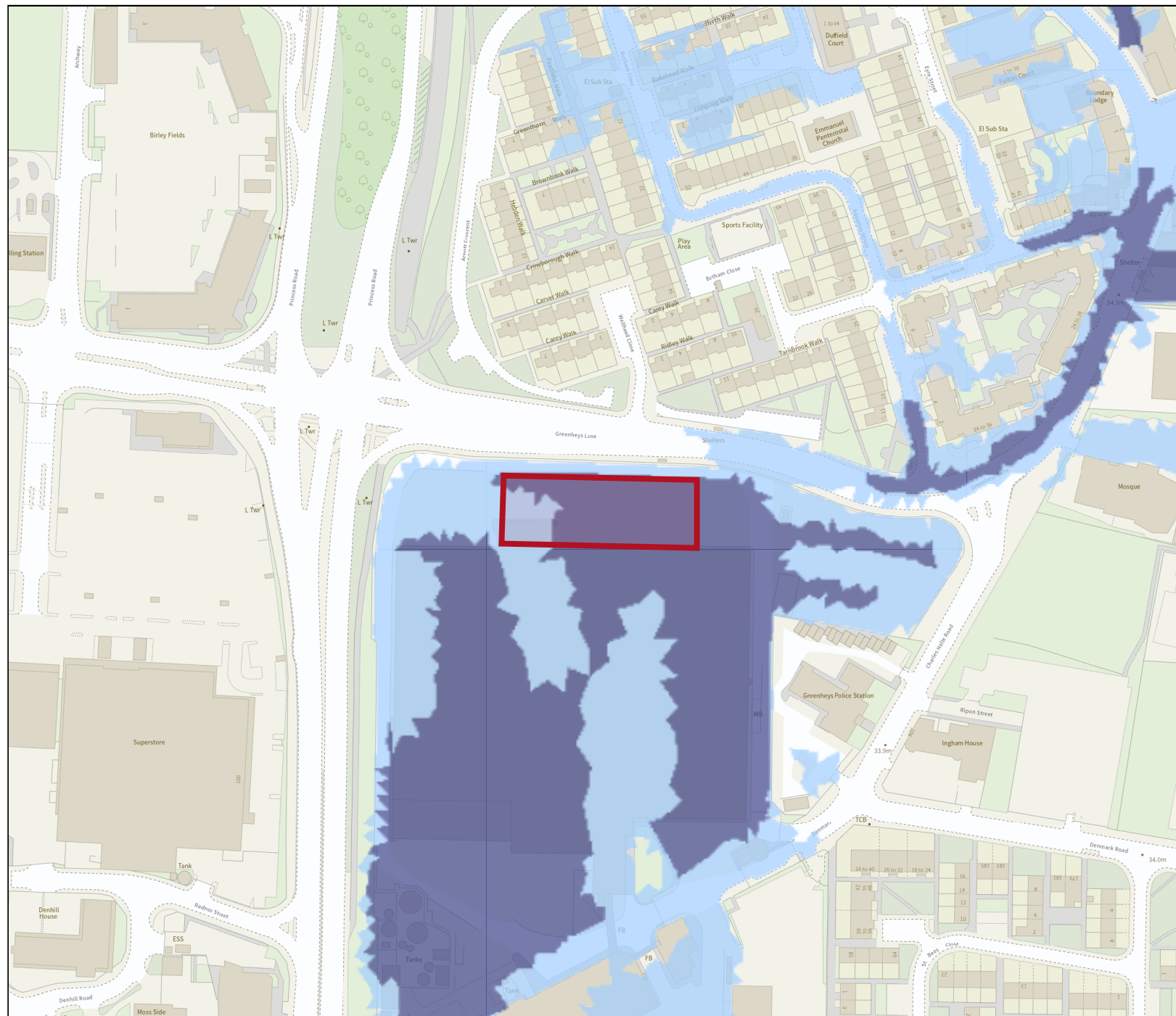
Location (easting/northing)
383957/396173

Scale
1:2500

Created
10 May 2023 9:48

-  Selected area
-  Flood zone 3
-  Flood zone 2
-  Flood zone 1
-  Flood defence
-  Main river
-  Water storage area

0 20 40 60m



Flood risk assessment data

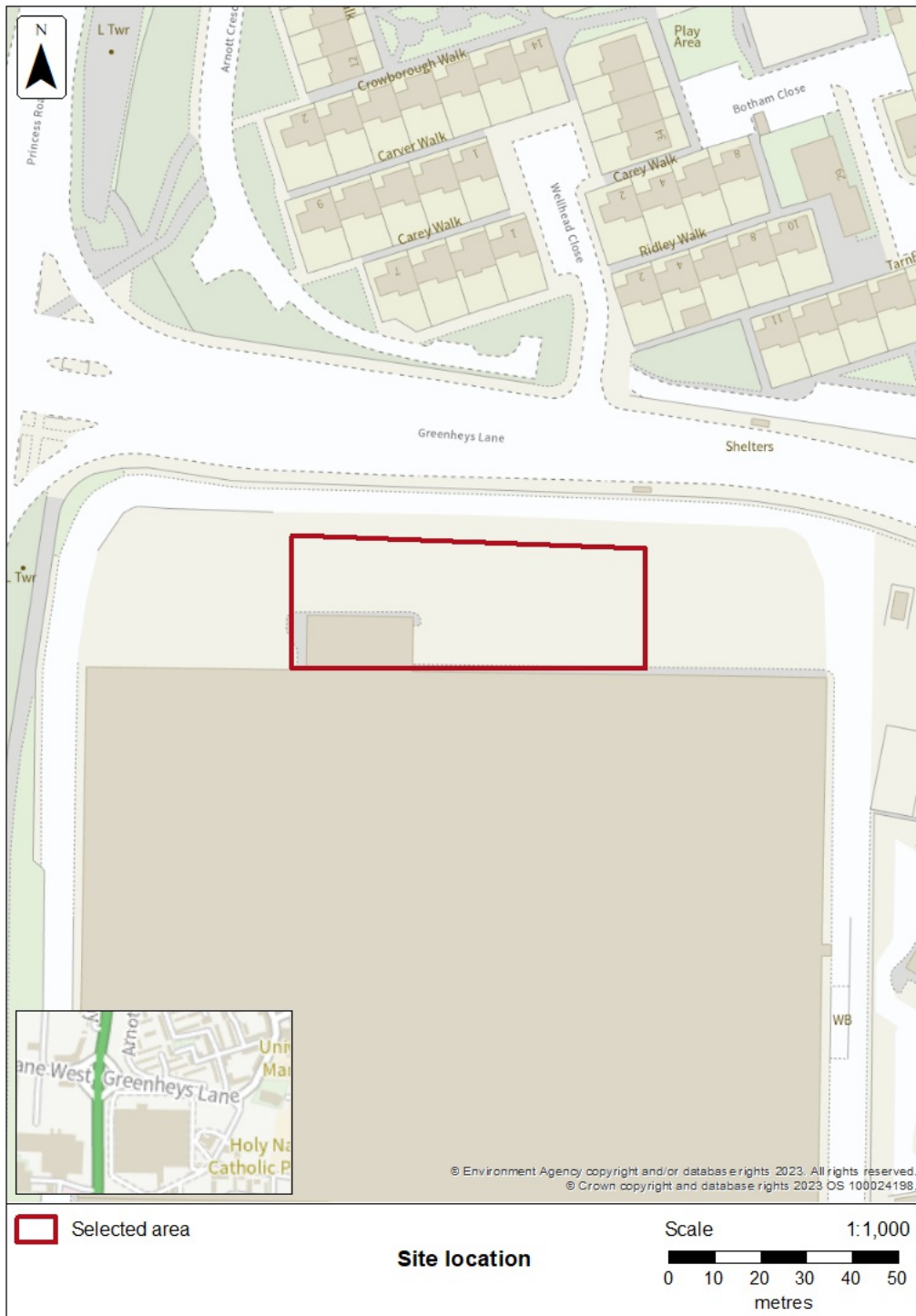
Location of site: 383947 / 396171 (shown as easting and northing coordinates)

Document created on: 30 August 2023

This information was previously known as a product 4.

Customer reference number: U1CDCCYUFGHP

Map showing the location that flood risk assessment data has been requested for.



How to use this information

You can use this information as part of a flood risk assessment for a planning application. To do this, you should include it in the appendix of your flood risk assessment.

We recommend that you work with a flood risk consultant to get your flood risk assessment.

Included in this document

In this document you'll find:

- how to find information about surface water and other sources of flooding
- information on the models used
- definitions for the terminology used throughout
- flood map for planning (rivers and the sea)
- modelled data
- climate change modelled data
- information about strategic flood risk assessments
- information about this data
- information about flood risk activity permits
- help and advice

Information that's unavailable

This document **does not** contain:

- historic flooding
- flood defences and attributes

We do not have historic flooding data for this location.

Please note that:

- flooding may have occurred that we do not have records for
- flooding can come from a range of different sources
- we can only supply flood risk data relating to flooding from rivers or the sea

You can contact your Lead Local Flood Authority or Internal Drainage Board to see if they

have other relevant local flood information. Please note that some areas do not have an Internal Drainage Board.

We aren't able to display flood defence locations and attributes as there are no formal flood defences in the area of interest.

Surface water and other sources of flooding

Use the [long term flood risk service](#) to find out about the risk of flooding from:

- surface water
- ordinary watercourses
- reservoirs

For information about sewer flooding, contact the relevant water company for the area.

About the models used

Model name: Corn Brook 2017

Scenario(s): No defences exist fluvial, no defences exist climate change fluvial

Date: 31 December 2017

This model contains the most relevant data for your area of interest.

Terminology used

Annual exceedance probability (AEP)

This refers to the probability of a flood event occurring in any year. The probability is expressed as a percentage. For example, a large flood which is calculated to have a 1% chance of occurring in any one year, is described as 1% AEP.

Metres above ordnance datum (mAOD)

All flood levels are given in metres above ordnance datum which is defined as the mean sea level at Newlyn, Cornwall.

Flood map for planning (rivers and the sea)

Your selected location is in flood zone 3.

Flood zone 3 shows the area at risk of flooding for an undefended flood event with a:

- 0.5% or greater probability of occurring in any year for flooding from the sea
- 1% or greater probability of occurring in any year for fluvial (river) flooding

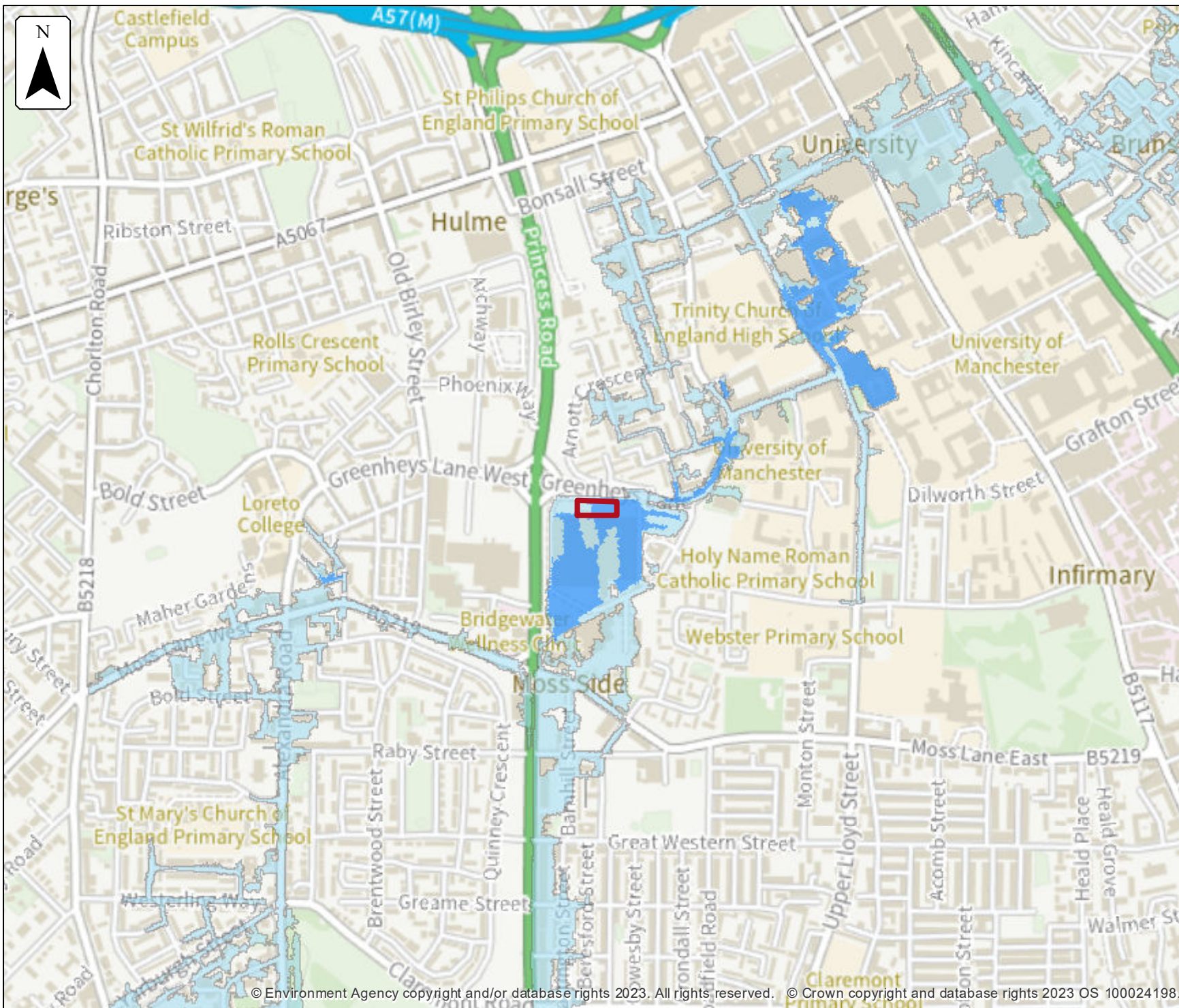
Flood zone 2 shows the area at risk of flooding for an undefended flood event with:

- between a 0.1% and 0.5% probability of occurring in any year for flooding from the sea
- between a 0.1% and 1% probability of occurring in any year for fluvial (river) flooding

It's important to remember that the flood zones on this map:

- refer to the land at risk of flooding and do not refer to individual properties
- refer to the probability of river and sea flooding, ignoring the presence of defences
- do not take into account potential impacts of climate change

This data is updated on a quarterly basis as better data becomes available.



