
Response to Condition 5 – Drainage Scheme

Chorlton Irish Club
Chorlton-Cum-Hardy

Client Name: Southway Housing Trust

Our ref: 31296/LRD

The following request for information has been requested by the Planners in discharging of condition 5.

- Clarity required, is the soakaway a viable option? (No BRE365 results provided.)
- Evidence of drainage hierarchy not provided.
- Details of pollution controls not provided.
- Please provide proposed levels plan.
- No correspondence from UU provided.
- Update to incorporate a 45% climate change for the 1:100 + cc

In order to discharge the drainage planning condition, further information is needed to confirm that the proposed drainage design is adequate to manage flood risk.

The outstanding evidence required:

- Consideration of alternative SuDS solution (that is either utilising infiltration or attenuation) such as permeable paving if practicable;
- Evidence that the drainage system has been designed (unless an area is designated to hold and/or convey water as part of the design) so that flooding does not occur during a 1 in 100 year rainfall event with allowance for 45% climate change in any part of a building; where localised flooding occurs on site during high level storm events this should be illustrated on the drainage layout, evidencing that floodwaters are attenuated safely on site.
- Results of ground investigation carried out under Building Research Establishment Digest 365, involving filling the trial pit 3 times. Site investigations should be undertaken in the exact location and at the exact proposed depths of the proposed infiltration devices.
- 5m easements should be illustrated surrounding the proposed soakaway to ensure it is positioned appropriately.
- Groundwater monitoring should be carried out to ensure that the bottom of the soakaway is a minimum of 1m above the groundwater level (considering seasonal variation)
- Details of pollution control measures, to ensure that the soakaway does not create a potential pollution pathway to the groundwater (from the existing made ground and/or the proposed carpark).
- Correspondence from United Utilities evidencing that they are satisfied that the proposed development does not clash with the existing sewer easements.
- Construction details of flow control and SuDS elements.

Is Soakaway a viable option.

The infiltration rate obtained from a soakaway test, as provided within the Drainage Strategy submitted with the planning application, obtained an infiltration rate of 4.26E-5, which is suitable for infiltration.

Evidence of drainage hierarchy not provided.

The Drainage Strategy clearly goes through the drainage hierarchy in sections 3.0, 4.0, 5.0 and 6.0 and discusses each one in suitable detail.

Details of pollution controls not provided.

It is proposed to use green gap permeable paving for car parking spaces and a suitable product within the tree root protection zones, both are clearly indicated on Sutcliffe proposed drainage layout drawing number 31296-SUT-ZZ-XX-C-6200 Revision P06. A copy of the drawing is included within the appendix of this document. These products will allow the surface water runoff to drain directly into the sub surface and runoff will only be collected by piped drainage during excessive rainfall events where the surface water will not be able to infiltrate to ground quick enough. Hydrocarbons will be filtered out within the gravel infill where it will break down naturally.

The soakaway will also provide some pollution control as surface water infiltrates into the surrounding ground.

Please provide proposed levels plan.

See attached Sutcliffe drawing 31296-SUT-ZZ-XX-C-6150 Revision P03. A copy of the drawing is included within the appendix of this document.

No correspondence from UU provided.

Correspondence from United Utilities is included within the Drainage Strategy. A copy of the Drainage Strategy is included within the appendix of this document.

Update to incorporate a 45% climate change for the 1:100 + cc.

The calculations were originally prepared when climate change was required to be set at 40%. We have provided updated calculations to include an allowance for 45%. The drainage layout did not require updating.

To discharge the queries raised the following information has been requested and is discussed below.

Consideration of alternative SuDS solution (that is either utilising infiltration or attenuation) such as permeable pavings if practicable;

Permeable paving was considered but as attenuation will still be required to attenuate surface water from the roof then it was decided that a soakaway would be more practical.

The budget for this project is challenged due to the high land cost in obtaining the land. The parking bays are to be constructed using permeable construction and is indicated on the drainage layout.

Drainage from domestic roof's carries a low pollutant level and so a soakaway is suitable for treatment.

Evidence that the drainage system has been designed (unless an area is designated to hold and/or convey water as part of the design) so that flooding does not occur during a 1 in 100 year rainfall event with allowance for 45% climate change in any part of a building; where localised flooding occurs on site during high level storm events this should be illustrated on the drainage layout, evidencing that floodwaters are attenuated safely on site.

The drainage network has been designed to hold water underground for all storm events up to and including storm durations of 100 years + 45% climate change.

Where localised flooding occurs due to flood exceedance, which may occur as a result of a blockage or failure of the drainage network or an exceedance storm event a flood flow plan is included within the Drainage Strategy. A copy of the Drainage Strategy is included within the appendix of this document.

Results of ground investigation carried out under Building Research Establishment Digest 365, involving filling the trial pit 3 times. Site investigations should be undertaken in the exact location and at the exact proposed depths of the proposed infiltration devices.

A test in accordance with the BRE365 at the exact location of the soakaway could not be carried out at the time of the original drainage design as we did not know how were going to manage surface water for the proposed development at the time of investigation. The test carried out has been carried out in the vicinity of the proposed soakaway and ground conditions have been found to be consistent across the site.

An infiltration test in accordance with BRE365 will be required prior to confirming the exact location is as previously identified by the infiltration test to date.

5m easements should be illustrated surrounding the proposed soakaway to ensure it is positioned appropriately.

A 5m easement is clearly indicated on the proposed Sutcliffe drainage layout drawing number 31296-SUT-ZZ-XX-C-6200 Revision P06.

Groundwater monitoring should be carried out to ensure that the bottom of the soakaway is a minimum of 1m above the groundwater level (considering seasonal variation)

Ground water was not found to be consistent across the site suggesting that there are localised pockets of perched ground water. These areas of perched ground water were identified to the north and west of the site where the building is located.

The ground water was found to be 3.0m below ground levels. The soakaway is to be set at 1.8. below ground levels, providing 1.2m between the base of the

soakaway and ground water that was identified remote from the area of proposed infiltration.

No ground water was identified to the south of the site, where the soakaway is to be located.

Details of pollution control measures, to ensure that the soakaway does not create a potential pollution pathway to the groundwater (from the existing made ground and/or the proposed carpark).

Pollution control will be provided by the provision of permeable pavement to the car parking bays, sump units to the channel drainage and a sump prior to surface water entering the soakaway.

Correspondence from United Utilities evidencing that they are satisfied that the proposed development does not clash with the existing sewer easements.

We submitted a Build Over Sewer application and United Utilities have approved it. The approval from United Utilities is include in the appendix to this document.

Construction details of flow control and SuDS elements.

Drawings included in appendix of this document.

Appendix



Drainage Strategy

And

Flood Risk Assessment

for

Proposed Development

Cross Street

Chorlton

Manchester M21 9DJ

Client Name: Southway Housing trust
Our ref: LRD31296
Date: January 2022
Revision: 002



SUTCLIFFE

Drainage Strategy & Flood Risk Assessment

DOCUMENT VERIFICATION

Report Reference: LRD31296

Issue: 002

Date: January 2022

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Qualifications:	BEng(Hons) CEng FICE MCIHT CMgr FCMI C.WEM MCIWEM

Revision History:

Rev	Date	Description	Prepared	Checked	Approved
001	Jan 2022	Issued for Planning	DS	MP	DS
002	Jan 2022	Introduction amended	DS	MP	DS
003	June 2022	Amended to revised Layout	DS	MP	DS

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1.0 Introduction

The site is currently occupied with a tarmac surfaced car park. The site is located within a developed area of Chorlton-cum-Hardy, Manchester.

The site accessed off Cross Road and currently part of the car park to the Irish Centre.

The proposed scheme consists of a 4-storey block of residential flats with associated parking.

The total site area is approximately 0.2 Ha and its location can be found by the following references;

Postcode M21 9DJ

Coordinates 381721, 393601

Grid reference SJ 81721 93601

The site has a level difference of up to 0.3m from west to east and 0.2m from north to south.

The purpose of the strategy and flood risk assessment is to consider how the surface water runoff from the development site will be effectually drained and managed throughout the lifetime of the development.

The site must, in the first instance, consider the use of SUDS in accordance with the National Planning Policy Framework.

SUDS are the provision of a surface water drainage scheme that manages the surface water runoff at source.

The hierarchy of managing surface water at source is:

- Infiltration to ground
- Discharge to a suitable water course
- Discharge to a suitable surface water sewer network
- Discharge to a combined sewer network

The following statements are made in relation to the above stated hierarchy in relation to the proposed scheme.

2.0 Existing Flood Risk

An inspection of the Environment Agency flood maps was made on the internet and the following can be stated.

- That the site is located within a Zone 1 low risk flood area and the site is, therefore, considered not to be at risk of flooding from rivers or sea.
- That the site is indicated as having a low risk of surface water flooding. The area of surface water flooding is shown to the existing car park area.
- That the site is at risk from flooding from reservoirs.

Flooding from reservoirs is very rare due to the legislation in place requiring regular inspections to be made of structures retaining water. It is very unlikely that a sudden failure of a dam, or water retaining structure, will occur and it is likely that there would be adequate warning in place to allow evacuation should it be required.

Therefore, from the above observations we confirm that the site is at low risk of flooding from existing sources.

3.0 Infiltration to ground

Infiltration to ground is, in the main, provided by the provision of soakaways and permeable paving.

Soakaways can be provided where the ground has a suitable infiltration rate and where there are no limiting factors that would preclude the use of soakaways.

Infiltration tests have been carried out and a suitable infiltration rate was calculated.

Therefore, the use of soakaways is possible for this site.

4.0 Discharge to suitable water course

An inspection of historic / current maps and the Environment Agency maps reveal that there are no watercourses either on, adjacent to, or within reasonable distance of the site.

Therefore, discharge of surface water into a water course is not available for this site.

5.0 Discharge to a suitable surface water network

A review of the existing drainage network within the surroundings of the development site was made and of United Utility sewer records. A drainage investigation of the on- site drainage was also undertaken.

No surface water drains or sewers were identified.

Therefore, disposal of surface water drainage into a surface water drain, sewer or highway drain is not available for this site.

6.0 Discharge to a suitable combined sewer network

A review of the existing drainage network and of United Utility sewer records within the surroundings of the development site was made.

The review indicated that there are available combined sewers in the vicinity of the site.

Therefore, discharge of surface water into a combined sewer is available for this site.