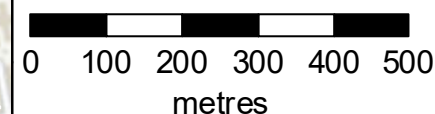
Flood map for planning

Location (easting/northing)
383947/396171

Scale
1:10,000

Created
30 Aug 2023

-  Selected area
-  Flood zone 3
-  Flood zone 2



Modelled data

This section provides details of different scenarios we have modelled and includes the following (where available):

- outline maps showing the area at risk from flooding in different modelled scenarios
- modelled node point map(s) showing the points used to get the data to model the scenarios and table(s) providing details of the flood risk for different return periods
- map(s) showing the approximate water levels for the return period with the largest flood extent for a scenario and table(s) of sample points providing details of the flood risk for different return periods

Climate change

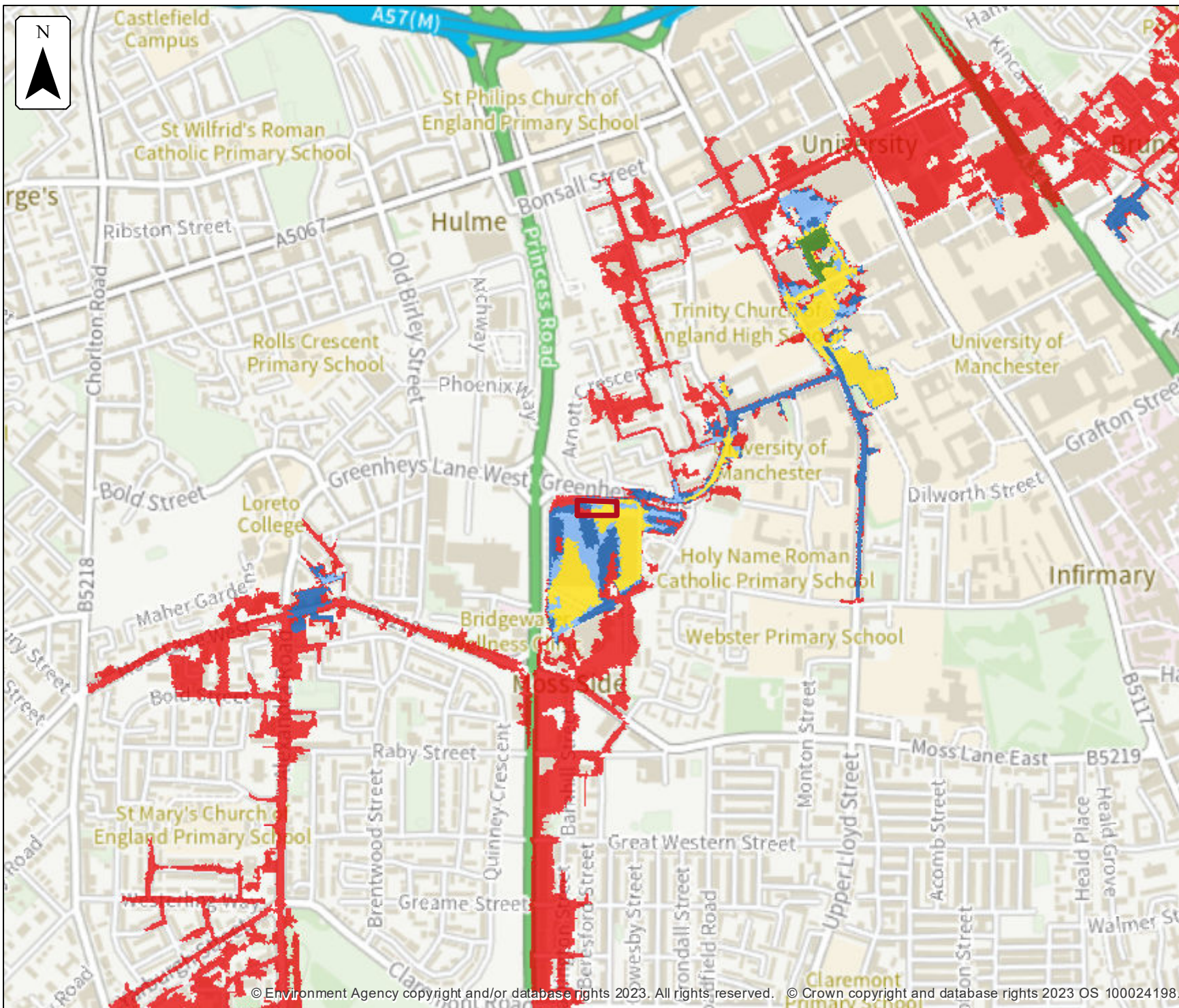
The climate change data included in the models may not include the latest [flood risk assessment climate change allowances](#). Where the new allowances are not available you will need to consider this data and factor in the new allowances to demonstrate the development will be safe from flooding.

The Environment Agency will incorporate the new allowances into future modelling studies. For now, it's your responsibility to demonstrate that new developments will be safe in flood risk terms for their lifetime.

Modelled scenarios

The following scenarios are included:

- No defences exist modelled fluvial: risk of flooding from rivers where there are no flood defences
- No defences exist climate change modelled fluvial: risk of flooding from rivers where there are no flood defences, including estimated impact of climate change



No defences exist modelled fluvial extent

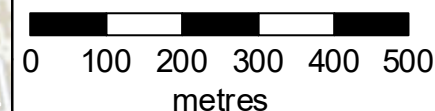
Location (easting/northing)
383947/396171

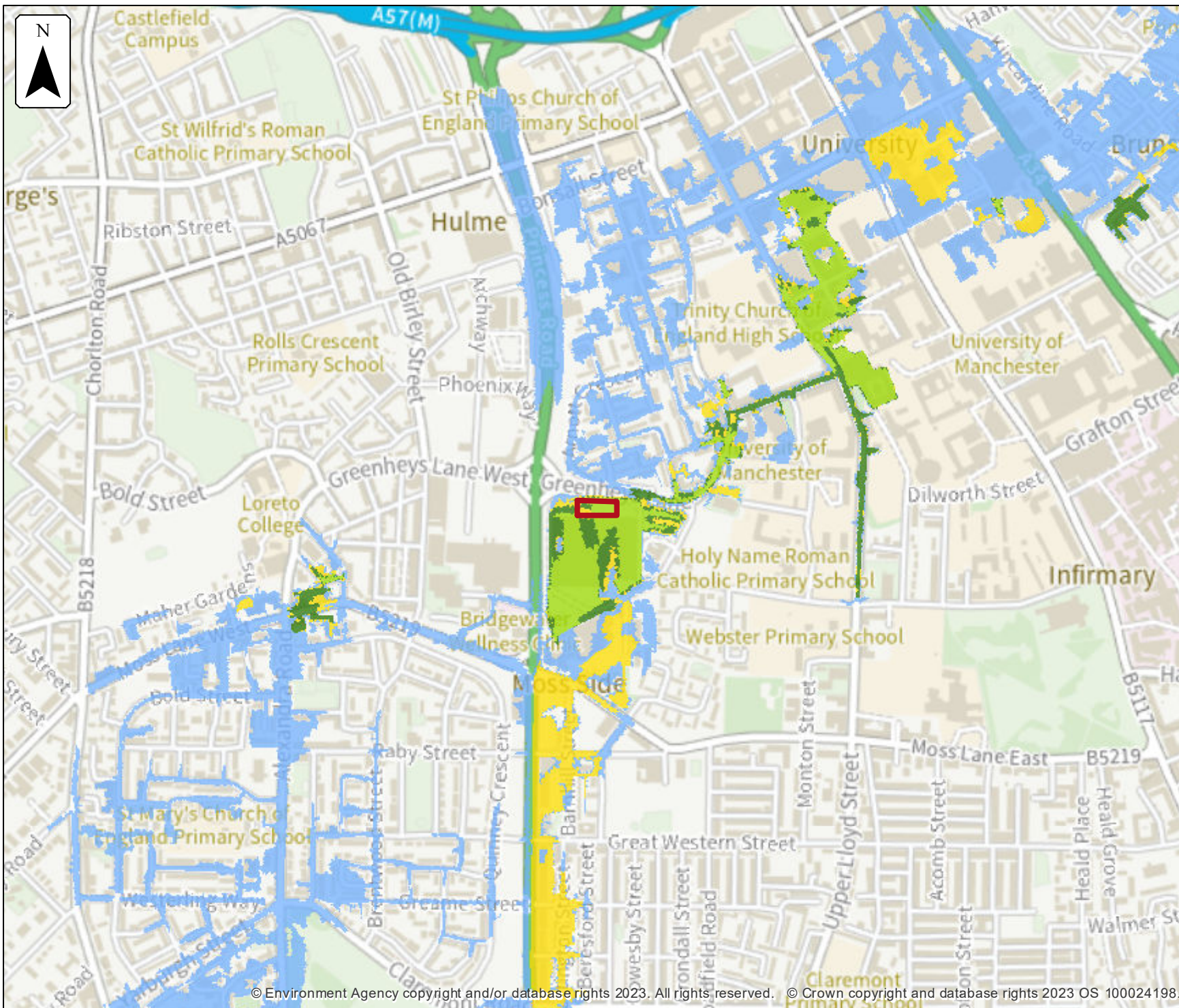
Scale Created
1:10,000 30 Aug 2023

Model name
Corn Brook 2017

- Selected area
- Modelled flood extent
 - 2% AEP
 - 1.33% AEP
 - 1% AEP
 - 0.5% AEP
 - 0.1% AEP

Flood extents may not be visible where they overlap other return periods





No defences exist climate change modelled fluvial extent

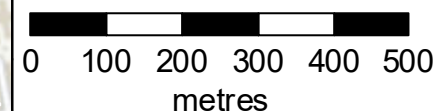
Location (easting/northing)
383947/396171

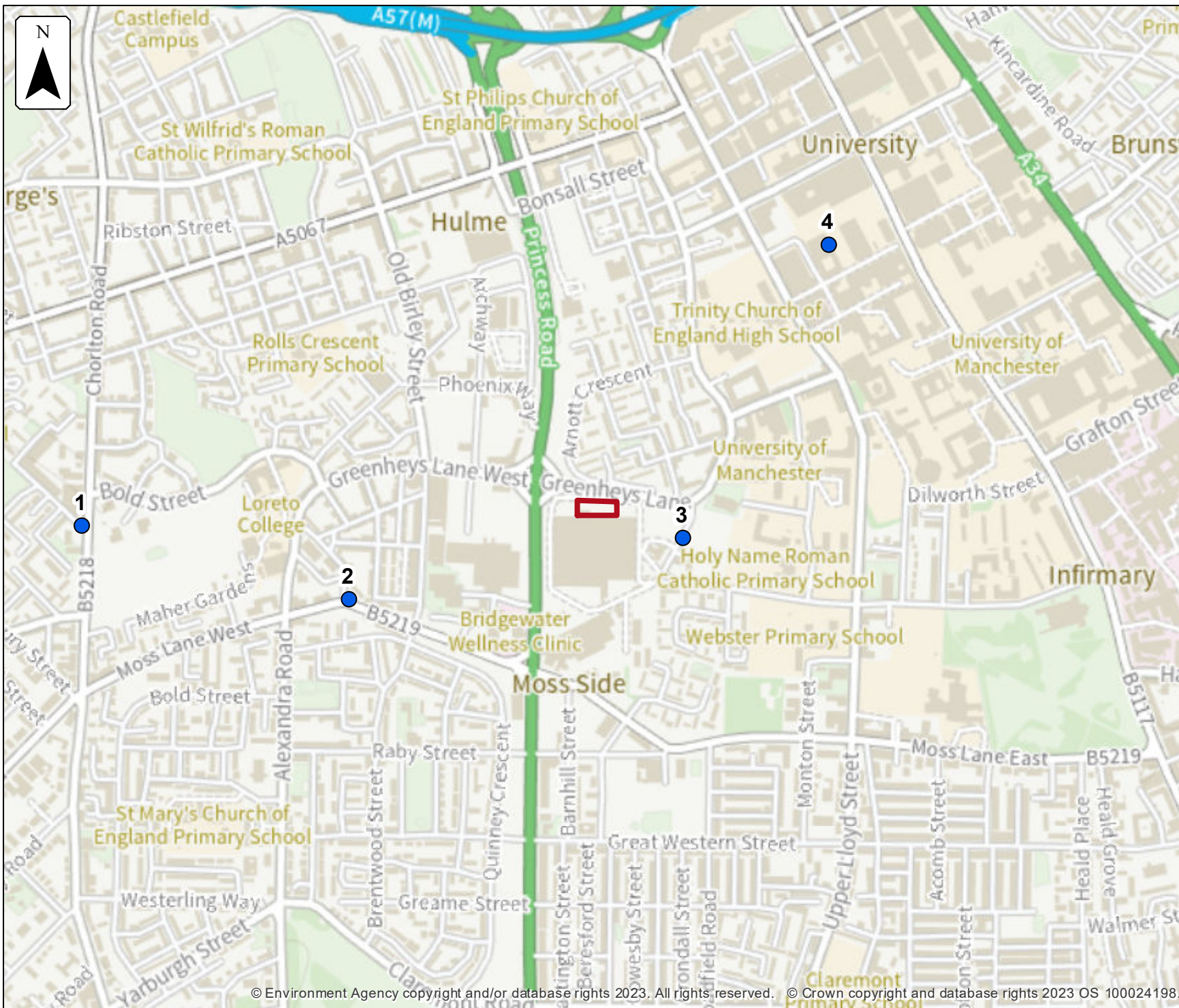
Scale Created
1:10,000 30 Aug 2023

Model name
Corn Brook 2017

- Selected area
- Main river
- Modelled flood extent
- 1.0% AEP (+30%)
- 1.0% AEP (+35%)
- 1.0% AEP (+70%)
- 0.1% AEP (+30%)

Flood extents may not be visible where they overlap other return periods





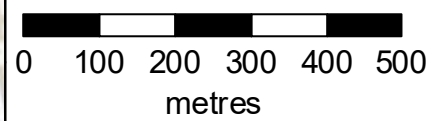
**No defences exist
modelled fluvial
node locations**

Location (easting/northing)
383947/396171

Scale Created
1:10,000 30 Aug 2023

Model name
Corn Brook 2017

-  Selected area
-  Modelled location



Modelled node locations data

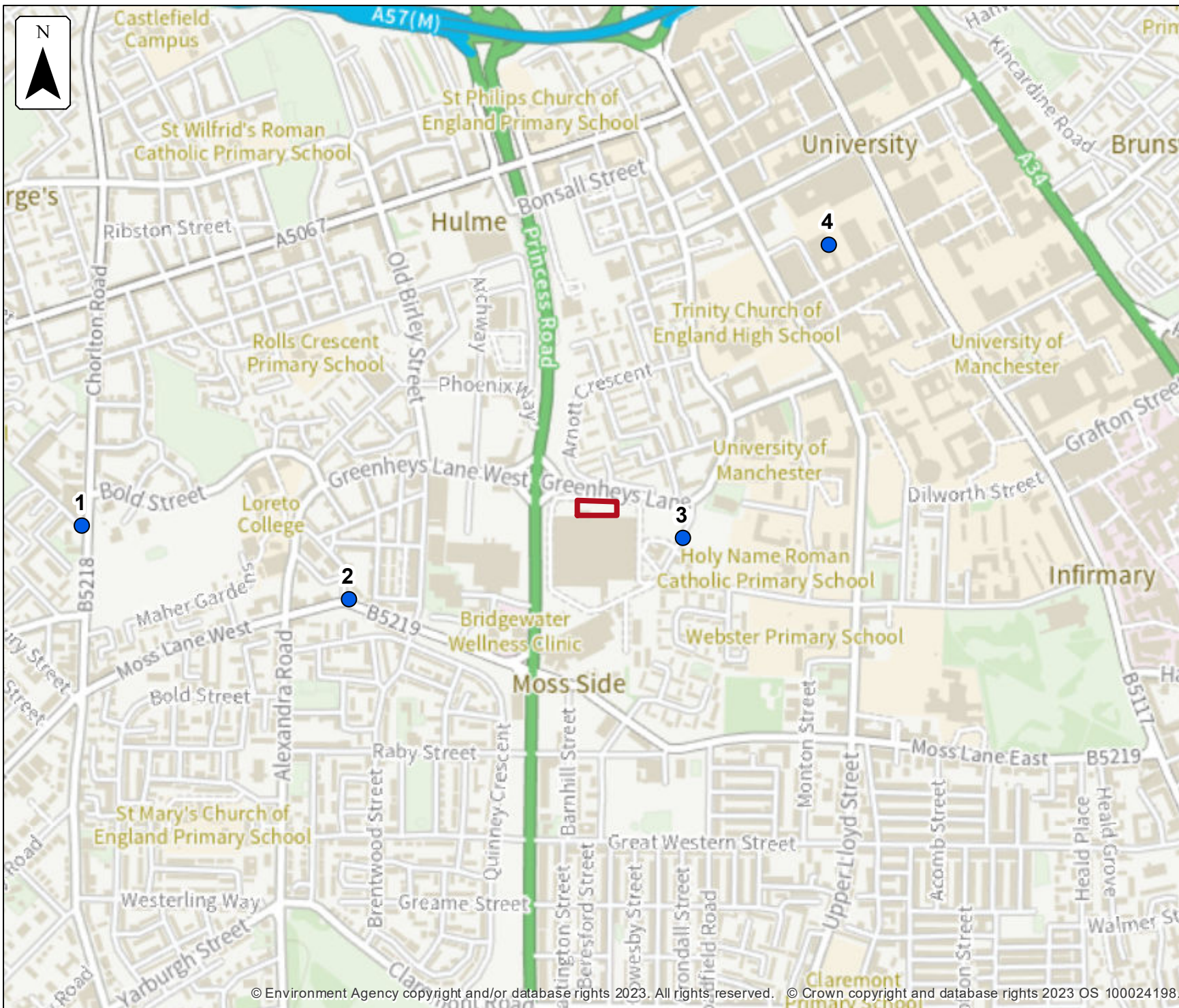
No defences exist

Label	Modelled location ID	Easting	Northing	5% AEP		2% AEP		1.33% AEP		1% AEP		0.5% AEP		0.1% AEP	
				Level	Flow	Level	Flow	Level	Flow	Level	Flow	Level	Flow	Level	Flow
1	952451	382950	396136	29.55	0.53	29.95	1.13	30.07	1.46	30.16	1.72	30.47	2.47	31.73	5.10
2	952452	383467	395993	30.90	0.49	31.93	1.03	32.43	1.32	32.58	1.57	32.84	2.24	33.28	4.60
3	952465	384116	396112	32.44	0.39	32.77	0.81	33.26	1.03	33.47	1.21	33.83	1.70	34.34	3.47
4	952467	384400	396681	35.44	0.57	36.89	1.20	37.13	1.56	37.27	1.84	37.41	2.65	37.66	5.49

Data in this table comes from the Corn Brook 2017 model.