7.0 Existing Site Surface Water Drainage

The existing surface water drainage for the existing car park is a not able to be properly calculated as one of the road gullies draining the car park was not identified during the CCTV drainage survey and so assumed to go to ground.

Greenfield run-off rates have been calculated using Micro Drainage software as being;

$$Q_{\text{bar}} = 1.3 \text{ l/s}$$

$$Q_{1 \text{ year}} = 1.2 \text{ l/s}$$

$$Q_{2 \text{ year}} = 1.4 \text{ l/s}$$

$$Q_{30 \text{ year}} = 1.9 \text{ l/s}$$

$$Q_{100 \text{ year}} = 2.1 \text{ l/s}$$

A copy of the hydraulic calculations is included in Appendix D.

8.0 Site Surface Water Runoff Drainage Proposals

From the above discussions made the following surface water drainage strategy is proposed.

Infiltration via soakaways is possible as a suitable infiltration test result was proved.

There are no watercourses within reasonable and economic distance of the site.

There is no existing surface water sewer on or adjacent to the site.

There are combined sewers available but will not be used as infiltration can be used to manage surface water disposal for this site.

United Utilities have been consulted and responded requesting we followed the surface water drainage hierarchy. Should the hierarchy be that discharge to a sewer was required then a connection to a combined sewer would be allowed at a discharge rate of no more than 5.0 l/s.

Therefore, from the above discussion it is proposed to provide soakaway drainage to drain surface water runoff from the site.

The soakaway will be designed to cater for all rainfall events up to and including the 100 year + 40% climate change.

No allowance for urban creep is proposed as the development is flats and urban creep is not required to be considered.

9.0 Site Surface Water Runoff Drainage Design

The site is located in an area where SUDS is to be utilised as ground conditions allow for infiltration.

From the discussion in the previous section, we propose to provide a soakaway suitably designed to manage surface water runoff for storm events up to and including the 100-year + 40% climate change storm events.

From the above discussion, it is our proposal to provide the following surface water drainage proposal;

To provide a private drainage network to collect and discharge surface water to a soakaway that will discharge the surface water into the underlying sands. The soakaway will be provided to attenuate all storm events up to and including the 100 year + 40% climate change storm event.

Hydraulic calculations undertaken for the proposed sewer network design using Micro Drainage provide evidence that the various storm events can be

managed within the proposed drainage network and reduce future flood risk both on and off site.

Calculations for the following rainfall events were interrogated;

- 1 Year
- 2 Year
- 30 Year
- 100 year + 20% Climate Change
- 100 year + 30% Climate Change
- 100 year + 40% Climate Change

No flooding occurred for any of the storm events up to and including the 100 year + 20% climate change.

There was minor flooding for the 100 year +30% and 40% events with flooding volumes not exceeding 0.875m³. The extent of flooding was interrogated on the 3D model and the flooding is contained within the confines of the car park and does not leave site or affect property. The extent of flooding is indicated on the flood routing drawing 32196-SUT-ZZ-XX-DR-C-6201.

The maximum half drain down time for the soakaway is 431minutes, which is acceptable.

Hydraulic calculations are included in Appendix E.

A proposed drainage plan, 32196-SUT-ZZ-XX-DR-C-6200, has been prepared in accordance with the above proposals and is included in Appendix C.

In the event of an exceedance storm event (greater than 100 year + 40% climate change) or a drainage failure event, such as a blockage, then overland flooding could be expected. Overland flood routes have been indicated on the site plan and indicate where the water will flow. No dwellings, either on or off site are affected. The flood routing drawing, 32047-SUT-ZZ-XX-DR-C-6201, is included in Appendix C.

From the above we believe that the requirements of the Lead Local Flood Authority have been met to manage surface water runoff from the development on the site.

10.0 Outstanding Flood Risk

From the above drainage strategy and flood risk assessment we confirm that the site is at a low risk of flooding from on site and external sources.

The drainage design has reduced current flood risk by retaining all surface water on site and discharging it via soakaway rather than a positive piped drainage network.

In the event of exceedance rainfall events and / or blocked drains, flooding is likely to occur as the drainage system becomes overwhelmed. A flood exceedance flood route is indicated on drawing 32196-SUT-ZZ-XX-DR-C-6201.

Therefore, the proposed development has a low risk of flooding both on and off site and reduces flood risk from the previous development and has been designed to retain and manage surface water discharge on site for up to and including the 100 year + 40% climate change storm events.

11.0 Future Surface Water Drainage Management and Maintenance

At this point of the design process, we are not able to provide a full management and maintenance plan but the following summary can be provided to give a general strategy of future management and maintenance. As the development proceeds to full design and construction stage the management and maintenance strategy can be developed.

At this stage of design, it is anticipated that the drainage within the proposed development will remain privately owned and maintained by Southway Housing Trust, who will be responsible for ongoing maintenance throughout the lifetime of the development.

The Housing Association will be responsible for the whole of the drainage network as it will remain privately owned.

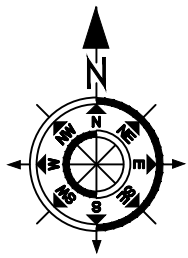
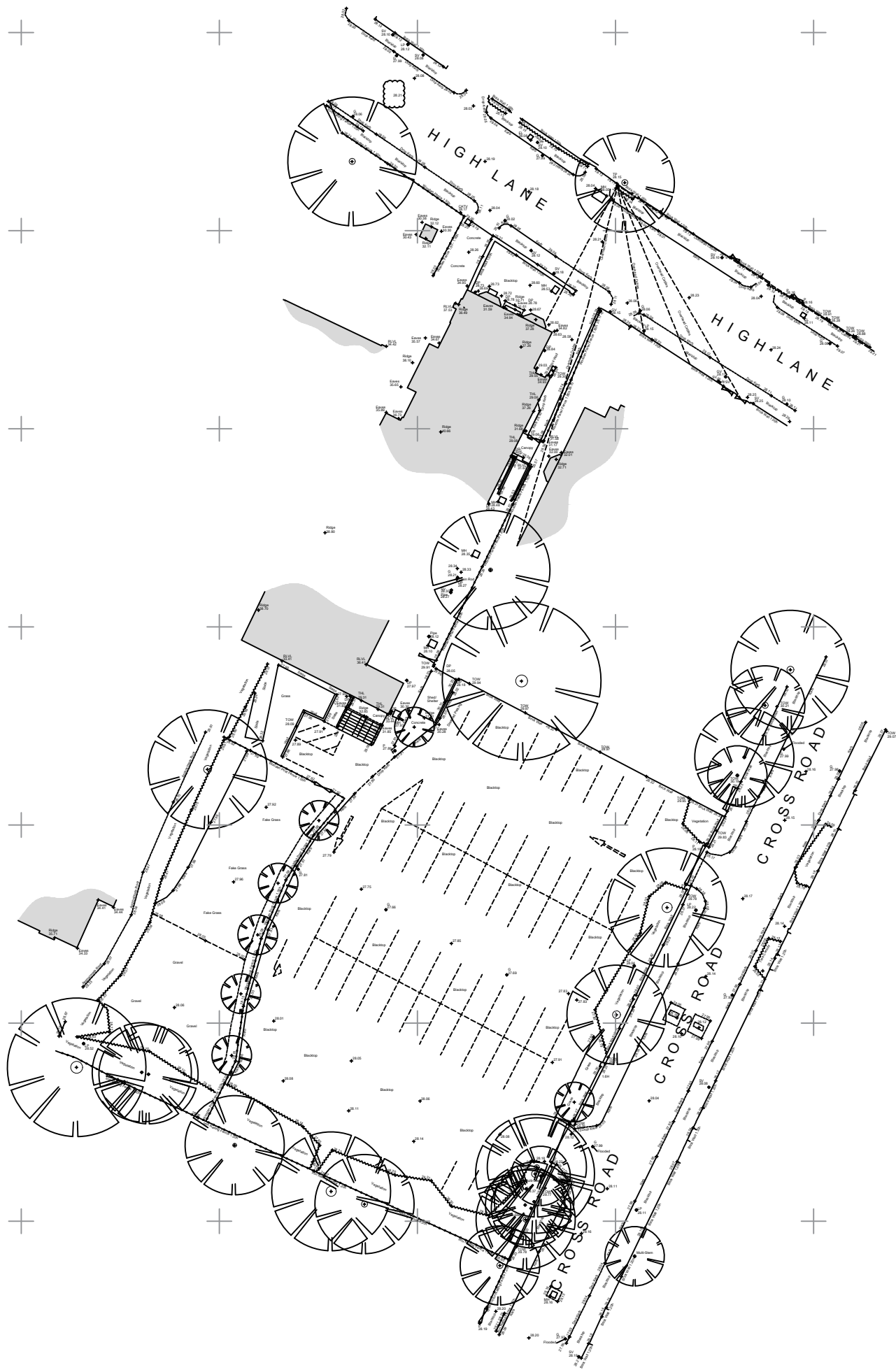
It is anticipated that the costs for ongoing maintenance will be charged via a management fee that tenants contribute towards as part of their rent. The Housing Association will ensure that the private drainage is maintained to the point of discharge into the adopted sewers.

The maintenance schedule will be developed once planning is awarded and the drainage design is finalised.

Appendix A

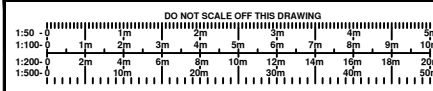
Existing Site Layout

Proposed Site Layout



Topographical Survey Abbreviations			
AC	AIR CONDITIONING UNIT	LP	LAMP POST
B	BOLLARD	MKR	MARKER POST
BB	BELLISHA BEACON	MH	MANHOLE
BED	BED LEVEL	OHC	OVERHEAD CABLE
BH	BOREHOLE (WITH No.)	PM	PARKING METER
BS	BUS STOP	P	POST
BT	BRITISH TELECOM COVER	RE	RODDING EYE
CATV	CABLE TELEVISION COVER	RS	ROAD SIGN
CC	CONTROL CABINET	RWL	RETAINING WALL
CL	COVER LEVEL (MANHOLE)	SA	SOAK AWAY
COL	COLUMN	SCAM	SECURITY CAMERA
CP	CATCH PIT	SG	STRIP GULLY
DP	RAIN WATER DOWN PIPE	SP	SIGN POST
EC	ELECTRIC CABLE	SV	STOP VALVE
EL	ELECTRIC COVER	TAP	WATER TAP
EP	ELECTRIC POLE	TCB	TELEPHONE CALL BACK
ER	EARTH ROD	THL	THRESHOLD LEVEL
FFL	FINISHED FLOOR LEVEL	TL	TRAFFIC LIGHT
FH	FIRE HYDRANT	TOF	TOP OF FENCE LEVEL
FLT	FLOODLIGHT	TOW	TOP OF WALL LEVEL
G	GULLY	TP	TELEGRAPH POLE
GP	GATE POST	TPIT	TRIAL PIT (WITH No.)
GV	GAS VALVE	VP	VENT PIPE
IC	INSPECTION CHAMBER	WLV	WATER LEVEL
IL	INVERT LEVEL	WM	WATER METER
KO	KERB OUTLET	WO	WASH OUT
LB	LITTER BIN		
 BUILDINGS			
 OPEN SIDED BUILDINGS			
 TREE		SPREAD & DIA. TO SCALE	

- Technical Notes:**
- All survey levels and co-ordinates are related to OS Datum using the GPS Active Network. The Grid is orientated to Grid North with a Scale Factor of 1.00.
 - All Boundaries surveyed are physical features. Please bear in mind that these may not represent the legally conveyed ownership.
 - Trees are drawn to scale showing the average canopy spread and are approximate only. Where heights are shown they have been taken from ground level and are an estimate only.
 - All underground features have been measured from the surface, therefore pipe sizes, depths etc. are only an estimate or assumption. If dimensions are critical information must be checked and verified prior to work commencing.
 - Whilst every effort has been made to locate all physical features during the survey no responsibility can be taken where features are obscured or hidden at the time of survey. This is especially important where high volumes of plant or vehicles are present on site.
 - Off site features may have been measured remotely and as such may not show the full detail of the feature due to limited access or obstructions with line of sight.
 - All critical dimensions including levels should be checked prior to construction. Any errors or discrepancies should be reported immediately.
 - All measurements have been taken from ground level only.
 - Do not scale from this drawing.
 - Tree Information
 - Trunk Diameters are measured where possible at approximately 1.5m above ground.
 - Tree Spreads are indicative only and are representative of the generalised canopy size, and therefore may not represent the true canopy position as outlined on site. Tree boles may also not sit centrally to the canopy.
 - Where tree heights are shown they are an approximate height to the top of the canopy and are an estimation from ground level.
 - Attention should be paid to the original survey brief as to which trees have actually been surveyed; i.e. Only Trees larger than "X"mm diameter, or Only Trees within the site.



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SURVEYS

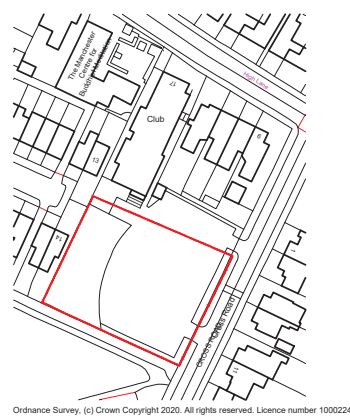
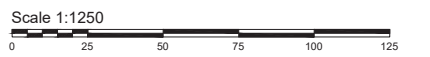
Client: View Associates

Project: High Lane / Cross Street
Chorlton
Manchester

Drawing Title: Topographical Survey

Rev.	Description	Surveyed	Approved	Date
SB	VM	AJS	24/08/20	02/09/20
DRAFT	1:200	A1	1 of 1	
Project No:	Drawing No:	Revision:		
RBS - 20/2061	RBS - 20/2061/001	-		

Do not scale from this drawing All dimensions and levels are to be checked on site. All areas are approximate and are for information only. This drawings is to be read in conjunction with all other relevant documents and all works are to comply with the current Building regulations, British Standards and Codes of Practice etc. All details of the existing structure are to be verified by others on site prior to any works commencing. The copyright of this drawing and all works executed from this drawing remain the property of View Associates Ltd. Any 3rd party survey information contained on this drawing is not the responsibility of View Associates Ltd



Revision	Description	Date

Drawing Number: (PL)001 REV B	Date: 22.12.21
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Stage: PLANNING

Scale: 1:200 + 1:1250 @ A1

Drawn By: HH	Checked By: IJ
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Client: Southway Housing Trust
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Drawing Title: PROPOSED SITE PLAN + LOCATION PLAN



ASSOCIATES
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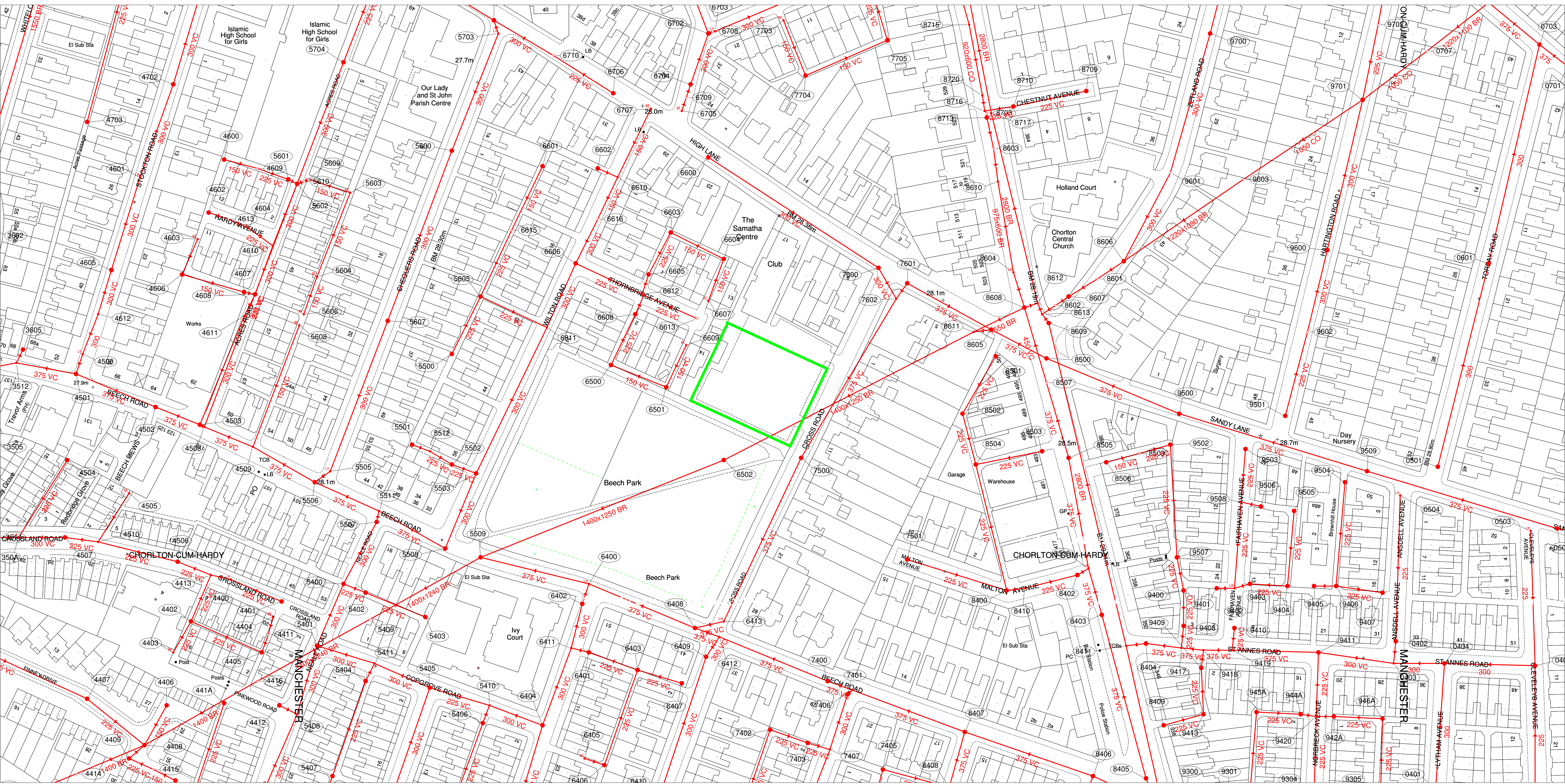
Cross Road, Chorlton

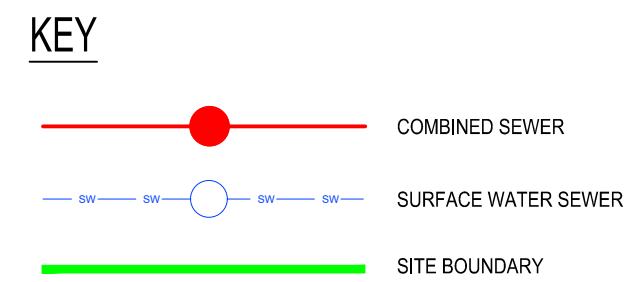


Appendix B

Existing United Utilities Sewer Records

Existing Drainage Plan





P02	SITE BOUNDARY UPDATED.		29.11.21	AJB	DS
P01	ISSUED FOR COMMENTS.		16.11.21	MG	DS
Rev	Description		Date	By	Chk
Date	Drawn By		Checked By		
16.11.2021	MG		DS		
Scale at A3 1:500			Project Number 31296		
Sheet Number 31296-SUT-ZZ-XX-DR-C-600-0001				Revision P02	