Level values are shown in mAOD, and flow values are shown in cubic metres per second.

Any blank cells show where a particular scenario has not been modelled for this location.



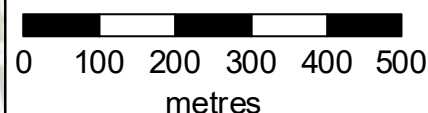
**No defences exist
climate change
modelled fluvial
node locations**

Location (easting/northing)
383947/396171

Scale Created
1:10,000 30 Aug 2023

Model name
Corn Brook 2017

-  Selected area
-  Modelled location



Modelled node locations data

No defences exist climate change

Label	Modelled location ID	Easting	Northing	1.0% AEP (+30%)		1.0% AEP (+35%)		1.0% AEP (+70%)		0.1% AEP (+30%)	
				Level	Flow	Level	Flow	Level	Flow	Level	Flow
1	952451	382950	396136	30.34	2.24	30.39	2.33	30.82	2.93	31.99	6.63
2	952452	383467	395993	32.78	2.03	32.81	2.11	32.99	2.66	33.39	5.98
3	952465	384116	396112	33.75	1.57	33.79	1.63	34.02	2.05	34.46	4.51
4	952467	384400	396681	37.38	2.40	37.39	2.49	37.47	3.13	37.77	7.13

Data in this table comes from the Corn Brook 2017 model.

Level values are shown in mAOD, and flow values are shown in cubic metres per second.

Any blank cells show where a particular scenario has not been modelled for this location.



No defences exist modelled fluvial extent and height

Location (easting/northing)
383947/396171

Scale Created
1:1,000 30 Aug 2023

Model name
Corn Brook 2017

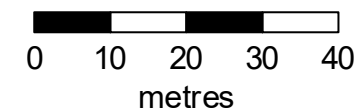
-  Selected area
-  Main river

Modelled 2D grid

Water level in mAOD

-  0 - 33.0
-  33.0 - 33.125
-  33.125 - 33.25
-  33.25 - 33.375
-  33.375 - 33.5
-  33.5 - 33.625
-  33.625 - 33.75
-  33.75 - 33.875
-  33.875 - 34.0

This map shows the
0.1% AEP height data



Sample point data

No defences exist

Label	Easting	Northing	5% AEP		2% AEP		1.33% AEP		1% AEP		0.5% AEP		0.1% AEP	
			Depth	Height	Depth	Height	Depth	Height	Depth	Height	Depth	Height	Depth	Height
1	383902	396150			NoData	NoData	NoData	NoData	0.14	33.40	0.31	33.57	0.48	33.74
2	383917	396150			NoData	NoData	NoData	NoData	NoData	NoData	0.19	33.57	0.33	33.71
3	383932	396150			NoData	NoData	NoData	NoData	0.11	33.45	0.22	33.57	0.36	33.71
4	383947	396150			NoData	NoData	0.13	33.43	0.15	33.45	0.27	33.57	0.41	33.71
5	383962	396150			NoData	NoData	0.13	33.43	0.15	33.45	0.27	33.57	0.41	33.71
6	383977	396150			NoData	NoData	0.11	33.44	0.14	33.46	0.24	33.57	0.38	33.71
7	383992	396150			NoData	NoData	0.15	33.44	0.18	33.46	0.28	33.57	0.43	33.71
8	383902	396165			NoData	NoData	NoData	NoData	NoData	NoData	0.26	33.57	0.44	33.74
9	383917	396165			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	0.11	33.71
10	383932	396165			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	0.22	33.71
11	383947	396165			NoData	NoData	0.15	33.49	0.17	33.54	0.27	33.61	0.43	33.78
12	383962	396165			NoData	NoData	0.14	33.49	0.18	33.54	0.28	33.62	0.45	33.78
13	383977	396165			NoData	NoData	0.16	33.49	0.20	33.54	0.28	33.62	0.45	33.78
14	383992	396165			NoData	NoData	0.19	33.49	0.22	33.54	0.32	33.62	0.49	33.79
15	383902	396180			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	0.23	33.75
16	383917	396180			NoData	NoData	NoData	NoData	NoData	NoData	0.10	33.60	0.26	33.76

Label	Easting	Northing	5% AEP		2% AEP		1.33% AEP		1% AEP		0.5% AEP		0.1% AEP	
			Depth	Height	Depth	Height	Depth	Height	Depth	Height	Depth	Height	Depth	Height
17	383932	396180			NoData	NoData	NoData	NoData	NoData	NoData	0.17	33.61	0.33	33.77
18	383947	396180			NoData	NoData	NoData	NoData	0.14	33.54	0.22	33.61	0.38	33.78
19	383962	396180			NoData	NoData	0.13	33.49	0.17	33.54	0.25	33.62	0.41	33.78
20	383977	396180			NoData	NoData	NoData	NoData	0.13	33.54	0.21	33.62	0.38	33.78
21	383992	396180			NoData	NoData	NoData	NoData	0.12	33.54	0.20	33.62	0.37	33.79
22	383902	396195			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	0.19	33.76
23	383917	396195			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	0.24	33.76
24	383932	396195			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	0.22	33.77
25	383947	396195			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	0.22	33.78
26	383962	396195			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData
27	383977	396195			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData
28	383992	396195			NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData

Data in this table comes from the Corn Brook 2017 model.

Height values are shown in mAOD, and depth values are shown in metres.

Any blank cells show where a particular scenario has not been modelled for this location.

Cells which contain text 'NoData' for a scenario show that return period has been modelled but there is no flood risk for that return period for that location.



**No defences exist
climate change
modelled fluvial
extent and height**

Location (easting/northing)
383947/396171

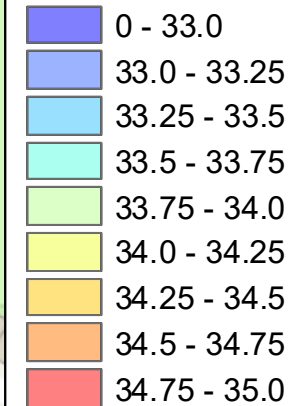
Scale Created
1:1,000 30 Aug 2023

Model name
Corn Brook 2017

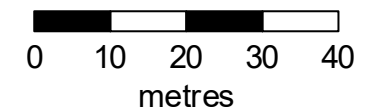
 Selected area

 Main river

Modelled 2D grid
Water level in mAOD



This map shows the
0.1% AEP +30% height data



Sample point data

No defences exist climate change

Label	Easting	Northing	1% AEP (+30%)		1% AEP (+35%)		1% AEP (+70%)		0.1% AEP (+30%)	
			Depth	Height	Depth	Height	Depth	Height	Depth	Height
1	383902	396150	0.28	33.54	0.29	33.55	0.37	33.63	0.57	33.83
2	383917	396150	0.16	33.54	0.18	33.56	0.24	33.62	0.40	33.77
3	383932	396150	0.19	33.54	0.21	33.56	0.27	33.62	0.43	33.78
4	383947	396150	0.24	33.54	0.26	33.56	0.32	33.62	0.48	33.78
5	383962	396150	0.25	33.54	0.26	33.56	0.32	33.62	0.48	33.77
6	383977	396150	0.21	33.54	0.23	33.56	0.29	33.62	0.45	33.78
7	383992	396150	0.26	33.54	0.27	33.56	0.33	33.62	0.49	33.78
8	383902	396165	0.23	33.54	0.25	33.55	0.33	33.63	0.52	33.82
9	383917	396165	NoData	NoData	NoData	NoData	NoData	NoData	0.17	33.78
10	383932	396165	NoData	NoData	NoData	NoData	0.13	33.62	0.28	33.77
11	383947	396165	0.24	33.59	0.26	33.60	0.32	33.67	0.50	33.87
12	383962	396165	0.26	33.59	0.27	33.60	0.34	33.67	0.51	33.87
13	383977	396165	0.26	33.59	0.27	33.60	0.34	33.67	0.54	33.87
14	383992	396165	0.29	33.59	0.31	33.61	0.37	33.67	0.58	33.88
15	383902	396180	NoData	NoData	NoData	NoData	0.13	33.64	0.32	33.83
16	383917	396180	NoData	NoData	NoData	NoData	0.15	33.65	0.34	33.84

Label	Easting	Northing	1% AEP (+30%)		1% AEP (+35%)		1% AEP (+70%)		0.1% AEP (+30%)	
			Depth	Height	Depth	Height	Depth	Height	Depth	Height
17	383932	396180	0.15	33.58	0.16	33.60	0.22	33.66	0.44	33.85
18	383947	396180	0.19	33.59	0.21	33.60	0.27	33.67	0.47	33.86
19	383962	396180	0.22	33.59	0.24	33.60	0.30	33.67	0.50	33.87
20	383977	396180	0.18	33.59	0.20	33.60	0.26	33.67	0.46	33.87
21	383992	396180	0.17	33.59	0.19	33.61	0.25	33.67	0.46	33.88
22	383902	396195	NoData	NoData	NoData	NoData	NoData	NoData	0.27	33.84
23	383917	396195	NoData	NoData	NoData	NoData	0.13	33.65	0.32	33.84
24	383932	396195	NoData	NoData	NoData	NoData	0.11	33.66	0.31	33.86
25	383947	396195	NoData	NoData	NoData	NoData	0.11	33.67	NoData	NoData
26	383962	396195	NoData	NoData	NoData	NoData	NoData	NoData	0.14	33.87
27	383977	396195	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData
28	383992	396195	NoData	NoData	NoData	NoData	NoData	NoData	NoData	NoData

Data in this table comes from the Corn Brook 2017 model.

Height values are shown in mAOD, and depth values are shown in metres.

Any blank cells show where a particular scenario has not been modelled for this location.

Cells which contain text 'NoData' for a scenario show that return period has been modelled but there is no flood risk for that return period for that location.

Strategic flood risk assessments

We recommend that you check the relevant local authority's strategic flood risk assessment (SFRA) as part of your work to prepare a site specific flood risk assessment.

This should give you information about:

- the potential impacts of climate change in this catchment
- areas defined as functional floodplain
- flooding from other sources, such as surface water, ground water and reservoirs

About this data

This data has been generated by strategic scale flood models and is not intended for use at the individual property scale. If you're intending to use this data as part of a flood risk assessment, please include an appropriate modelling tolerance as part of your assessment. The Environment Agency regularly updates its modelling. We recommend that you check the data provided is the most recent, before submitting your flood risk assessment.

Flood risk activity permits

Under the Environmental Permitting (England and Wales) Regulations 2016 some developments may require an environmental permit for flood risk activities from the Environment Agency. This includes any permanent or temporary works that are in, over, under, or nearby a designated main river or flood defence structure.

[Find out more about flood risk activity permits](#)

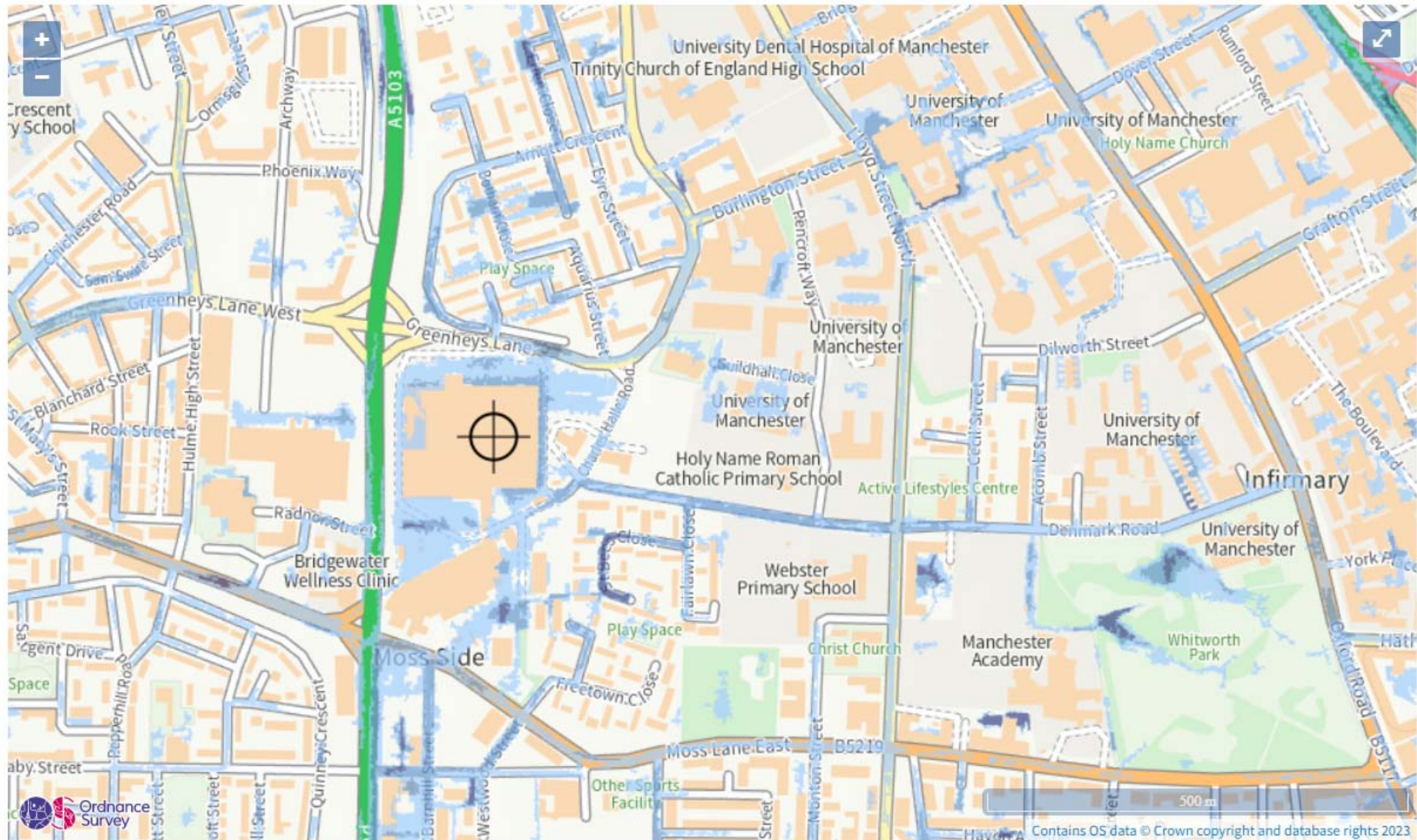
Help and advice

Contact the Greater Manchester Merseyside and Cheshire Environment Agency team at inforequests.gmmc@environment-agency.gov.uk for:

- [more information about getting a product 5, 6, 7 or 8](#)
- general help and advice about the site you're requesting data for

Heineken, Manchester

Flood Risk From Surface Water



Extent of flooding from surface water

● High ● Medium ● Low ○ Very low ⊕ Location you selected

Appendix E – Sewer and Drainage Records

SEWER United



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red - combined blue - surface water
brown - foul purple - overflow

- ★ Manhole
- ★ Head of System
- ★ End of Survey
- ★ Rising Inlet
- ★ Jolt
- ★ Discharge Point
- ★ Penstock
- ★ Washout Chamber
- ★ Valve
- ★ Air Valve
- ★ Non Return Valve
- ★ Sashway
- ★ Gully
- ★ Cascade
- ★ Flume Meter
- ★ Hatch Box
- ★ Oil Interceptor
- ★ Summit
- ★ Drop Shaft
- ★ Orifice Plate
- ★ Side Entry Manhole
- ★ Outfall
- ★ Screen Chamber
- ★ Inspection Chamber
- ★ Effluent Chamber
- ★ Lagoon Hole
- ★ T Junction / Saddle
- ★ Catchpot
- ★ Valve Chamber
- ★ Vent Column
- ★ Vortex Chamber
- ★ Perist Chamber
- ★ Network Storage Tank
- ★ Sewer Overflow
- ★ We Treatment Works
- ★ We Pumping Station
- ★ Reuse Tank
- ★ Control Kiosk
- ★ Change of Characteristic

FO Foul
SW Surface Water
CO Combined
OV Overflow

CI	Circular	TR	Trapezoidal
EG	Egg	AR	Arch
OV	Oval	BA	Barrel
FT	Flat Top	HO	Horseshoe
RE	Rectangular	UN	Unspecified
SQ	Square		

AC	Asbestos Cement
BR	Brick
PE	Polyethylene
RP	Reinforced Plastic Matrix
CO	Concrete
C8B	Concrete Segment Bullied
CSU	Concrete Segment Unbullied
CC	Concrete Box Culverted
PSC	Plastic / Steel Composite
GRC	Glass Reinforced Plastic
DI	Ductile Iron
PVC	Polyvinyl Chloride
CI	Cast Iron
SI	Spun Iron
ST	Steel
VC	Verticilled Clay
PP	Polypropylene
FF	Fiberglass Fibers
MAC	Masonry Course

Heineken Manchester Denmark Road,

Printed by: Property Searches



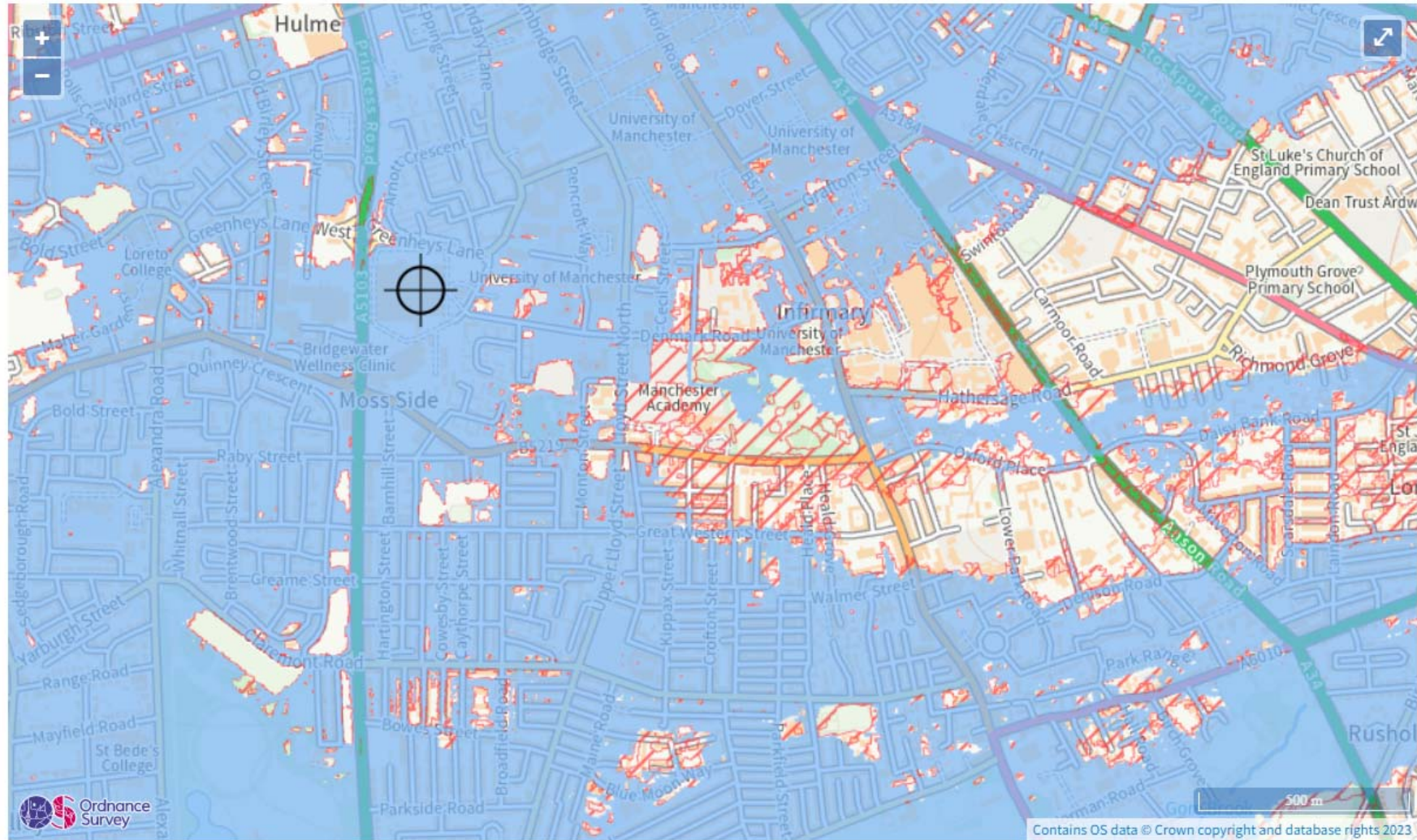
United Utilities



SEWER RECORDS

Heineken, Manchester

Flood Risk From Reservoirs



Maximum extent of flooding from reservoirs:

● when river levels are normal ■ when there is also flooding from rivers ⊕ Location you selected

Appendix F – Emcus Letter Regarding Infiltration

14 June 2023

J1834_LTR_03

Norder Design Associates,
Beech Lawn,
Green Lane,
Belper,
Derbyshire,
DE56 1BY



Emcus Limited, 1E Campbell Street, Belper, Derbyshire

tel: 01773 850365 email: advice@emcus.co.uk web: www.emcus.co.uk

For the attention of: Anne Fox

Dear Sirs,

Can Intake and Warehouse, Royal Brewery, Manchester

Further to the issue of our geotechnical & geo-environmental investigation report for the above site, (report reference MJH/J1834/D3/1, November 2019), it is considered that the cohesive natural strata predominantly encountered immediately beneath the site will not be sufficiently permeable to receive water from soakaways. Furthermore, it is recommended that water from soakaways, permeable pavements etc., is not discharged into the Made Ground beneath the site.

Yours faithfully

For & on behalf of Emcus Limited

Marcus Hanford
Technical Director
mhanford@emcus.co.uk



Appendix G – Micro Drainage Print Outs

Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY		
Date 23/06/2023 14:02 File 8901 GREENFIELD RUN OFF...	Designed by anne-fox Checked by	
Innovyze	Source Control 2020.1	

IH 124 Mean Annual Flood

Input

Return Period (years)	1	Soil	0.300
Area (ha)	0.265	Urban	0.750
SAAR (mm)	848	Region Number	Region 10

Results l/s

QBAR Rural 1.1
QBAR Urban 3.2

Q1 year 2.7

Q1 year 2.7
Q2 years 3.2
Q5 years 3.9
Q10 years 4.2
Q20 years 4.4
Q25 years 4.5
Q30 years 4.6
Q50 years 4.7
Q100 years 4.9
Q200 years 5.3
Q250 years 5.4
Q1000 years 6.0

Warning: It is unusual to use the IH124 method with an area < 50ha. The Interim Code of Practice recommends that the IH124 method is applied with 50ha and the resulting discharge is linearly interpolated for the required area. The ICP SUDS tab will do this automatically.

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Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	Finished Goods Warehouse Brownfield Run Off 1 in 1 Year Storm	
Date 23/06/2023 14:10 File EXISTING BROWNFIELD RUN...	Designed by AF Checked by	
Innovyze	Network 2020.1	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	1	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.360	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	200	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	10.000	0.050	200.0	0.265	5.00	0.0	0.600	o	375	Pipe/Conduit	
1.001	10.000	0.050	200.0	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	46.30	5.13	31.300	0.265	0.0	0.0	0.0	1.28	141.1	33.2
1.001	45.80	5.26	31.250	0.265	0.0	0.0	0.0	1.28	141.1	33.2

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Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	Finished Goods Warehouse Brownfield Run Off 1 in 100 Year Storm	
Date 23/06/2023 14:13 File EXISTING BROWNFIELD RUN...	Designed by AF Checked by	
Innovyze	Network 2020.1	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.360	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	200	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	10.000	0.050	200.0	0.265	5.00	0.0	0.600	o	375	Pipe/Conduit	
1.001	10.000	0.050	200.0	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	136.40	5.13	31.300	0.265	0.0	0.0	0.0	1.28	141.1	97.9
1.001	135.16	5.26	31.250	0.265	0.0	0.0	0.0	1.28	141.1	97.9

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Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester Finished Goods Warehouse Network Details	
Date 23/06/2023	Designed by AF	
File 230623 SW Design with A...	Checked by AF	
Innovyze	Network 2020.1	

STORM SEWER DESIGN by the Modified Rational Method

Pipe Sizes STANDARD Manhole Sizes STANDARD

Designed with Level Soffits

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.176	4-8	0.150

Network Design Table for Storm

Network Results Table

Norder Design Assoc

Beech Lawn
Green Lane, Belper
DE56 1BY

Date 23/06/2023
File 230623 SW Design with A...

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Heineken, Manchester
Finished Goods Warehouse
Network Details

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Network 2020.1

Page 2



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Norder Design Assoc		Page 3
Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester Finished Goods Warehouse Network Details	
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Online Controls for Storm

Hydro-Brake® Optimum Manhole: 9, DS/PN: 1.004, Volume (m³): 3.2

Unit Reference	MD-SHE-0169-1600-1750-1600
Design Head (m)	1.750
Design Flow (l/s)	16.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	169
Invert Level (m)	31.435
Minimum Outlet Pipe Diameter (mm)	225
Suggested Manhole Diameter (mm)	1500

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.750	16.0
Flush-Flo™	0.511	15.9
Kick-Flo®	1.085	12.7
Mean Flow over Head Range	-	13.9

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	6.0	1.200	13.4	3.000	20.6	7.000	31.0
0.200	13.8	1.400	14.4	3.500	22.2	7.500	32.1
0.300	15.1	1.600	15.3	4.000	23.7	8.000	33.1
0.400	15.7	1.800	16.2	4.500	25.1	8.500	34.1
0.500	15.9	2.000	17.0	5.000	26.4	9.000	35.0
0.600	15.8	2.200	17.8	5.500	27.6	9.500	35.9
0.800	15.3	2.400	18.6	6.000	28.8		
1.000	13.9	2.600	19.3	6.500	29.9		

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Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester Finished Goods Warehouse Simulation 1 in 1 Year	
Date 23/06/2023	Designed by AF	
File 230623 SW Design with A...	Checked by AF	
Innovyze	Network 2020.1	

Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

```
Margin for Flood Risk Warning (mm) 300.0      DVD Status OFF
      Analysis Timestep    Fine Inertia Status OFF
            DTS Status      ON
```

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080
Return Period(s) (years)	1
Climate Change (%)	0

PN	US/MH		Return Period	Climate Change	First (X) Surcharges	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level
	Name	Storm							(m)
1.000	1	15 Winter	1	+0%					32.055
1.001	2	15 Winter	1	+0%					31.921
1.002	3	15 Winter	1	+0%					31.706
2.000	5	15 Winter	1	+0%					32.203
2.001	6	15 Winter	1	+0%					31.838
1.003	8	15 Winter	1	+0%					31.662
1.004	9	30 Winter	1	+0%					31.625

PN	US/MH Name	Surcharged	Flooded	Flow / Cap.	Overflow (l/s)	Half Drain	Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)			Time (mins)	Flow (l/s)		
1.000	1	-0.225	0.000	0.14			10.2	OK	
1.001	2	-0.182	0.000	0.31			20.5	OK	
1.002	3	-0.169	0.000	0.33			20.0	OK	
2.000	5	-0.197	0.000	0.23			15.7	OK	
2.001	6	-0.231	0.000	0.12			15.7	OK	
1.003	8	-0.134	0.000	0.58			34.3	OK	