Appendix C

Proposed Drainage Layout

Proposed Flood Routing

ALL INTERNAL SP/SVP POSITIONS TO BE CONFIRMED BY ARCHITECT PRIOR TO CONSTRUCTION OF GROUND FLOOR SLAB



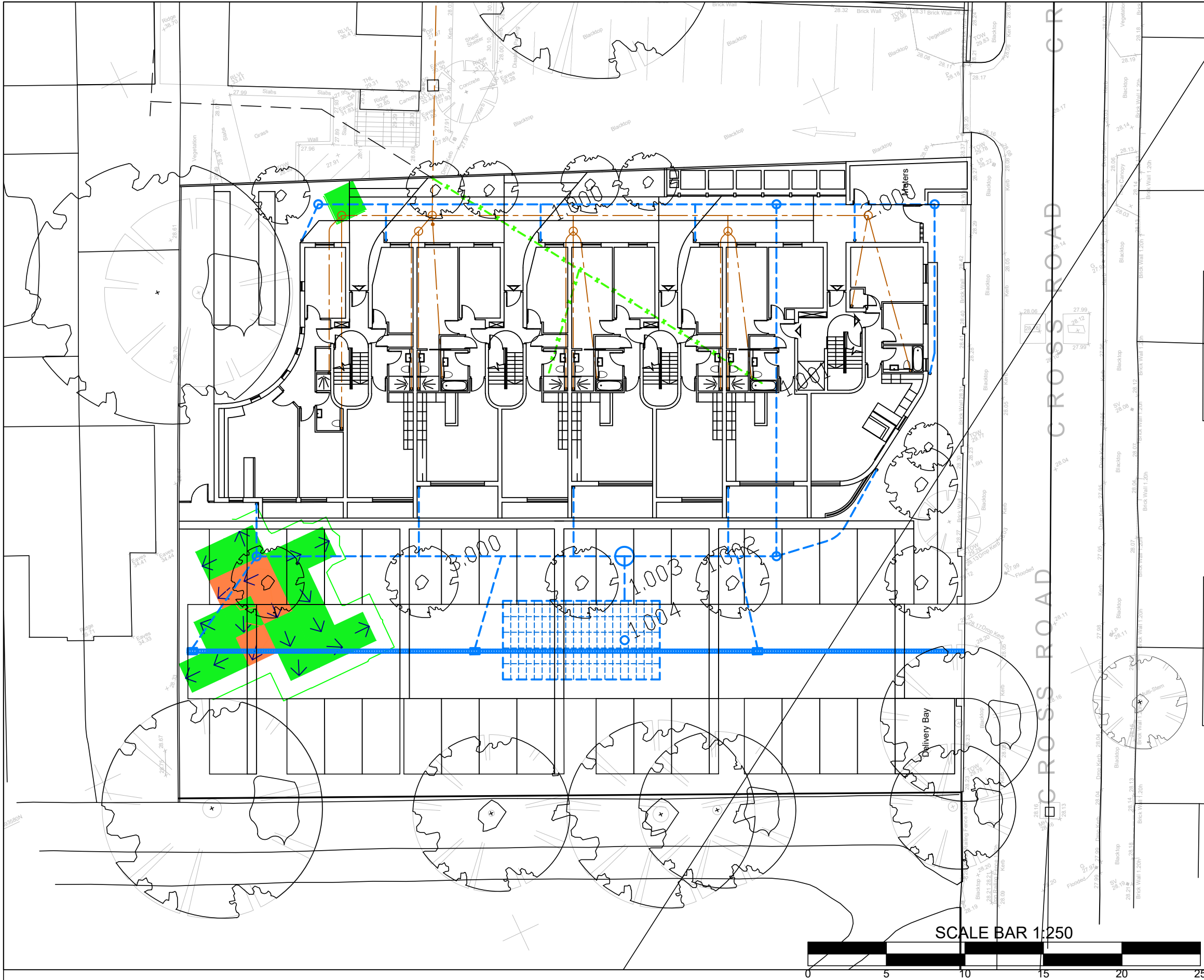
Sheet Number	Revision
31296-SUT-ZZ-XX-DR-C-6200	P03



NOTE:
SP POSITIONS HAVE BEEN ASSUMED BASED
ON ARCHITECTS PLANNING DRAWINGS DATED
OCTOBER 2021.
RWP LOCATIONS TO BE CONFIRMED BY
ARCHITECT.

EX MH 7500
CL: 28.16
IL: 24.63





General Notes

DO NOT SCALE OFF THIS DRAWING

THIS DRAWING TO READ IN CONJUNCTION WITH ALL RELEVANT STRUCTURAL AND ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.

ALL DIMENSIONS TO BE CHECKED ON SITE BY THE CONTRACTOR / FABRICATOR PRIOR TO COMMENCEMENT OF WORKS.

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS STATED OTHERWISE.

ALL WORKS TO BE CARRIED OUT IN STRICT ACCORDANCE WITH THE ENGINEER'S SPECIFICATIONS, RELEVANT BRITISH STANDARDS, STATUTORY HEALTH AND SAFETY/C.D.M. REGULATIONS, RELEVANT CODES OF PRACTICE AND THE CURRENT EDITION OF THE BUILDING REGULATIONS.

FOR FINAL SETTING OUT INFORMATION RELATING TO GRID LINES AND WALL POSITIONS REFER TO THE ARCHITECT'S DRAWINGS.

KEY:

-  DEPTH OF WATER < 25MM
-  DEPTH OF WATER 25 - 100MM
-  DEPTH OF WATER >100MM
-  DIRECTION OF SURFACE WATER FLOOD FLOW



Sutcliffe

Structural & Civil Engineering
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Client
SOUTHWAY HOUSING TRUST

Project
**CHORLTON IRISH CLUB
CHORLTON-CUM-HARDY**

Drawing Status
PRELIMINARY DRAWING

Drawing Title
FLOOD FLOW PLAN

Scale at A3
1:250

Project Number
31296

Sheet Number
31296-SUT-ZZ-XX-DR-C-6201

Revision
P02

P02	AMENDED TO NEW LAYOUT	09.06.22	AJB	DS
P01	PRELIMINARY DRAWING	07.01.22	AJB	DS
Rev	Description	Date	By	Chkd
		JAN 2022	AJB	DS
Scale at A3 1:250		Project Number 31296		
Sheet Number 31296-SUT-ZZ-XX-DR-C-6201				Revision P02

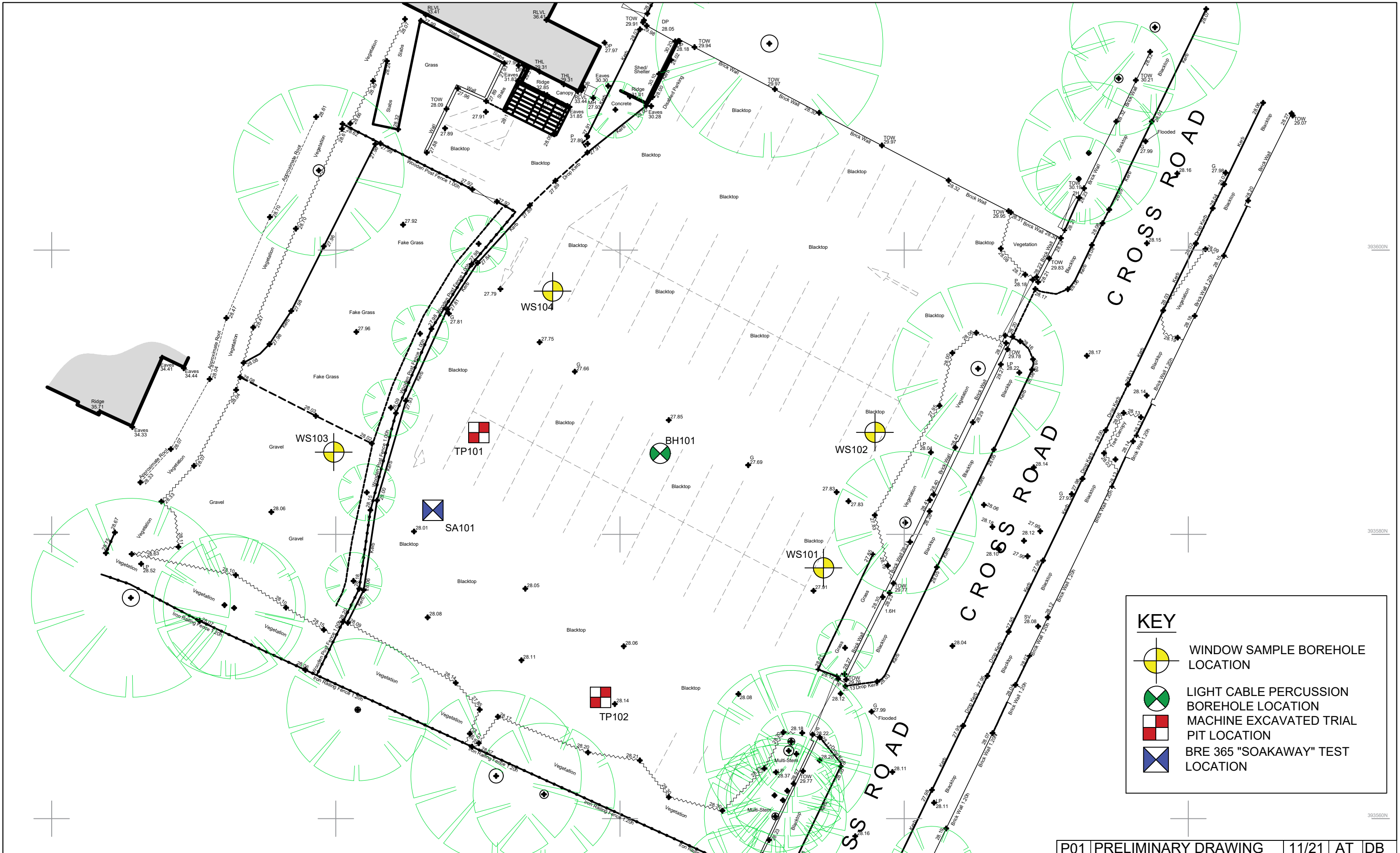
Appendix D

Investigation Location Plan

Borehole Logs

Trial Hole Logs

Infiltration Test Result



KEY

WINDOW SAMPLE BOREHOLE LOCATION

LIGHT CABLE PERCUSSION BOREHOLE LOCATION

MACHINE EXCAVATED TRIAL PIT LOCATION

BRE 365 "SOAKAWAY" TEST LOCATION

P01	PRELIMINARY DRAWING	11/21	AT	DB
Rev	Description	Date	By	Chkd
Date	Nov 2021	Drawn By	A TOSH	Checked By
Scale at A3	NTS	Project Number	31296LG	
Sheet Number	31296-SUT-ZZ-00-DR-G-720-0001	Revision	P01	

Structural & Civil Engineering

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Building Surveying

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London Office: 020 8016 4320

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Drawing Status

FOR INFORMATION

Client

SOUTHWAY HOUSING TRUST

Project

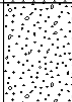
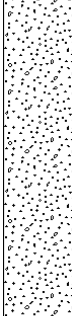
CHORLTON IRISH CLUB
CROSS ROAD, CHORLTON