Norder Design Assoc					Page 2	
Beech Lawn Green Lane, Belper DE56 1BY			Heineken, Manchester Finished Goods Warehouse Simulation 1 in 1 Year			
Date 23/06/2023 File 230623 SW Design with A...			Designed by AF Checked by AF			
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Beech Lawn Green Lane, Belper DE56 1BY				Heineken, Manchester Finished Goods Warehouse Simulation 1 in 30 Year																																																																																																																	
Date 23/06/2023 File 230623 SW Design with A...				Designed by AF Checked by AF																																																																																																																	
Innovyze				Network 2020.1																																																																																																																	
Summary of Critical Results by Maximum Level (Rank 1) for Storm Simulation Criteria Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000 Hot Start Level (mm) 0 Inlet Coeffiecient 0.800 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000 Foul Sewage per hectare (l/s) 0.000 Number of Input Hydrographs 0 Number of Storage Structures 1 Number of Online Controls 1 Number of Time/Area Diagrams 0 Number of Offline Controls 0 Number of Real Time Controls 0 Synthetic Rainfall Details Rainfall Model FSR Ratio R 0.346 Region England and Wales Cv (Summer) 0.750 M5-60 (mm) 18.300 Cv (Winter) 0.840 Margin for Flood Risk Warning (mm) 300.0 DVD Status OFF Analysis Timestep Fine Inertia Status OFF DTS Status ON Profile(s) Summer and Winter Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080 Return Period(s) (years) 30 Climate Change (%) 0																																																																																																																					
<table><tr><th colspan="9"></th><th>Water</th></tr><tr><th>PN</th><th>US/MH</th><th></th><th></th><th>Return</th><th>Climate</th><th>First (X)</th><th>First (Y)</th><th>First (Z)</th><th>Overflow</th><th>Level</th></tr><tr><th></th><th>Name</th><th>Storm</th><th></th><th>Period</th><th>Change</th><th>Surcharge</th><th>Flood</th><th>Overflow</th><th>Act.</th><th>(m)</th></tr><tr><td>1.000</td><td>1</td><td>15 Winter</td><td></td><td>30</td><td>+0%</td><td></td><td></td><td></td><td></td><td>32.102</td></tr><tr><td>1.001</td><td>2</td><td>15 Winter</td><td></td><td>30</td><td>+0%</td><td></td><td></td><td></td><td></td><td>32.027</td></tr><tr><td>1.002</td><td>3</td><td>60 Winter</td><td></td><td>30</td><td>+0%</td><td>30/15 Summer</td><td></td><td></td><td></td><td>31.967</td></tr><tr><td>2.000</td><td>5</td><td>15 Winter</td><td></td><td>30</td><td>+0%</td><td></td><td></td><td></td><td></td><td>32.273</td></tr><tr><td>2.001</td><td>6</td><td>60 Winter</td><td></td><td>30</td><td>+0%</td><td></td><td></td><td></td><td></td><td>31.962</td></tr><tr><td>1.003</td><td>8</td><td>60 Winter</td><td></td><td>30</td><td>+0%</td><td>30/15 Summer</td><td></td><td></td><td></td><td>31.957</td></tr><tr><td>1.004</td><td>9</td><td>60 Winter</td><td></td><td>30</td><td>+0%</td><td>30/15 Summer</td><td></td><td></td><td></td><td>31.942</td></tr></table>																		Water	PN	US/MH			Return	Climate	First (X)	First (Y)	First (Z)	Overflow	Level		Name	Storm		Period	Change	Surcharge	Flood	Overflow	Act.	(m)	1.000	1	15 Winter		30	+0%					32.102	1.001	2	15 Winter		30	+0%					32.027	1.002	3	60 Winter		30	+0%	30/15 Summer				31.967	2.000	5	15 Winter		30	+0%					32.273	2.001	6	60 Winter		30	+0%					31.962	1.003	8	60 Winter		30	+0%	30/15 Summer				31.957	1.004	9	60 Winter		30	+0%	30/15 Summer				31.942
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1.000	1	15 Winter		30	+0%					32.102																																																																																																											
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<table><tr><th colspan="5">Surcharged</th><th>Flooded</th><th colspan="2">Half Drain</th><th>Pipe</th><th colspan="2"></th></tr><tr><th>PN</th><th>US/MH</th><th>Depth</th><th>Volume</th><th>Flow /</th><th>Overflow</th><th>Time</th><th>Flow</th><th></th><th></th><th>Level</th></tr><tr><th></th><th>Name</th><th>(m)</th><th>(m³)</th><th>Cap.</th><th>(l/s)</th><th>(mins)</th><th>(l/s)</th><th>Status</th><th></th><th>Exceeded</th></tr><tr><td>1.000</td><td>1</td><td>-0.178</td><td>0.000</td><td>0.35</td><td></td><td></td><td>25.0</td><td>OK</td><td></td><td></td></tr><tr><td>1.001</td><td>2</td><td>-0.076</td><td>0.000</td><td>0.83</td><td></td><td></td><td>55.3</td><td>OK</td><td></td><td></td></tr><tr><td>1.002</td><td>3</td><td>0.092</td><td>0.000</td><td>0.41</td><td></td><td></td><td>25.1</td><td>SURCHARGED</td><td></td><td></td></tr><tr><td>2.000</td><td>5</td><td>-0.127</td><td>0.000</td><td>0.57</td><td></td><td></td><td>38.5</td><td>OK</td><td></td><td></td></tr><tr><td>2.001</td><td>6</td><td>-0.107</td><td>0.000</td><td>0.16</td><td></td><td></td><td>21.2</td><td>OK</td><td></td><td></td></tr><tr><td>1.003</td><td>8</td><td>0.161</td><td>0.000</td><td>0.73</td><td></td><td></td><td>43.0</td><td>SURCHARGED</td><td></td><td></td></tr></table>									Surcharged					Flooded	Half Drain		Pipe			PN	US/MH	Depth	Volume	Flow /	Overflow	Time	Flow			Level		Name	(m)	(m³)	Cap.	(l/s)	(mins)	(l/s)	Status		Exceeded	1.000	1	-0.178	0.000	0.35			25.0	OK			1.001	2	-0.076	0.000	0.83			55.3	OK			1.002	3	0.092	0.000	0.41			25.1	SURCHARGED			2.000	5	-0.127	0.000	0.57			38.5	OK			2.001	6	-0.107	0.000	0.16			21.2	OK			1.003	8	0.161	0.000	0.73			43.0	SURCHARGED												
Surcharged					Flooded	Half Drain		Pipe																																																																																																													
PN	US/MH	Depth	Volume	Flow /	Overflow	Time	Flow			Level																																																																																																											
	Name	(m)	(m³)	Cap.	(l/s)	(mins)	(l/s)	Status		Exceeded																																																																																																											
1.000	1	-0.178	0.000	0.35			25.0	OK																																																																																																													
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2.000	5	-0.127	0.000	0.57			38.5	OK																																																																																																													
2.001	6	-0.107	0.000	0.16			21.2	OK																																																																																																													
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Date 23/06/2023 File 230623 SW Design with A...				Designed by AF Checked by AF			
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Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
		Depth (m)	Volume (m³)							
1.004	9	0.206	0.000	0.11				15.9	SURCHARGED	

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Beech Lawn
Green Lane, Belper
DE56 1BY

Date 23/06/2023
File 230623 SW Design with A...

Heineken, Manchester
Finished Goods Warehouse
Simulation 1 in 100 Year

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Network 2020.1

Page 1



Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000

Hot Start (mins) 0

Hot Start Level (mm) 0

Manhole Headloss Coeff (Global) 0.500

Foul Sewage per hectare (l/s) 0.000

Additional Flow - % of Total Flow 0.000

MADD Factor * 10m³/ha Storage 2.000

Inlet Coeffiecient 0.800

Flow per Person per Day (l/per/day) 0.000

Number of Input Hydrographs 0

Number of Storage Structures 1

Number of Online Controls 1

Number of Time/Area Diagrams 0

Number of Offline Controls 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Region England and Wales Cv (Summer) 0.750

M5-60 (mm) 18.300 Cv (Winter) 0.840

Ratio R 0.346

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep Fine

DTS Status ON

DVD Status OFF

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Period(s) (years) 100

Climate Change (%) 0

									Water
PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Level (m)
1.000	1	15 Winter	100	+0%					32.209
1.001	2	60 Winter	100	+0%	100/15 Summer				32.178
1.002	3	60 Winter	100	+0%	100/15 Summer				32.162
2.000	5	15 Winter	100	+0%					32.306
2.001	6	60 Winter	100	+0%	100/30 Winter				32.158
1.003	8	60 Winter	100	+0%	100/15 Summer				32.153
1.004	9	60 Winter	100	+0%	100/15 Summer				32.139

									Level
PN	US/MH Name	Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	Status	Exceeded
1.000	1	-0.071	0.000	0.43			31.1	OK	
1.001	2	0.075	0.000	0.59			39.1	SURCHARGED	
1.002	3	0.287	0.000	0.52			31.5	SURCHARGED	
2.000	5	-0.094	0.000	0.74			49.7	OK	
2.001	6	0.089	0.000	0.19			25.4	SURCHARGED	
1.003	8	0.357	0.000	0.90			52.9	SURCHARGED	

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Innovyze			Network 2020.1			

Micro Drainage

Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded		Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
		Depth (m)	Volume (m³)								
1.004	9	0.403	0.000	0.11					15.9	SURCHARGED	

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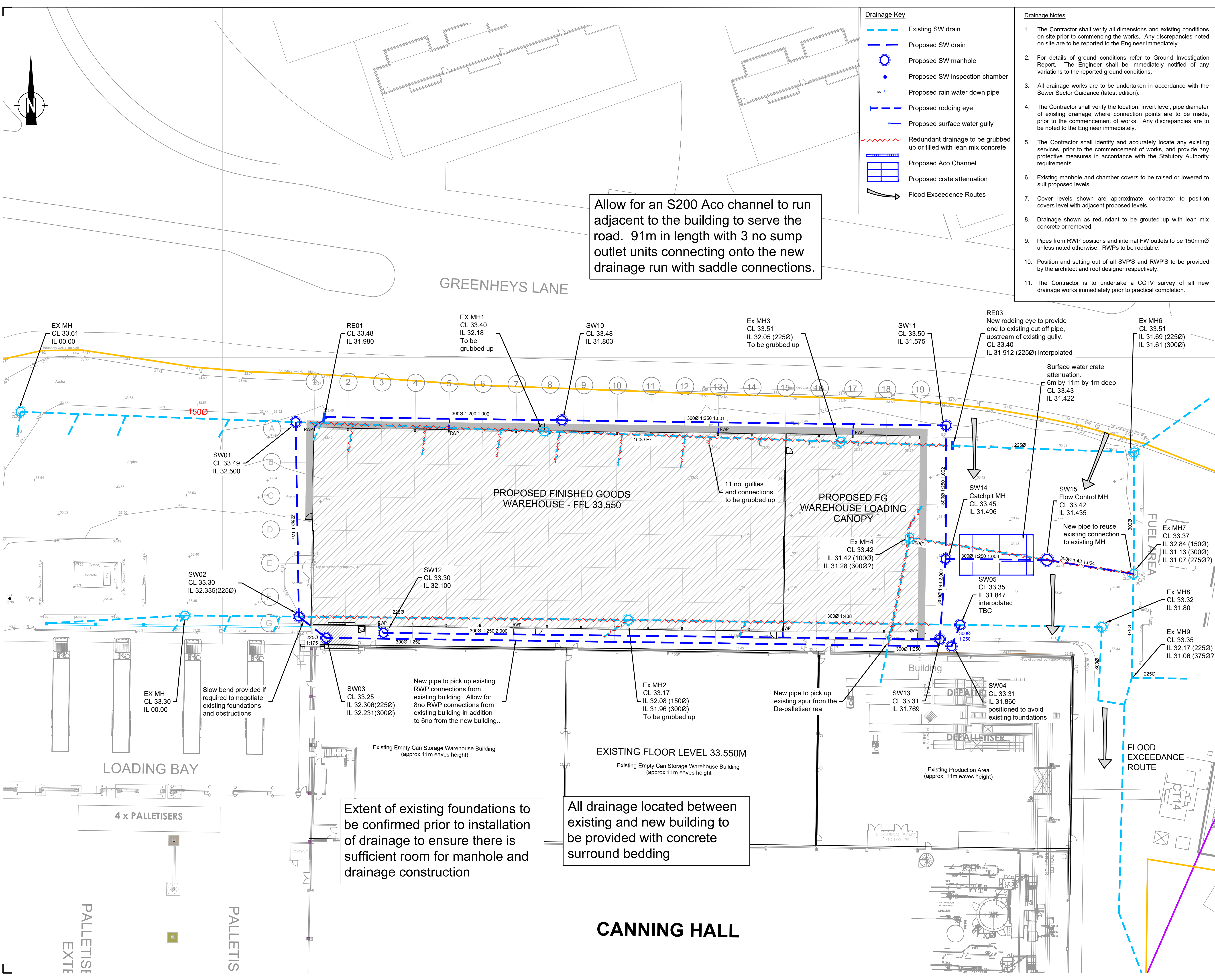
Norder Design Assoc					Page 2	
Beech Lawn Green Lane, Belper DE56 1BY			Heineken, Manchester Finished Goods Warehouse Simulation 1 in 100 Year+40%			
Date 23/06/2023 File 230623 SW Design with A...			Designed by AF Checked by AF			
Innovyze			Network 2020.1			

Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Surcharged Flooded		Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
		Depth (m)	Volume (m³)						
1.004	9	1.463	0.000	0.12			16.0	FLOOD RISK	

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Appendix H – Drainage Drawings



P02 23/06/23 For approval AF ML
P01 04/05/23 DRAFT For Tender AF ML
REV DATE AMENDMENT DRW CHK
CLIENT

HEINEKEN

PROJECT

**Manchester Brewery
Finished Goods Warehouse**

DRAWING TITLE

Drainage Layout

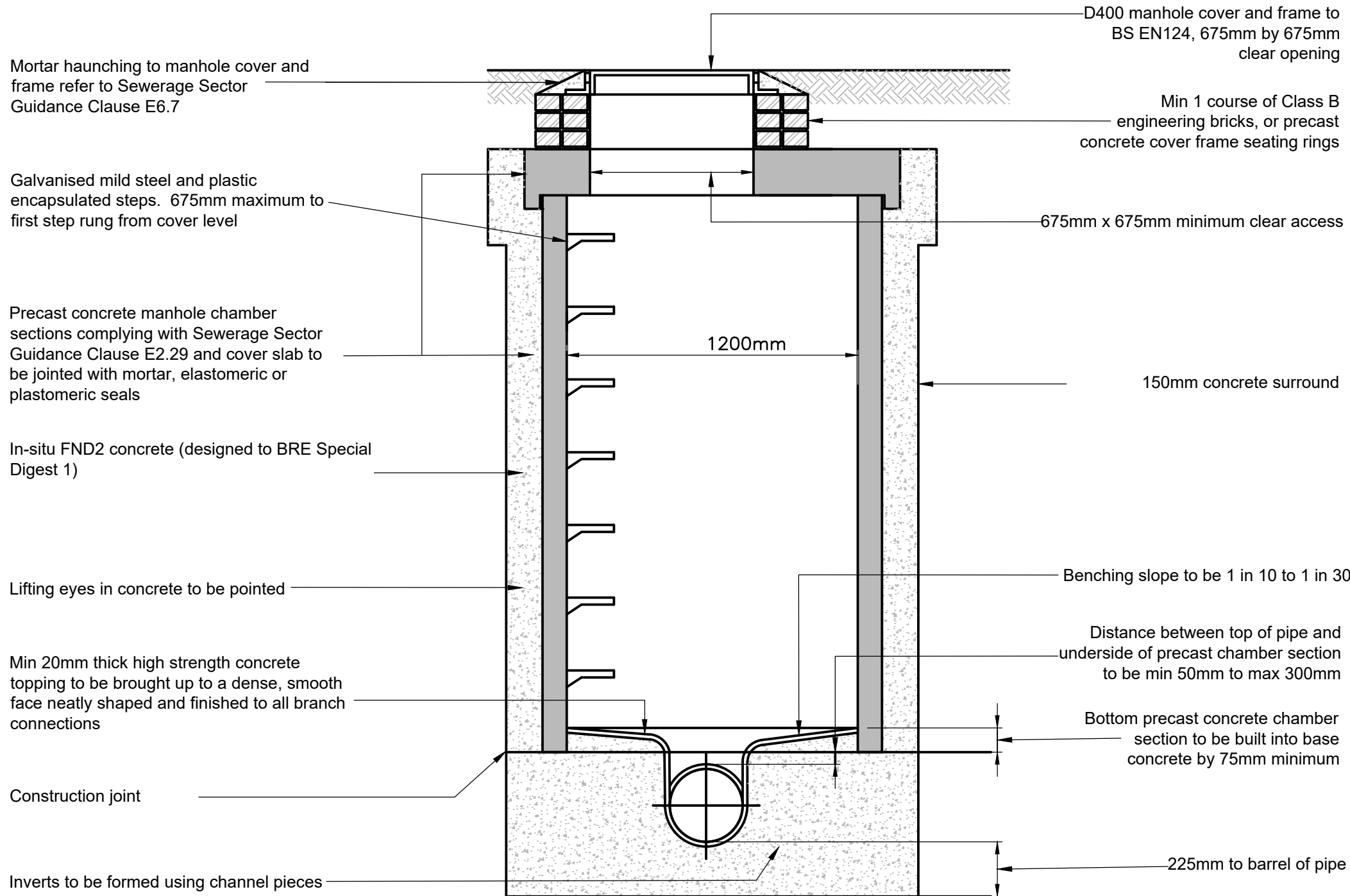
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Engineers. Architects. Innovators.

Beech Lawn, Green Lane, Belper, Derby DE56 1BY
Tel: 01773 824414
E-mail: enquiries@norder.co.uk

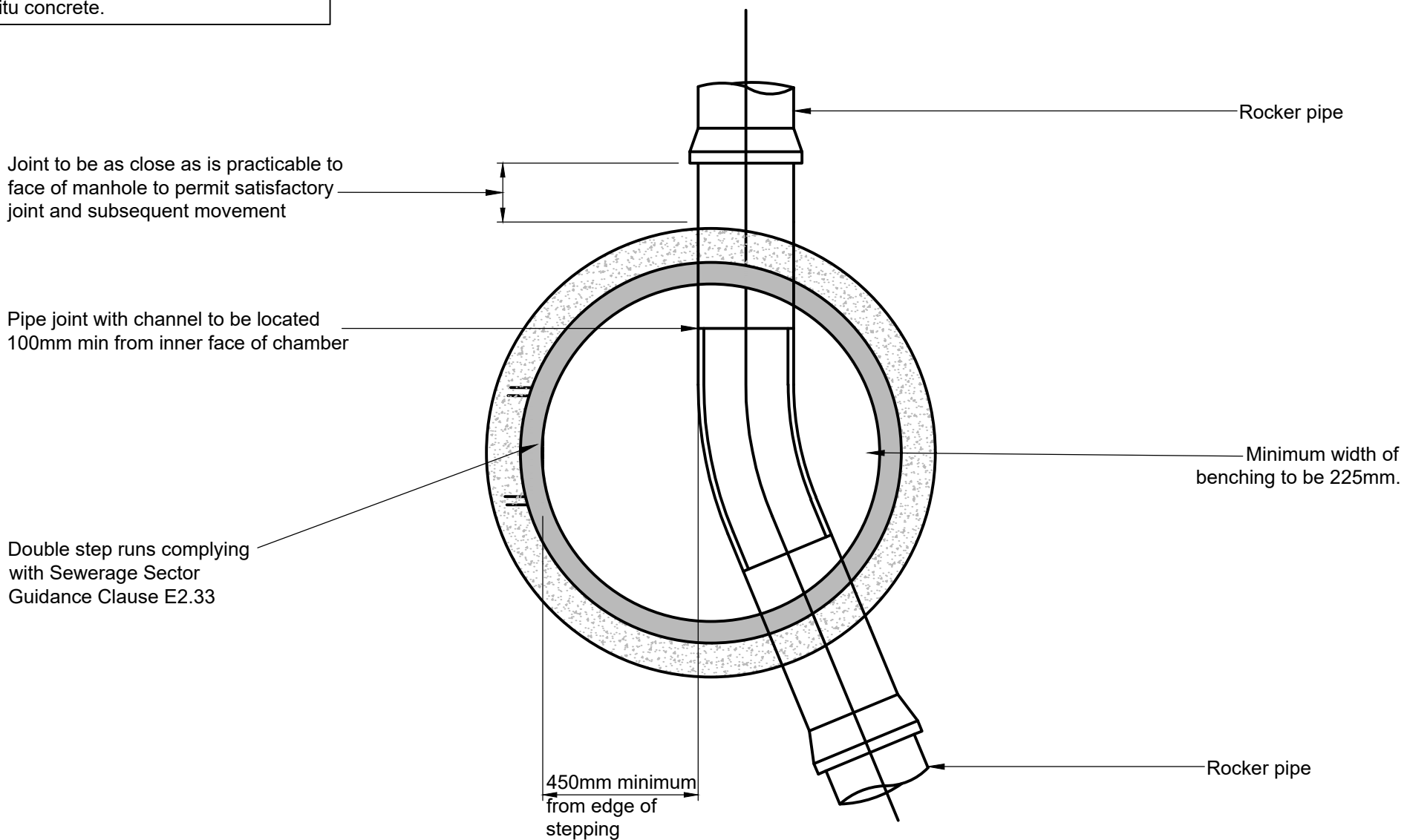
SCALE SHEET SIZE NDA REF
Scale 1 : 250 A1 8901

SUITABILITY
S2 - Suitable for Information

DRAWING NO. REV
8901-NDA-XX-XX-DR-C-1201 P02

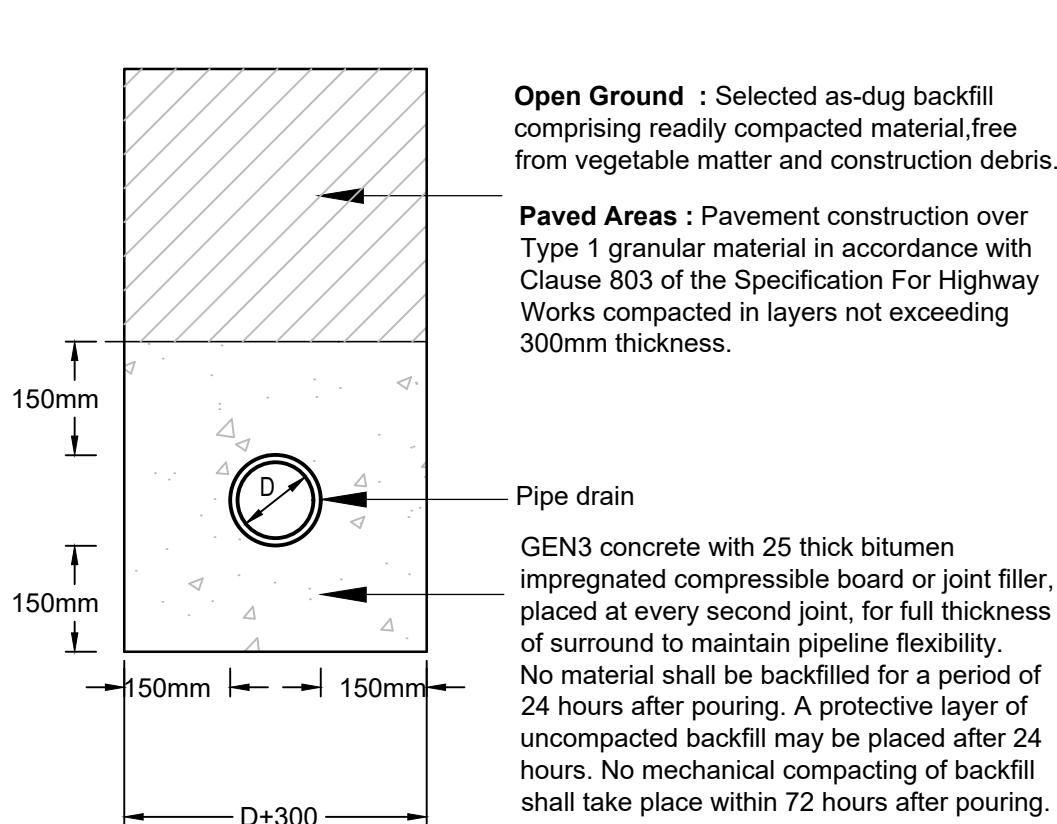


Precast concrete manhole bases can be used as an alternative to insitu concrete.



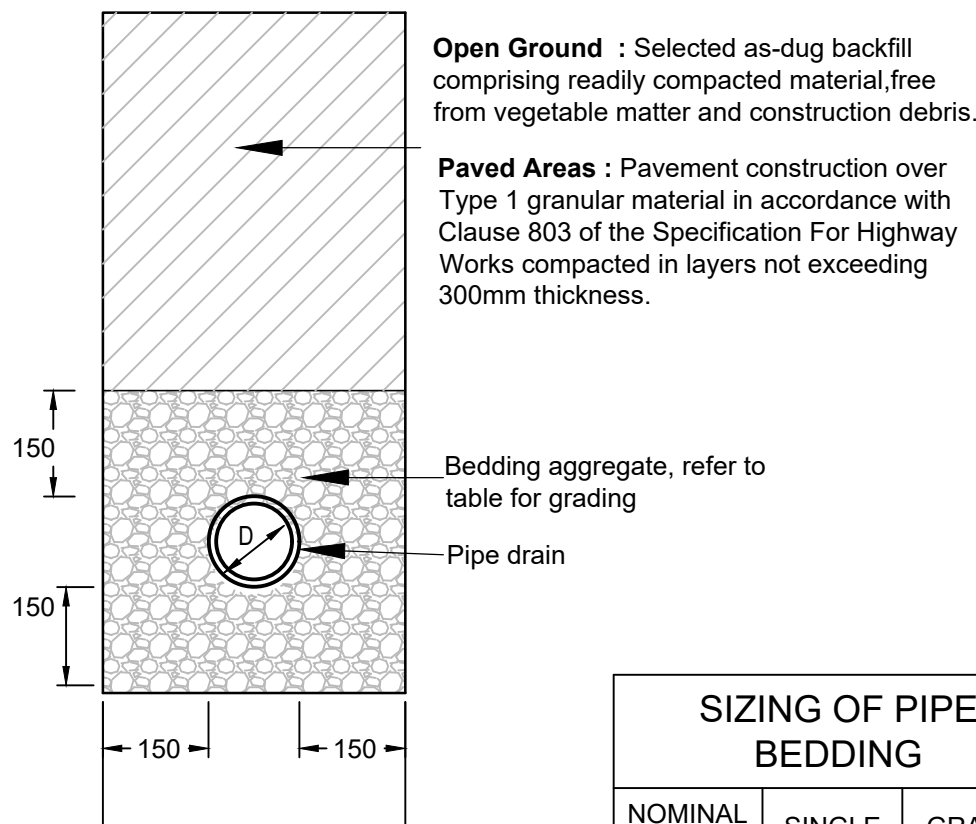
Typical Manhole Detail

Scale 1:20



Type Z Bedding

Depth to soffit <1.2m in trafficked areas
Depth to soffit <0.9 in pedestrian areas



Type S Bedding

Depth to soffit >1.2m in trafficked areas
Depth to soffit <0.9 in pedestrian areas

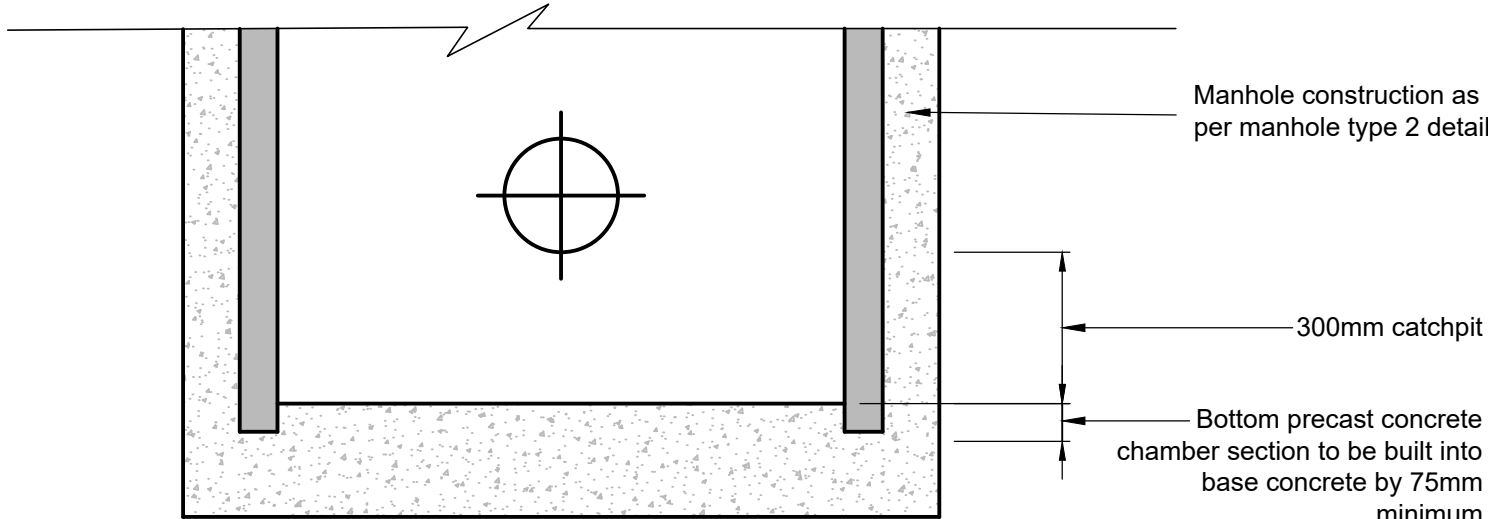
SIZING OF PIPE BEDDING		
NOMINAL BORE OF PIPE	SINGLE SIZE (mm)	GRADED (mm)
>100 to 150	10 or 14	14 to 5
>150 - 300	10, 14 or 20	14 to 5 or 20 to 5
>300 - 550	14 to 20	14 to 5 or 20 to 5
>550	14, 20 or 40	14 to 5 or 20 to 5 or 40 to 5

Pipe Bedding Details

Scale 1:10

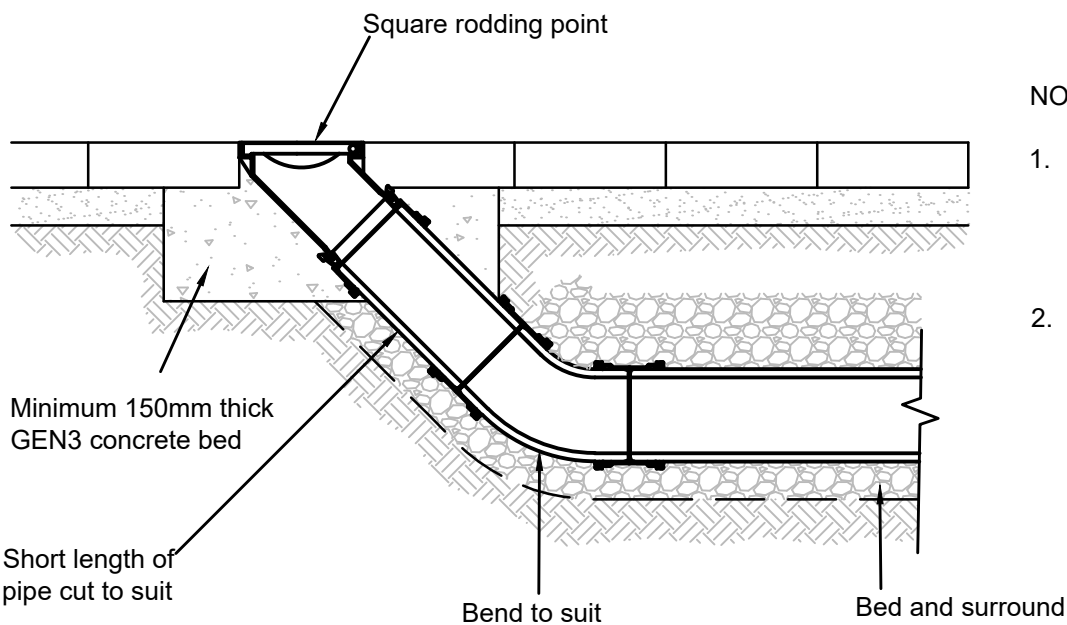
NOTES: PIPE BEDDING / TRENCHES

- Trench excavation and pipe bedding shall comply with the requirements of Sewers For Adoption.
- Prior to any excavation the position of all underground services shall be established by utility company plans and marked onsite using detection equipment and hand dug trial pits.
- Trenches shall be excavated and supported as and when necessary.
- To ensure uniform support along the run all hard or soft spots, that may cause differential settlement, shall be removed and replaced with tamped selected as-dug backfill or Type 1 granular material.
- The finished trench base shall be true to line and gradient.
- Trench bases shall be free of standing water and have minimal disturbance by foot traffic to reduce damage to the formation.
- Immediately following the excavation of the trench, the pipes shall be laid and jointed on the pipe bed. Pipes shall be laid so that each one is in contact with the bed throughout the length of its barrel.
- Trenches shall be backfilled as soon as possible to prevent injury by falls, where trenches are left open they shall be appropriately secured by barriers to prevent access into the excavation. Backfilling shall be undertaken in such manner to prevent uneven loading or damage to the pipe. Filling material shall be deposited in layers not exceeding 225 unconsolidated thickness, and then compacted to form a stable backfill. Where the excavations have been supported and the supports are to be removed, these where practicable, shall be withdrawn progressively as backfilling proceeds. The progressive withdrawal shall be done in such a manner as to minimise the danger of collapse and all voids formed behind the supports shall be carefully filled and compacted to prevent depressions forming in the final surface.
- Lever type chain cutters should be used on 100 & 150mmØ pipes with 225mmØ cut by either a double screw type chain cutter or masonry saw. cut ends shall be trimmed with a pipe trimmer / emery stone.