Drawing Title

SITE INVESTIGATION LOCATION
ON EXISTING PLAN

<div> Sutcliffe</div>						Site Chorlton Irish Club		Number WS101	
Excavation Method Drive-in Windowless Sampler		Dimensions		Ground Level (mOD)		Client Southway Devco Ltd		Job Number 31296LG	
		Location		Dates 17/05/2021		Engineer Michael Wiggins, Jack Smith		Sheet 1/1	
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Legend	Water
0.30-0.50	ES1				<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div><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<div> Sutcliffe</div>						Site Chorlton Irish Club		Number WS102	
Excavation Method Drive-in Windowless Sampler		Dimensions		Ground Level (mOD)		Client Southway Devco Ltd		Job Number 31296LG	
		Location				Dates 17/05/2021		Engineer Michael Wiggins, Jack Smith	
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Legend	Water
0.10-0.30	ES1				(0.10) (0.20) 0.30 (0.20) 0.50	(MADE GROUND) Tarmac.			
0.50-1.00	B1					(MADE GROUND) Black, sandy, angular-subangular, fine-coarse GRAVEL, of tarmac. Sand is fine-coarse.			
						(MADE GROUND) Dark brown, very gravelly, ashy, fine-coarse SAND. Gravel is fine-coarse, angular-subangular, of brick. Occasional fine-coarse clinker.			
1.20-1.65	SPT N=8		2,2/2,2,2,2		(2.00)	Light brown, slightly gravelly, fine-medium SAND. Gravel is fine-medium, rounded-subrounded, of natural stone.			
2.00-2.45	SPT N=11		2,3/3,2,3,3						
3.00-3.45	SPT N=14		Water strike(1) at 3.00m. 3,3/4,3,3,4		2.50 (0.80) 3.30 (0.20) 3.50 (0.50) 4.00 (0.40) 4.40 (0.60) 5.00	Orangey brown, clayey, gravelly, slightly cobbly, fine-coarse SAND. Gravel is fine-coarse, subangular-subrounded, of natural stone. Cobbles are subrounded-rounded, of natural stone.			V1
						Firm-stiff, brown, slightly sandy, slightly gravelly CLAY. Gravel is fine-medium, subrounded-rounded, of natural stone.			
						Stiff-very stiff, brown, slightly sandy, slightly gravelly CLAY. Gravel is fine-medium, subrounded-rounded, of natural stone. Sand is fine-medium.			
						Light brown, gravelly, fine-coarse, wet SAND. Gravel is fine-medium, rounded, of mixed lithology.			
						Firm-stiff, light brown, very sandy, slightly gravelly CLAY. Sand is fine-medium, gravel is fine-medium, subrounded-rounded, of natural stone.			
						Complete at 5.00m			
Remarks Drilling contractor: VC Drilling. Took barrel to 4.00m, hole collapsed to 3.00m, barrel sent back down to 5.00m.								Scale (approx) 1:40	Logged By JS
								Figure No. 31296LG.WS102	

<div> Sutcliffe</div>						Site Chorlton Irish Club		Number WS103	
Excavation Method Drive-in Windowless Sampler		Dimensions		Ground Level (mOD)		Client Southway Devco Ltd		Job Number 31296LG	
		Location				Engineer Michael Wiggins, Jack Smith		Sheet 1/1	
				Dates 17/05/2021					
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Legend	Water
0.25-0.70	ES1				(0.10)	(MADE GROUND) Greyish white, sandy, fine-coarse, subangular-subrounded GRAVEL, of natural stone. Sand is fine-medium.			
					(0.15)				
					(0.25)				
1.20-1.65	SPT N=8		2,2/2,2,2,2		(0.45)	(MADE GROUND) Dark brown blackish, sandy, angular-subangular, fine-coarse GRAVEL, of tarmac. Sand is fine-coarse. Slight hydrocarbon odour.			
					0.70				
2.00-2.45	SPT N=18		2,3/4,4,5,5			(MADE GROUND) Dark brown, slightly silty, slightly gravelly, fine-coarse SAND. Gravel is fine-coarse, angular-subangular, of brick and natural stone with occasional glass.			
3.00-3.45	SPT N=13		Water strike(1) at 3.00m. 3,3/4,3,3,3		(3.30)	Light brown, slightly gravelly, fine-coarse SAND. Gravel is fine-medium, subrounded-rounded, of natural stone.			
4.00-4.45	SPT N=10		4,3/3,2,2,3		4.00	Complete at 4.45m			
Remarks Drilling contractor: VC Drilling. Hole collapsed from 4.00m to 3.00m after SPT.								Scale (approx) 1:40	Logged By MW
								Figure No. 31296LG.WS103	

						Site Chorlton Irish Club		Number WS104	
Excavation Method Drive-in Windowless Sampler		Dimensions		Ground Level (mOD)		Client Southway Devco Ltd		Job Number 31296LG	
		Location		Dates 17/05/2021		Engineer Michael Wiggins, Jack Smith		Sheet 1/1	
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description		Legend	Water
0.30-0.70 0.30-0.70	B1 ES1				(0.10)	(MADE GROUND) Tarmac.			
					(0.20)				
					0.30	(MADE GROUND) Dark brown, gravelly, fine-coarse SAND. Gravel is fine-coarse, angular, of brick, tarmac and natural stone.			
					(0.40)				
					0.70	(MADE GROUND) Dark brown, slightly silty, slightly gravelly, fine-medium SAND. Gravel is fine-coarse, angular-subangular, of brick and natural stone.			
1.20-1.65	SPT N=8		1,2/2,2,2,2			Light brown, slightly gravelly, fine-coarse SAND. Gravel is fine-medium, subrounded-rounded, of sandstone. (Below 2.20m is very gravelly).			
2.00-2.45	SPT N=11		2,3/3,3,2,3		(3.00)				
3.00-3.45	SPT N=13		3,3/3,3,3,4						
					3.70	Soft, brown, sandy CLAY. Sand is fine-medium.			
4.00-4.45	SPT N=11		2,2/3,2,3,3		(0.45)				
					4.15	Brown, silty, very gravelly, fine-coarse SAND. Gravel is fine-medium, rounded, of natural stone.			
					(0.15)				
					4.30	Firm-stiff, brown, slightly sandy CLAY. Sand is fine-medium.			
					(0.70)				
5.00-5.45	SPT N=14		3,2/3,4,3,4		5.00	Complete at 5.45m			
Remarks Drilling contractor: VC Drilling.								Scale (approx) 1:40	Logged By MW
								Figure No. 31296LG.WS104	



Sutcliffe

Site

Chorlton Irish Club

**Trial Pit
Number**
TP101

Excavation Method

Trial Pit

Dimensions

Ground Level (mOD)

Client

Southway Devco Ltd

**Job
Number**
31296LG

Location

Dates

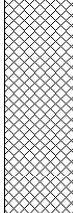
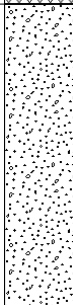
17/05/2021

Engineer

Michael Wiggins, Jack Smith

Sheet

1/1

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
0.20-0.40	ES1				(0.10)	(MADE GROUND) Tarmac.		
					0.10	(MADE GROUND) Dark brown, fine-coarse, angular-subangular GRAVEL, of tarmac. Slight hydrocarbon odour.		
					(0.10)			
					0.20			
					(0.60)	(MADE GROUND) Dark brown, gravelly, fine-coarse SAND. Gravel is fine-coarse, angular, of brick, tarmac and occasional pottery. Occasional cobbles of brick.		
0.80	Light brown, slightly gravelly, fine-coarse SAND. Gravel is fine-coarse, subrounded, of sandstone.							
					(0.80)			
					1.60	Complete at 1.60m		

Remarks

Excavation/Plant contractor: Clive Hurt Plant Hire



Scale (approx)

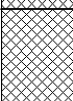
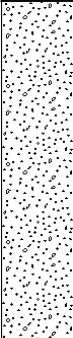
1:20

Logged By

MW

Figure No.

31296LG.TP101

 Sutcliffe						Site Chorlton Irish Club		Trial Pit Number TP102			
Excavation Method Trial Pit		Dimensions		Ground Level (mOD)		Client Southway Devco Ltd		Job Number 31296LG			
		Location		Dates 17/05/2021		Engineer Michael Wiggins, Jack Smith		Sheet 1/1			
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water			
0.20-0.50	ES1				(0.10)	(MADE GROUND) Tarmac.					
					0.10	(MADE GROUND) Dark brown, gravelly, fine-coarse SAND. Gravel is fine-coarse, angular, of tarmac and brick.					
					(0.10)						
					0.20	(MADE GROUND) Dark brown, slightly silty, gravelly, fine-coarse SAND. Gravel is fine-coarse, angular, of brick. Occasional cobbles of brick.					
					(0.60)						
					0.80	Light brown, slightly gravelly, fine-coarse SAND. Gravel is fine-coarse, subangular-subrounded, of natural stone.					
					(0.90)						
					1.70	Complete at 1.70m					
						Remarks Excavation/Plant contractor: Clive Hurt Plant Hire.					
						Scale (approx) 1:20		Logged By MW		Figure No. 31296LG.TP102	

Site : Chorlton Irish Club

Client : Southway Devco Ltd

Engineer : Michael Wiggins, Jack Smith

Job Number
31296LG

Sheet
1 / 1

Location	Date	Level	Location
SA101	05/11/2021		E: 381710 N: 393587

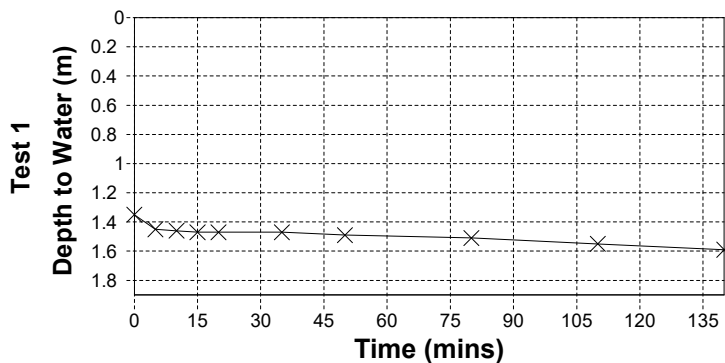
Pit Width (m)	0.70
Pit Depth (m)	1.90
Pit Length (m)	2.10

Soil type at test level	SAND
Groundwater	
Drain discharge depth	
Sidewall stability	Unstable
Stone filled or open pit	Open pit

	1
Effective depth (m)	0.55
Volume outflowing between 75% & 25% (m3)	0.40
Mean surface area through which outflow occurs (m2)	3.01
Time for outflow between 75% & 25% (min)	52.50
SOIL INFILTRATION RATE (ms-1), f	4.26E-5

Remarks

Elapsed time (mins)	Depth to Water Test 1
0	1.35
5	1.45
10	1.46
15	1.47
20	1.47
35	1.47
50	1.49
80	1.51
110	1.55
140	1.59



Appendix E

Q_{bar} Calculation

Proposed Drainage Areas Plan

Proposed Drainage Network Hydraulic Calculations

Sutcliffe		Page 1
18-20 Harrington Street Liverpool L2 9QA	Cross Street Chorlton-cum-Hardy Manchester	
Date 01/01/2022 File	Designed by DS Checked by	
Innovyze Source Control 2020.1.3		

ICP SUDS Mean Annual Flood

Input

Return Period (years)	2	Soil	0.300
Area (ha)	0.200	Urban	0.750
SAAR (mm)	842	Region Number	Region 10

Results 1/s

QBAR Rural	0.5
QBAR Urban	1.3
Q2 years	1.4
Q1 year	1.2
Q30 years	1.9
Q100 years	2.1

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Area Summary for Surface Network 1

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	User	-	100	0.025	0.025	0.025
2.000	User	-	100	0.009	0.009	0.009
1.001	-	-	100	0.000	0.000	0.000
1.002	User	-	100	0.015	0.015	0.015
3.000	User	-	100	0.025	0.025	0.025
	User	-	100	0.067	0.067	0.092
1.003	-	-	100	0.000	0.000	0.000
1.004	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				0.140	0.140	0.140

Free Flowing Outfall Details for Surface Network 1

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
---------------------	--------------	--------------	--------------	------------------	----------	--------

1.004	PS7	28.068	27.710	0.000	180	0
-------	-----	--------	--------	-------	-----	---

Simulation Criteria for Surface Network 1

Volumetric Runoff Coeff	1.000	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	0.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1
Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	0	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	2	Cv (Summer)	1.000
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	18.000	Storm Duration (mins)	30
Ratio R	0.378		

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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 0.000
 Hot Start Level (mm) 0 Inlet Coefficient 0.800
 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000
 Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0 Number of Storage Structures 1
 Number of Online Controls 0 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Ratio R 0.378
 Region England and Wales Cv (Summer) 1.000
 M5-60 (mm) 18.000 Cv (Winter) 1.000

Margin for Flood Risk Warning (mm) 300.0
 Analysis Timestep 2.5 Second Increment (Extended)
 DTS Status ON
 DVD Status OFF
 Inertia Status OFF

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
 Return Period(s) (years) 1
 Climate Change (%) 0

									Water
PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Level (m)
1.000	PS1	15 Summer	1	+0%					27.688
2.000	PS6	15 Summer	1	+0%					27.596
1.001	PS2	15 Summer	1	+0%					27.003
1.002	PS3	15 Summer	1	+0%					26.693
3.000	PS5	15 Summer	1	+0%					27.088
1.003	PS4	240 Summer	1	+0%	1/15 Summer				26.284
1.004	S1	240 Summer	1	+0%	1/15 Summer				26.282

Surcharged Flooded					Half Drain	Pipe		
PN	US/MH Name	Depth (m)	Volume (m³)	Flow / Overflow Cap.	Time (mins)	Flow (l/s)	Status	Level Exceeded
1.000	PS1	-0.112	0.000	0.15		3.9	OK	
2.000	PS6	-0.079	0.000	0.10		1.3	OK	
1.001	PS2	-0.097	0.000	0.26		5.1	OK	
1.002	PS3	-0.107	0.000	0.18		7.0	OK	
3.000	PS5	-0.168	0.000	0.15		14.3	OK	
1.003	PS4	0.079	0.000	0.48		6.4	SURCHARGED	
1.004	S1	0.122	0.000	0.00	143	0.0	SURCHARGED	

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Innovyze	Network 2020.1.3	

Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 0.000
 Hot Start Level (mm) 0 Inlet Coefficient 0.800
 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000
 Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0 Number of Storage Structures 1
 Number of Online Controls 0 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Ratio R 0.378
 Region England and Wales Cv (Summer) 1.000
 M5-60 (mm) 18.000 Cv (Winter) 1.000

Margin for Flood Risk Warning (mm) 300.0
 Analysis Timestep 2.5 Second Increment (Extended)
 DTS Status ON
 DVD Status OFF
 Inertia Status OFF

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
 Return Period(s) (years) 2
 Climate Change (%) 0

									Water
PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Level (m)
1.000	PS1	15 Summer	2	+0%					27.694
2.000	PS6	15 Summer	2	+0%					27.599
1.001	PS2	15 Summer	2	+0%					27.011
1.002	PS3	15 Summer	2	+0%					26.699
3.000	PS5	15 Summer	2	+0%					27.097
1.003	PS4	120 Winter	2	+0%	2/15 Summer				26.374
1.004	S1	120 Winter	2	+0%	2/15 Summer				26.372

Surcharged Flooded					Half Drain	Pipe			
PN	US/MH Name	Depth (m)	Volume (m³)	Flow / Overflow Cap.	Time (mins)	Flow (l/s)	Status	Level Exceeded	
1.000	PS1	-0.106	0.000	0.19		5.0	OK		
2.000	PS6	-0.076	0.000	0.13		1.7	OK		
1.001	PS2	-0.089	0.000	0.34		6.7	OK		
1.002	PS3	-0.101	0.000	0.23		9.1	OK		
3.000	PS5	-0.159	0.000	0.19		18.5	OK		
1.003	PS4	0.169	0.000	0.63		8.4	SURCHARGED		
1.004	S1	0.212	0.000	0.00	128	0.0	SURCHARGED		

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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 0.000
 Hot Start Level (mm) 0 Inlet Coefficient 0.800
 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000
 Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0 Number of Storage Structures 1
 Number of Online Controls 0 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Ratio R 0.378
 Region England and Wales Cv (Summer) 1.000
 M5-60 (mm) 18.000 Cv (Winter) 1.000

Margin for Flood Risk Warning (mm) 300.0
 Analysis Timestep 2.5 Second Increment (Extended)
 DTS Status ON
 DVD Status OFF
 Inertia Status OFF

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
 Return Period(s) (years) 30
 Climate Change (%) 0

									Water
	US/MH			Return Climate	First (X)	First (Y)	First (Z)	Overflow	Level
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	(m)
1.000	PS1	15 Summer	30	+0%					27.712
2.000	PS6	15 Summer	30	+0%					27.608
1.001	PS2	15 Summer	30	+0%					27.068
1.002	PS3	15 Summer	30	+0%	30/15 Summer				26.961
3.000	PS5	15 Summer	30	+0%					27.124
1.003	PS4	15 Summer	30	+0%	30/15 Summer				26.861
1.004	S1	240 Winter	30	+0%	30/15 Summer				26.803

		Surcharged Flooded		Half Drain		Pipe			
PN	US/MH Name	Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	Status	Level Exceeded
1.000	PS1	-0.088	0.000	0.36			9.4	OK	
2.000	PS6	-0.067	0.000	0.24			3.3	OK	
1.001	PS2	-0.032	0.000	0.64			12.4	OK	
1.002	PS3	0.161	0.000	0.41			16.2	SURCHARGED	
3.000	PS5	-0.132	0.000	0.36			35.0	OK	
1.003	PS4	0.656	0.000	3.62			48.0	SURCHARGED	
1.004	S1	0.643	0.000	0.00		242	0.0	SURCHARGED	

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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

Simulation Criteria

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	0	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Rainfall Model	FSR	Ratio R	0.378
Region	England and Wales	Cv (Summer)	1.000
M5-60 (mm)	18.000	Cv (Winter)	1.000

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years)	100
Climate Change (%)	20

PN	US/MH Name	Surcharged	Flooded	Flow / Cap.	Overflow (l/s)	Half Drain	Pipe	Status	Level Exceeded
		Depth	Volume			Time	Flow		
		(m)	(m³)			(mins)	(l/s)		
1.000	PS1	0.450	0.000	0.52			13.6	FLOOD	RISK
2.000	PS6	0.474	0.000	0.35			4.7	FLOOD	RISK
1.001	PS2	0.995	0.000	0.87			16.9	FLOOD	RISK
1.002	PS3	1.084	0.000	0.56			22.1	FLOOD	RISK
3.000	PS5	0.676	0.000	0.52			50.8	FLOOD	RISK
1.003	PS4	1.466	0.000	5.24			69.6	SURCHARGED	
1.004	S1	1.215	0.000	0.00		347	0.0	SURCHARGED	

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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 0.000
 Hot Start Level (mm) 0 Inlet Coefficient 0.800
 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000
 Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0 Number of Storage Structures 1
 Number of Online Controls 0 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Ratio R 0.378
 Region England and Wales Cv (Summer) 1.000
 M5-60 (mm) 18.000 Cv (Winter) 1.000

Margin for Flood Risk Warning (mm) 300.0
 Analysis Timestep 2.5 Second Increment (Extended)
 DTS Status ON
 DVD Status OFF
 Inertia Status OFF

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
 Return Period(s) (years) 100
 Climate Change (%) 30

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
1.000	PS1	15 Summer	100	+30%	100/15 Summer			
2.000	PS6	15 Summer	100	+30%	100/15 Summer			
1.001	PS2	15 Summer	100	+30%	100/15 Summer			
1.002	PS3	15 Summer	100	+30%	100/15 Summer			
3.000	PS5	15 Summer	100	+30%	100/15 Summer	100/15 Summer		
1.003	PS4	15 Summer	100	+30%	100/15 Summer			
1.004	S1	240 Winter	100	+30%	100/15 Summer			

PN	US/MH Name	Water Level	Surcharged Depth	Flooded Volume	Flow / Overflow		Half Drain Time	Pipe Flow	Status
		(m)	(m)	(m³)	Cap.	(l/s)	(mins)	(l/s)	
1.000	PS1	28.255	0.455	0.000	0.54			14.4	FLOOD RISK
2.000	PS6	28.271	0.596	0.000	0.37			5.1	FLOOD RISK
1.001	PS2	28.178	1.078	0.000	1.00			19.5	FLOOD RISK
1.002	PS3	28.024	1.224	0.000	0.59			23.4	FLOOD RISK
3.000	PS5	28.085	0.829	0.191	0.56			54.4	FLOOD
1.003	PS4	27.823	1.618	0.000	5.58			74.2	SURCHARGED
1.004	S1	27.508	1.348	0.000	0.00		367	0.0	SURCHARGED