Catchpit Manhole Base

Scale 1:20

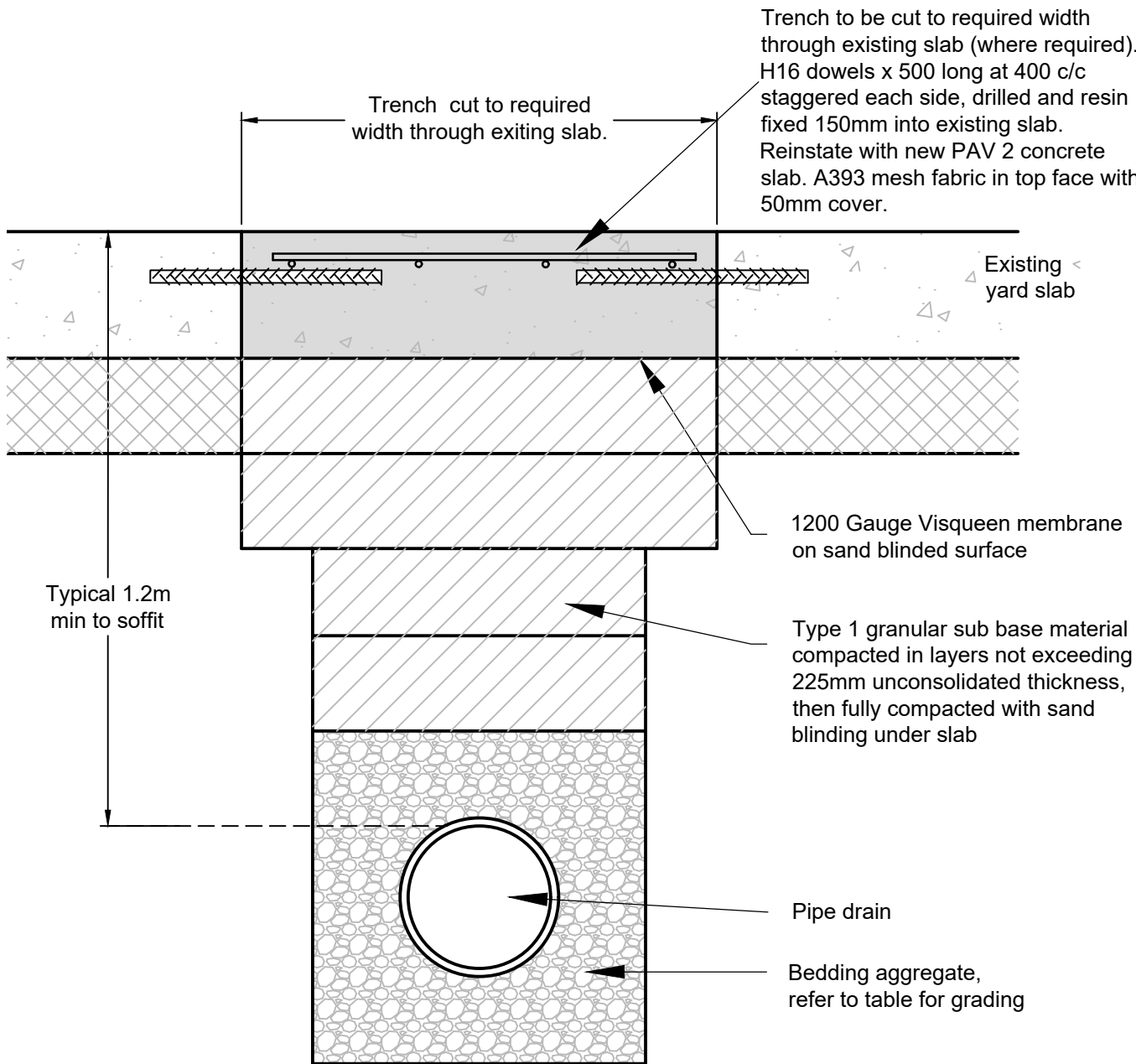


NOTES: RODDING EYE

- Access shall be installed with proprietary frames and covers of suitable strength to suit location.
- Covers shall be set to level and fall of finished surface.

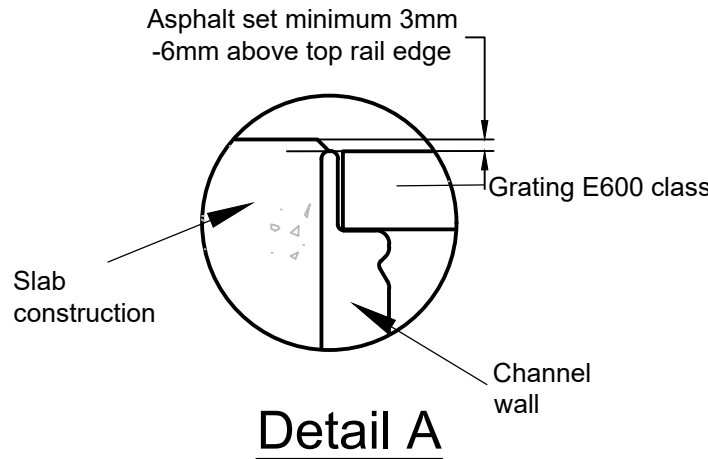
Rodding Eye Detail

Scale 1:10

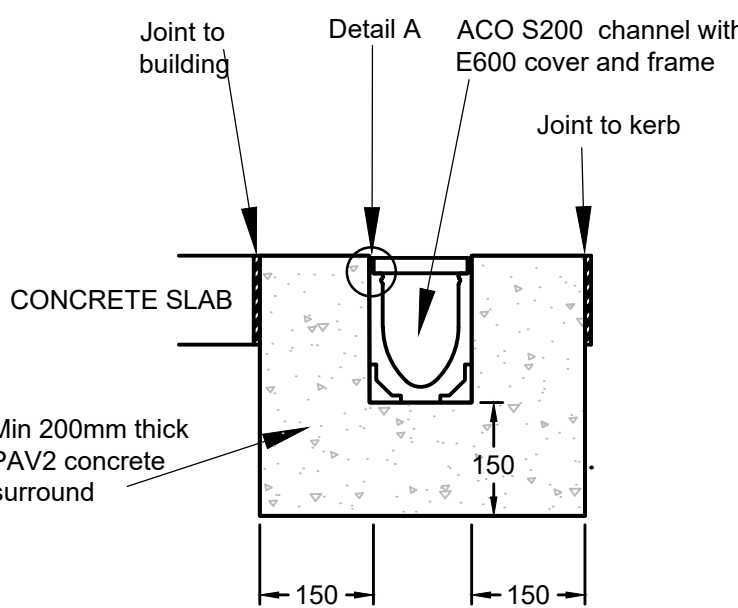


Pipe Bedding Under Existing External Concrete Slab

Scale 1:10



Detail A



Aco Channel Detail - S200

Scale 1:10

Drainage Notes

- All drainage works to be undertaken in accordance with the Sewerage Sector Guidance (latest edition).
- All private drainage works are to be undertaken in accordance with the current British Standards and Building Regulations.
- Highway construction materials and workmanship shall be in accordance with the Specification for Highway Works.
- The Principal Contractor shall be responsible for checking the existing line, invert level and pipe diameter of the existing connection points for both the foul and surface water systems, prior to undertaking installation of any new drainage works. Any deviation to the levels and positions indicated on the drawing should be brought to the immediate attention of the Engineer.
- Private pipes up to and including 300mm diameter to be PVC-U to BSEN 1401, PE to BSEN 122666-1 or clay ware to BSEN295 Class 160. Private pipes over 300mm diameter to be concrete to BS5911 Class M.
- All polypropylene inspection chambers shall be in accordance with BS7158.
- All pipe runs to be laid with flexible joints.
- Connections to be made by purpose made junctions as far as practicable, or using pipe bends to be turned in direction of flow.
- Design Sulphate Class DS-1 concrete mixes are to be used (all in accordance with BS8500). The concrete mix has been selected for BS8500 Class AC-1.
- All precast concrete products (ie pipes, manholes rings etc.) shall be of suitable concrete mix to cater for Class 2 Sulphates.
- All pipes entering and exiting manholes are to be connected with pipe soffits level unless noted otherwise.
- Pre-formed channels are to be used in manholes where applicable.
- First flexible joint in pipes adjacent to a manhole shall be a maximum of 600mm from inside face of manhole, connecting to a rocker pipe.
- The length of the rocker pipe to be 600mm for pipes between 150mm and 600mm diameter, 1000mm long for pipes between 675mm and 750mm diameter and 1250mm long for pipes 825mm diameter and over.
- Manhole covers and frames to be in accordance with the Sewerage Sector Guidance (latest edition), Adoptable frames to be 150mm deep. Frame strengths to be D400 in trafficked areas and B125 in footway and verge areas.
- Granolithic concrete benching to be steel trowelled to a dense smooth face neatly shaped and finished to all branch connections.
- The first manhole upstream from the connection to the connection to the existing drainage should be fitted with a screen in order to prevent debris entering the existing sewer/drain. The screen should not be removed until immediately prior to the occupation of the premises to be served by the sewer.
- Internal inspection chambers, manholes and access fittings to be provided with lockable double sealed manhole cover and frames grade D44 to BSEN 124.
- Road gully connections to be 150mm diameter laid at a minimum gradient of 1:150 unless noted otherwise.
- All drainage works within the zone of influence of trees are to be constructed in accordance with the NHBC standards and tree preservation offices requirements.
- All SVPS and RWPS to have a vertical rodding access positioned above ground level.
- Position and setting out of all SVPS and RWPS to be provided by the architect.
- All drains to be tested prior to backfilling, and upon completion of surfacing. All drains to be inspected by CCTV immediately prior to practical completion and at the end of the maintenance period.

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- Calculations and drawings of all Contractor designed structural items associated with NDA's design responsibilities shall be submitted to the Engineer for comment at least 10 days prior to manufacture. Any comments made by NDA do not absolve the Contractor of his design responsibilities.
- Contractor to refer to the Health and Safety File for identification of hazards, including the existence of underground and overhead utilities.
- The contractor is to work within any planning restriction including working hours, noise levels etc.
- Design and drawing information is based on the MSE topographical survey drawing number NDA4034-1-Cramlington. NDA does not accept any liability for any inaccuracies this information may contain.

P01 04/05/23 DRAFT For Tender	AF	ML
REV	DATE	AMENDMENT
CLIENT	DRW	CHK



PROJECT

Manchester Brewery
Finished Goods Warehouse

DRAWING TITLE

Drainage Details
Sheet 1 of 2

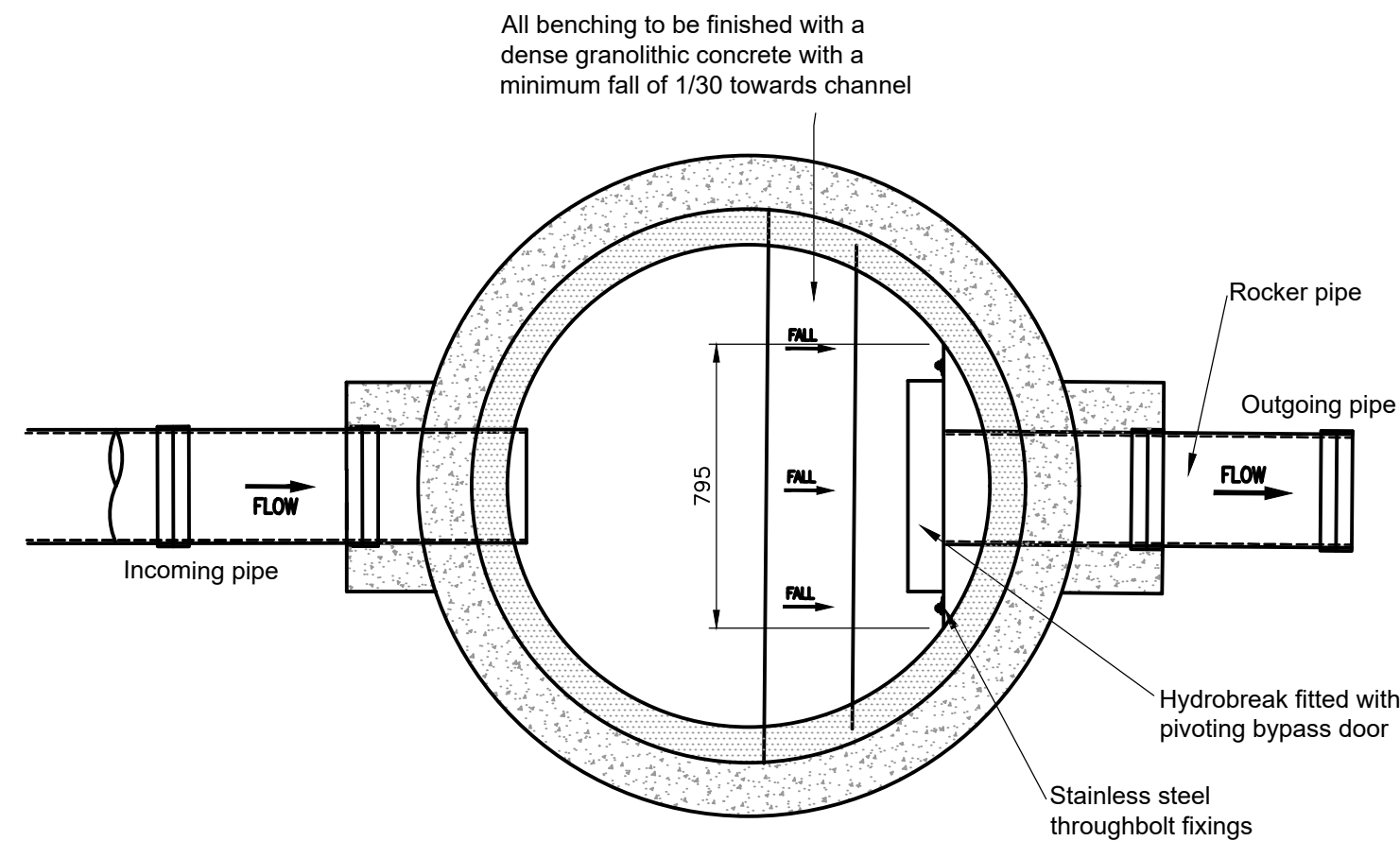
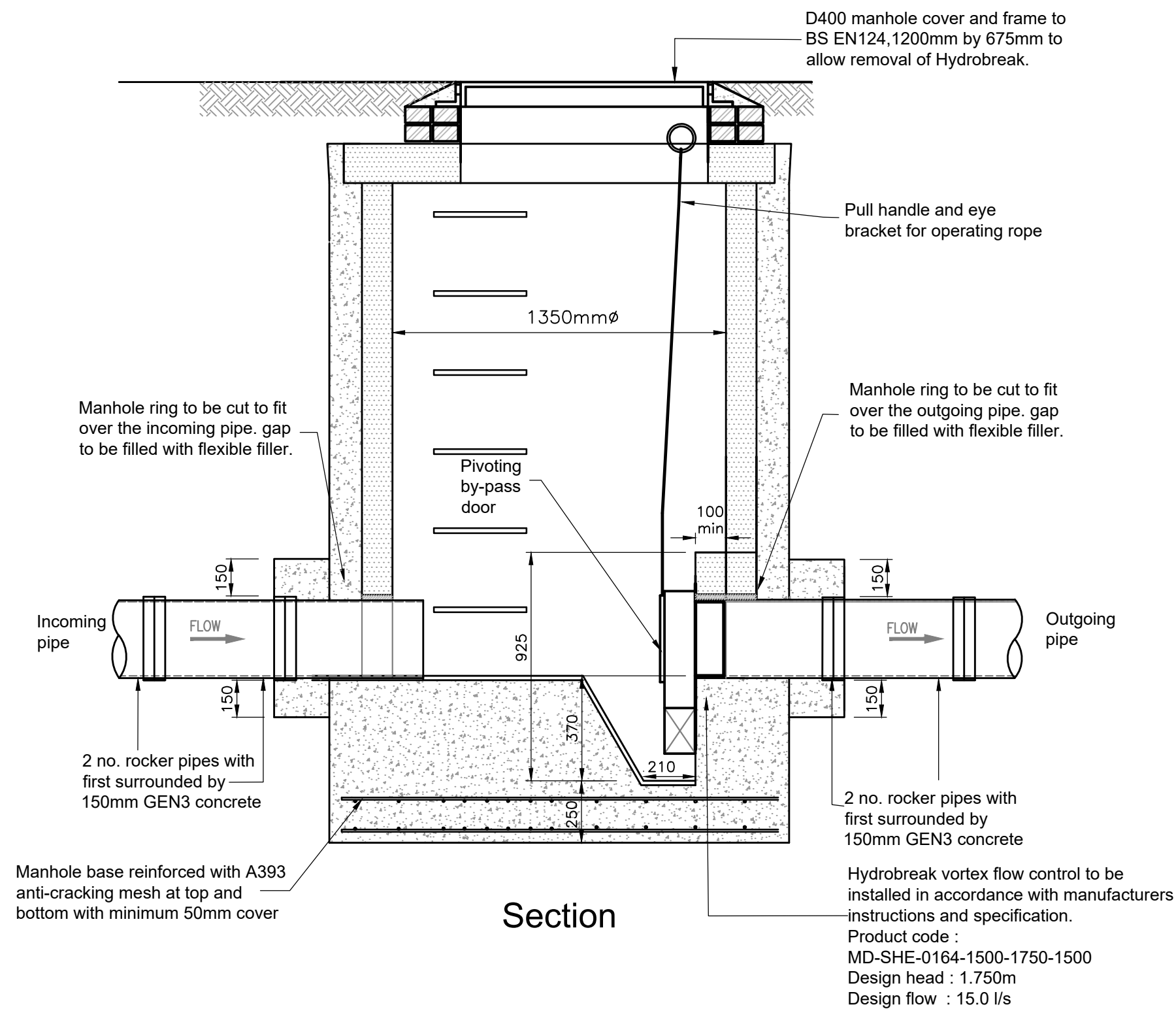