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Innovyze	Network 2020.1.3																											
<u>Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1</u>																												
	<table><thead><tr><th></th><th>US/MH</th><th>Level</th></tr><tr><th>PN</th><th>Name</th><th>Exceeded</th></tr></thead><tbody><tr><td>1.000</td><td>PS1</td><td></td></tr><tr><td>2.000</td><td>PS6</td><td></td></tr><tr><td>1.001</td><td>PS2</td><td></td></tr><tr><td>1.002</td><td>PS3</td><td></td></tr><tr><td>3.000</td><td>PS5</td><td>1</td></tr><tr><td>1.003</td><td>PS4</td><td></td></tr><tr><td>1.004</td><td>S1</td><td></td></tr></tbody></table>		US/MH	Level	PN	Name	Exceeded	1.000	PS1		2.000	PS6		1.001	PS2		1.002	PS3		3.000	PS5	1	1.003	PS4		1.004	S1	
	US/MH	Level																										
PN	Name	Exceeded																										
1.000	PS1																											
2.000	PS6																											
1.001	PS2																											
1.002	PS3																											
3.000	PS5	1																										
1.003	PS4																											
1.004	S1																											
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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 0.000
 Hot Start Level (mm) 0 Inlet Coefficient 0.800
 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000
 Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0 Number of Storage Structures 1
 Number of Online Controls 0 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Ratio R 0.378
 Region England and Wales Cv (Summer) 1.000
 M5-60 (mm) 18.000 Cv (Winter) 1.000

Margin for Flood Risk Warning (mm) 300.0
 Analysis Timestep 2.5 Second Increment (Extended)
 DTS Status ON
 DVD Status OFF
 Inertia Status OFF

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
 Return Period(s) (years) 100
 Climate Change (%) 40

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
1.000	PS1	15 Summer	100	+40%	100/15 Summer	100/15 Summer		
2.000	PS6	15 Summer	100	+40%	100/15 Summer			
1.001	PS2	15 Summer	100	+40%	100/15 Summer			
1.002	PS3	15 Summer	100	+40%	100/15 Summer			
3.000	PS5	15 Summer	100	+40%	100/15 Summer	100/15 Summer		
1.003	PS4	30 Summer	100	+40%	100/15 Summer			
1.004	S1	240 Winter	100	+40%	100/15 Summer			

PN	US/MH Name	Water Level	Surcharged Depth	Flooded Volume	Flow / Overflow	Half Drain Time	Pipe Flow	Status
		(m)	(m)	(m³)	Cap. (l/s)	(mins)	(l/s)	
1.000	PS1	28.260	0.460	1.018	0.60		15.9	FLOOD
2.000	PS6	28.316	0.641	0.000	0.43		5.9	FLOOD RISK
1.001	PS2	28.222	1.122	0.000	1.05		20.4	FLOOD RISK
1.002	PS3	28.104	1.304	0.000	0.60		23.7	FLOOD RISK
3.000	PS5	28.086	0.830	0.875	0.58		56.8	FLOOD
1.003	PS4	27.851	1.646	0.000	5.40		71.8	FLOOD RISK
1.004	S1	27.843	1.683	0.000	0.32	431	3.4	FLOOD RISK

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18-20 Harrington Street Liverpool L2 9QA	Cross Street Chorlton-cum-Hardy Manchester																									
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<u>Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1</u>																										
	<table><thead><tr><th>PN</th><th>US/MH Name</th><th>Level Exceeded</th></tr></thead><tbody><tr><td>1.000</td><td>PS1</td><td>1</td></tr><tr><td>2.000</td><td>PS6</td><td></td></tr><tr><td>1.001</td><td>PS2</td><td></td></tr><tr><td>1.002</td><td>PS3</td><td></td></tr><tr><td>3.000</td><td>PS5</td><td>3</td></tr><tr><td>1.003</td><td>PS4</td><td></td></tr><tr><td>1.004</td><td>S1</td><td></td></tr></tbody></table>	PN	US/MH Name	Level Exceeded	1.000	PS1	1	2.000	PS6		1.001	PS2		1.002	PS3		3.000	PS5	3	1.003	PS4		1.004	S1		
PN	US/MH Name	Level Exceeded																								
1.000	PS1	1																								
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3.000	PS5	3																								
1.003	PS4																									
1.004	S1																									
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Appendix F

Correspondence

From: [Wastewater Developer Services](#)
To: [Martin Pocock](#)
Cc: [Wastewater Developer Services](#)
Subject: FW: 31296 - Chorlton Irish Club - UU Developer Enquiry - 4200034114
Date: 13 October 2020 12:47:47
Attachments: [image001.jpg](#)
[image002.png](#)
[image003.png](#)
[31296 - Chorlton Irish Club - UU Developer Enquiry 06102020.pdf](#)
[31296-SUT-ZZ-XX-DR-C-6000-0001-P01 - SITE LOCATION PLAN.pdf](#)
[31296-SUT-ZZ-XX-DR-C-6000-0002-P01 - EXISTING DEVELOPMENT CATCHMENT AREA.pdf](#)
[31296-SUT-ZZ-XX-DR-C-6000-0003-P01 - PROPOSED DEVELOPMENT CATCHMENT AREA.pdf](#)
[31296-SUT-ZZ-XX-DR-C-6000-0004-P01 - PROPOSED EASEMENT TO EXISTING UNITED UTILITIES SEWER.pdf](#)
[RBS-20-2061-001.pdf](#)

Good Afternoon Martin,

We have carried out an assessment of your application which is based on the information provided. This pre-development advice on your drainage strategy will be valid for 12 months. Your drainage strategy will need to be reviewed by other competent authorities as part of the planning process, and we advise that you carry out the necessary site investigations to confirm the viability of your proposals.

If your investigations require access to our public sewer network, we ask that you contact our network engineers with a request for an access certificate via our main contact telephone number 0345 3723223 or refer to the link below:

<https://www.unitedutilities.com/builders-developers/working-near-our-assets/>

Foul Water

Foul flow from this site will be allowed to drain into the public foul water/combined sewer system an unrestricted rate.

Surface Water

All surface water flow from the proposed development should drain in-line with the drainage hierarchy, as outlined in Paragraph 80, (Reference ID: 7-080-20150323), of the National Planning Practice Guidance. We also recommend you prioritise the use of multi-functional sustainable drainage systems for the management of surface water in accordance with national planning policy.

Generally, the aim should be to discharge surface run off as high up the following hierarchy of drainage options as reasonably practicable.

This is outlined as follows, in order of priority:

- 1. into the ground (infiltration);**
- 2. to a surface waterbody;**
- 3. to a surface water sewer or highway drain;**
- 4. to a combined sewer.**

For guidance, The North West SuDS Pro-Forma provides information on the appropriate evidence required at each stage of the hierarchy, to demonstrate how each level has been discounted.

The Lead Local Flood Authority has responsibility for all surface water drainage concerns and their input to your proposal is critical. You should also consider whether it is necessary to discuss your

proposal with the Environment Agency, or Internal Drainage Board (if operating in your area).

The Local Planning Authority are the determining authority for any application for planning permission and the appropriate authority for determining cost viability of a proposed drainage scheme, such assessments are outside of the jurisdiction of United Utilities.

Infiltration

Surface water runoff generated from this development should discharge to the ground via infiltration system where feasible. According to online data, infiltration may be possible at this site due to ground being made up of sand and gravel.

A detailed evidence based feasibility assessment must be carried out in line with Chapter 25 of the CIRIA SuDS Manual 2015 to determine whether infiltration is a suitable method of surface water disposal.

Particular attention must be paid to Ground Water Source Protection Zones to ensure that the risk of pollution to these valuable resources is not compromised. Details can be obtained from the government website:

<https://www.gov.uk/guidance/groundwater-source-protection-zones-spzs#find-groundwater-spzs>

If your site is in a Groundwater Source Protection Zone, you should have regard to the Environment Agency's approach to Groundwater Protection. Information on this is available via the link below:

<https://www.gov.uk/government/publications/groundwater-protection-position-statements>

Please note that such a location could have implications for the principle of your development and the need for additional mitigating measures to protect the groundwater environment and public water supply in the detailed design of your site.

-

Public Sewer

In accordance with the hierarchy of drainage options within the National Planning Practice Guidance, both discharge to ground via infiltration and discharge to a waterbody should be discounted prior to consideration of discharging surface water to the public sewer system. Evidence should be provided to demonstrate how these have been discounted, as outlined in the North West SuDS pro-forma.

Once evidence is provided as outlined above, United Utilities will consider a connection to the 300mm diameter public combined sewer located 13m North of the proposed site at a pass forward flow to be agreed by the Lead Local Flood Authority. United Utilities request that any agreed rate does not exceed 5 l/s.

As a Water Company, we have no obligation to accept highway drainage into our public sewer network. However, should your proposals include runoff from highways, we would request that consideration is given to SuDS components that deliver source control are incorporated within the design of the scheme to reduce the volume and frequency of discharges of these flows to the public sewer.

Levels

For low-lying sites, (where the ground level of the site or the level of a basement is below the ground level at the point where the drainage connects to the public sewer), care should be taken to ensure that the property is not at increased risk of flooding. If these circumstances exist, we recommend that you contact us to discuss further. It could affect the detailed design of your site and result in the need to incorporate appropriate mitigating measures in your drainage scheme.

Existing Wastewater Assets Crossing the Site

According to our public sewer records there are public sewers located within your site boundary. We will require unrestricted access to the sewer for maintenance purposes, we would ask that you maintain a minimum clearance of 6m which is measured 3m from the centre line of the pipe unless there happens to be a formal easement agreement in place, in which case the specified easement width would apply. If you cannot achieve this then you may wish to consider diverting and or abandoning the public sewer.

Please refer to the link below to obtain full details of the processes involved with building over,

<https://www.unitedutilities.com/builders-developers/small-housing-developments/building-over-a-public-sewer/>

Please refer to the link below to obtain full details of the processes involved with diversions,

<https://www.unitedutilities.com/builders-developers/larger-developments/wastewater/sewer-diversions/>

-

Existing Water Assets Crossing the Site

It is the developer responsibility to identify utilities on-site. Where clean water assets are shown on our records, we recommend that you contact our Water Pre-Development Team, via the following email address: DeveloperServicesWater@uuplc.co.uk. Further information for this service can be found on our website via the link below:

<https://www.unitedutilities.com/builders-developers/larger-developments/pre-development/water-pre-dev/>

Connection Application

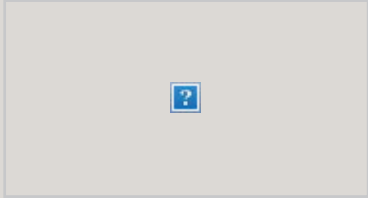
Although we may discuss and agree discharge points and rates in principle, please be aware that you will have to apply for a formal sewer connection. This is so that we can assess the method of construction, Health & Safety requirements and to ultimately inspect the connection when it is made. Details of the application process and the form itself can be obtained from our website by following the link below:

<https://www.unitedutilities.com/builders-developers/larger-developments/wastewater/sewer-connections/>

We recommend that the detailed design should confirm the locations of all utilities in the area and ensure that any proposed drainage solution considers routing and clash checks where required.