Beech Lawn, Green Lane, Belper, Derby DE56 1BY
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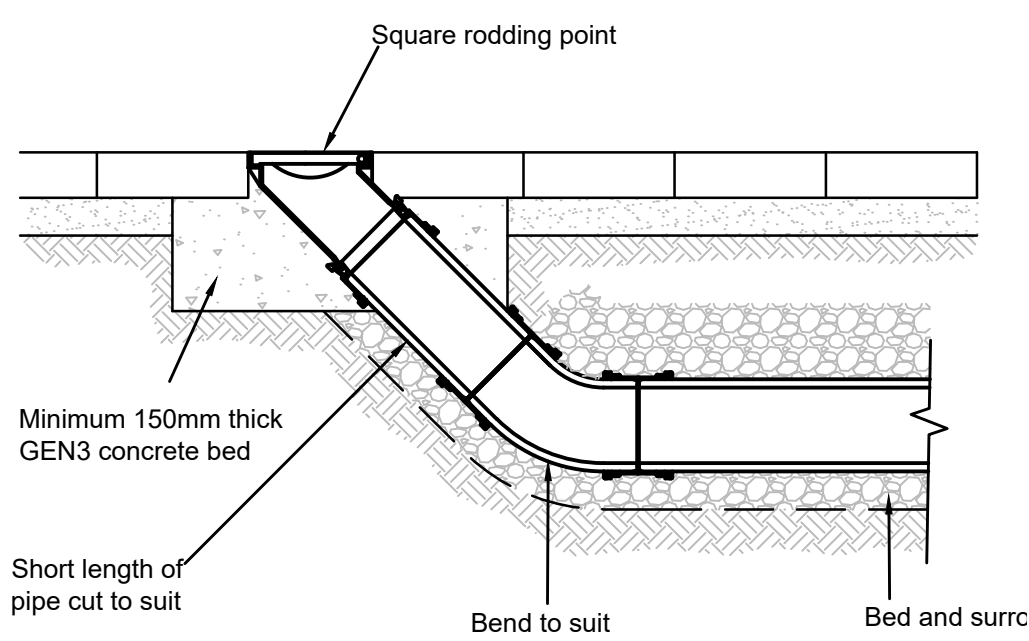
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As shown	A1	8901

SUITABILITY
S2 - Suitable for Information

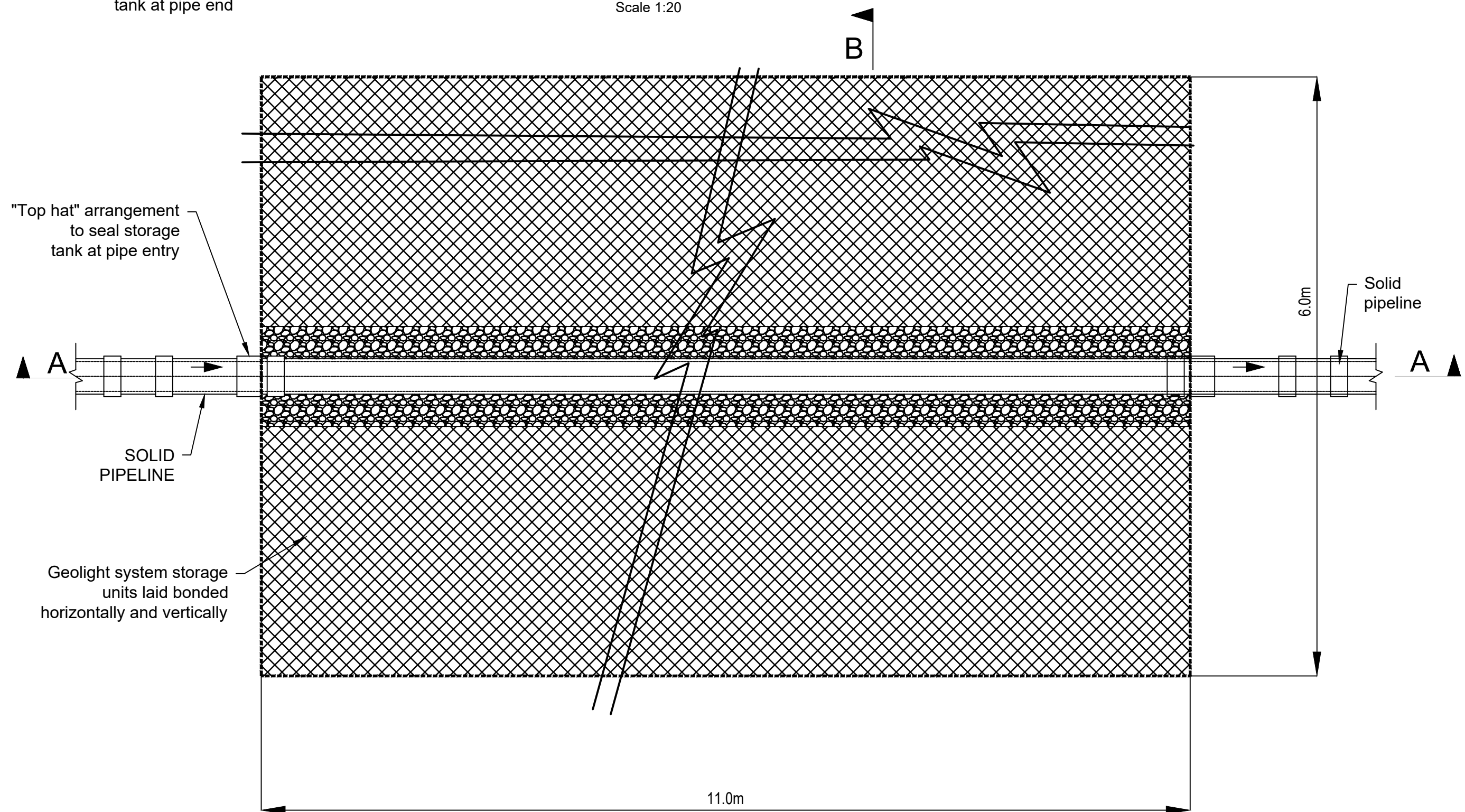
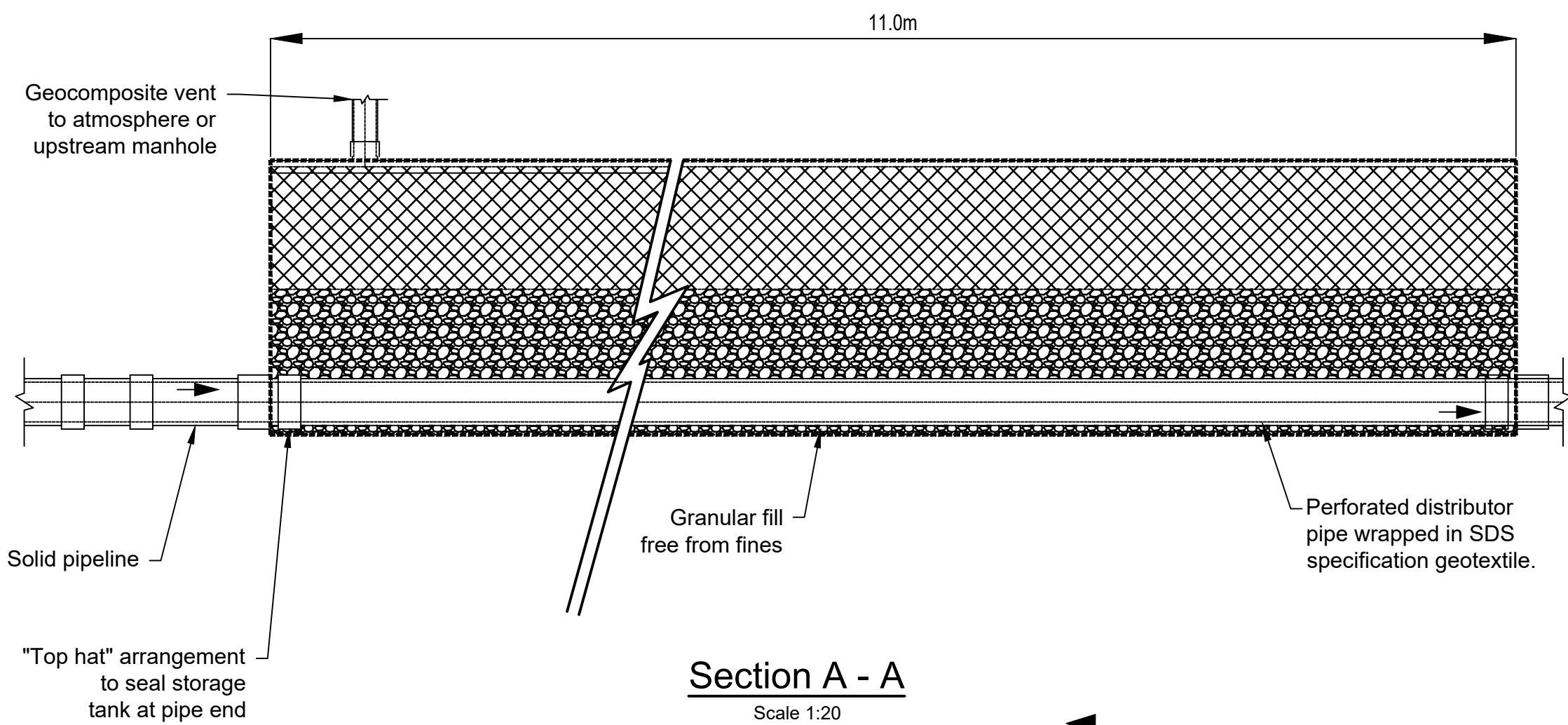
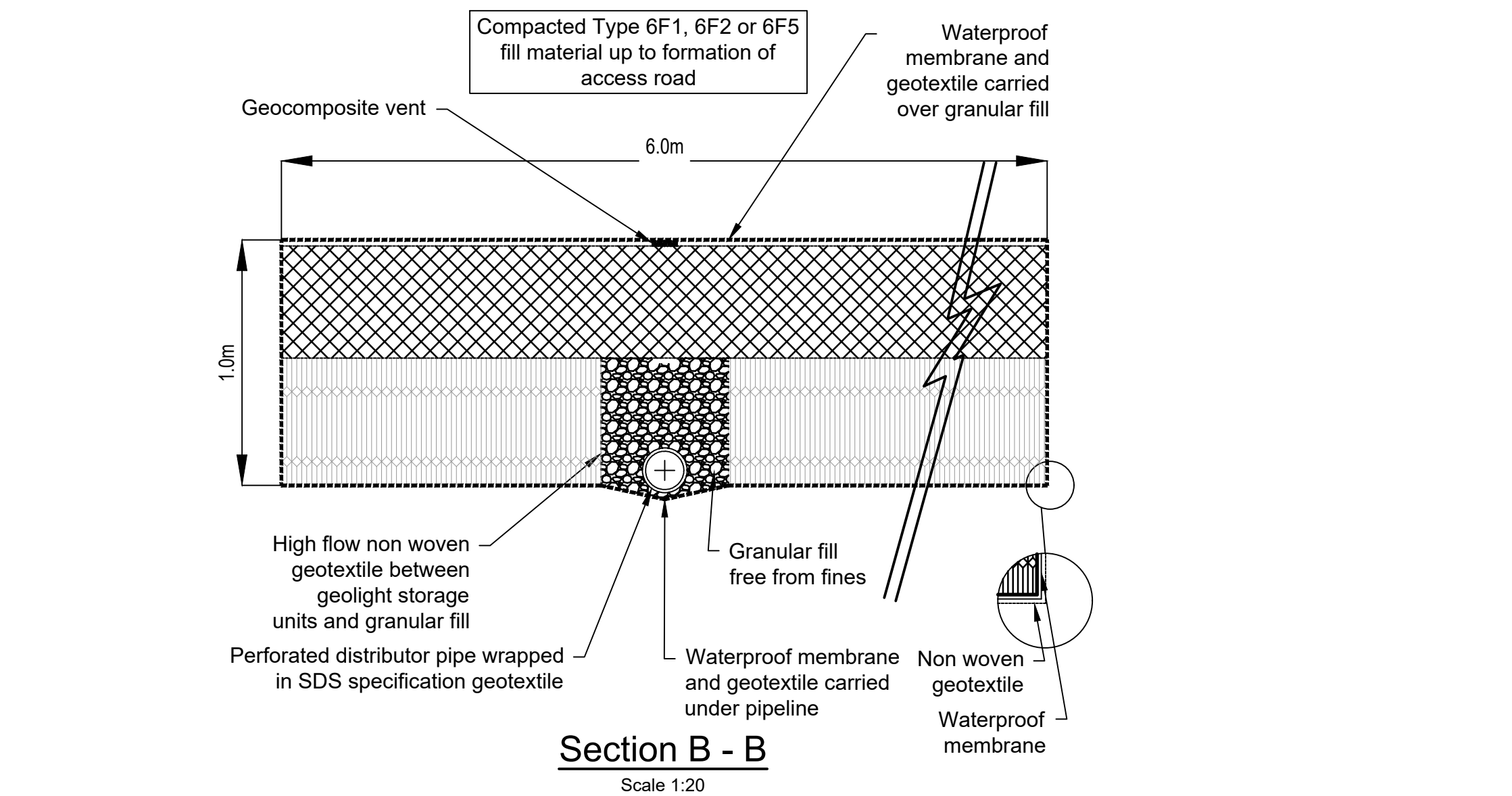
DRAWING NO.	REV
8901-NDA-XX-XX-DR-C-1202	P01



Flow Control Manhole
Scale 1:20



Rodding Eye Detail
Scale 1:10



SDS Geolight Surface Water Attenuation Crates
Scale 1:20

Attenuation Crate Notes

1. All installation works and specifications to be undertaken in accordance with SDS GEOLight requirements.

Drainage Notes

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22. Position and setting out of all SVPS and RWPS to be provided by the architect and roof designer respectively.
23. All drains to be tested prior to backfilling, and upon completion of surfacing. All drains to be inspected by CCTV immediately prior to practical completion and at the end of the maintenance period.

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P01 04/05/23 DRAFT For Tender	AF	ML
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PROJECT

**Manchester Brewery
Finished Goods Warehouse**

DRAWING TITLE

**Drainage Details
Sheet 2 of 2**



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SCALE	SHEET SIZE	NDA REF
As shown	A1	8901
SUITABILITY		
S2 - Suitable for Information		
DRAWING NO.		REV
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