If we can be of any further assistance please don't hesitate to contact us further.

Kind regards,

	<p>Marie Mura Assistant Developer Engineer Developer Services & Metering Customer Services T: 01925 537745 unitedutilities.com</p>
---	--

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cid:image002.png@01D60DA3.399625A0



From: Martin Pocock [mailto:Martin.Pocock@sutcliffe.co.uk]

Sent: 06 October 2020 15:34

To: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>

Cc: William Baldwin <William.Baldwin@sutcliffe.co.uk>; Tom Williams
<tom.williams@neoprojects.co.uk>

Subject: 31296 - Chorlton Irish Club - UU Developer Enquiry

I have attached a completed UU Developer enquiry, drawings and topographical survey of the site.

The site is a brownfield redevelopment of the existing Chorlton Irish Club car park. As yet no CCTV has been carried out to confirm the existing surface water outfall.

I have also attached a plan s=indicating the proposed easement relating to the existing combined sewer cutting across the south east corner of the development area.

I would be grateful if UU can confirm if a building over agreement required with UU, as the building offset distance is greater than 3.0m from the centre line of the existing sewer.

Please let me know if you need any further information.

Look forward to your comments.

Regards

Martin Pocock
BEng(Hons) CEng FICE MCIHT CMgr FCMI C.WEM MCIWEM
Director

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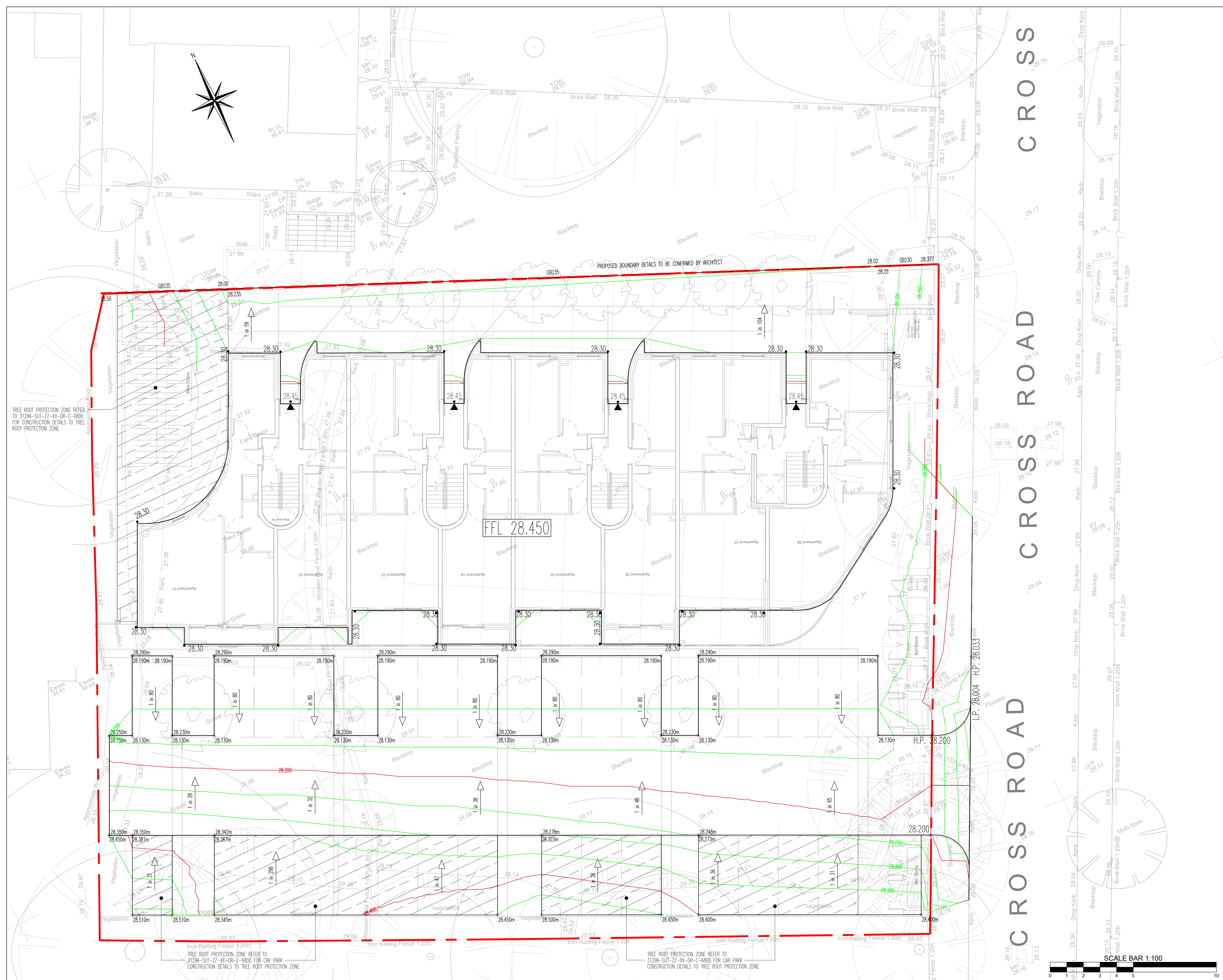
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FOR FINAL SETTING OUT INFORMATION RELATING TO GRID LINES AND WALL POSITIONS REFER TO THE ARCHITECT'S DRAWINGS.

EXISTING SITE CONDITIONS

THE DEVELOPER SHALL NOT COMMENCE ANY WORK WHICH AFFECTS THE PUBLIC HIGHWAY UNTIL ALL THE APPROVED TRAFFIC SAFETY AND MANAGEMENT MEASURES NECESSITATED BY THE WORKS ARE IN PLACE.

THE DEVELOPER SHALL MAINTAIN ACCESS TO AND ALONG ALL EXISTING FOOTPATHS, PRIVATE ACCESSSES, PUBLIC RIGHTS OF WAY AND PROPOSED CROSSINGS OF THE WORKS THROUGHOUT THE CONSTRUCTION PERIOD.

THE CONTRACTOR IS TO CONFIRM EXISTING ROAD LEVELS AT ALL PROPOSED JUNCTIONS BETWEEN NEW AND EXISTING ROAD CONSTRUCTION PRIOR TO COMMENCEMENT OF WORKS.

PROPOSED HIGHWAY WORKS

ALL HIGHWAY WORKS TO BE DESIGNED AND CONSTRUCTED TO THE APPROVAL OF THE LOCAL AUTHORITY HIGHWAYS DEPARTMENT.

ALL ROAD OPENING NOTICES ETC., AS REQUIRED UNDER THE HIGHWAYS ACTS ARE TO BE OBTAINED PRIOR TO COMMENCEMENT OF WORKS ON SITE. ALL WORKS ARE TO BE INSPECTED BY THE LOCAL AUTHORITY.

ALL MATERIALS SHALL BE TRANSPORTED & LAID IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE APPROPRIATE CURRENT BRITISH STANDARD FOR THAT MATERIAL.

ALL GULLIES TO BE PROTECTED FROM SILT DURING THE CONSTRUCTION PROCESS WITH A LAYER OF HESSIAN SHEETING PLACED UNDER THE GULLY GRATING. THE HESSIAN SHALL BE CLEANED AND MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD.

REFER TO SEPARATE DRAWINGS FOR ALL KERB / ROAD / FOOTPATHS NOTES AND DETAILS. REFER TO DRAWING ISSUE SHEET FOR A FULL LIST OF DRAWINGS.

EXISTING TREES

ALL EXISTING TREES, BUSHES SHALL BE UP ROOTED OR CUT DOWN AS NEAR TO GROUND LEVEL AS POSSIBLE. ALL FELLED TIMBER TO BE DISPOSED OF BY THE CONTRACTOR.

STUMPS AND ROOTS FROM FELLED TREES, BUSHES AND HEDGES SHALL BE GRUBBED UP AND DISPOSED OF BY THE CONTRACTOR. HOLES LEFT FROM STUMPS AND ROOTS SHALL BE FILLED WITH ACCEPTABLE MATERIAL AND COMPACTED.

ALL RETAINED EXISTING TREES, SHRUBS, GRASSLAND AND THE LIKE ON OR ADJOINING THE SITE SHALL BE PROTECTED FROM DAMAGE ARISING THROUGH SITE OPERATIONS.

RETAINING STRUCTURES

EXTENT OF ALL RETAINING WALLS, CATIONS AND FLAG ON EDGE RETAINING STRUCTURES NOTED ON THIS DRAWING ARE INDICATIVE ONLY. REFER TO SEPARATE DRAWINGS FOR ALL NOTES AND DETAILS. REFER TO DRAWING ISSUE SHEET FOR A FULL LIST OF DRAWINGS.

THE CONTRACTOR IS TO BE FULLY RESPONSIBLE FOR ALL TEMPORARY / PERMANENT PROPPING AND SHORING ON THE PROJECT. CONTRACTOR TO ENSURE THAT ALL PARTS OF THE STRUCTURE ARE ADEQUATELY PROPPED PRIOR TO ANY COMMENCEMENT OF WORKS. ADVICE MAY BE SOUGHT FROM A SPECIALIST OR FROM THE ENGINEER.

KEY PLAN

- × 28.235 PROPOSED SPOT LEVEL
- 28.30 PROPOSED LEVEL AT CORNER OF BUILDING
- 1 in 40 PROPOSED SURFACE GRADIENT
- × 27.96 EXISTING SPOT LEVELS
- [F.F.L. 28.450] FINISHED FLOOR LEVEL
- 28.30 --- PROPOSED GRAVEL BOARD DIFFERENCE FROM PROPOSED LOWER GROUND LEVEL TO HIGHER GROUND LEVEL (mm)
- 28.000 — PROPOSED MAJOR CONTOUR
- 28.050 — PROPOSED MINOR CONTOUR

P03	SITE LAYOUT UPDATED	18.11.22	AJB	DS
P02	AMENDED TO SUIT NEW LAYOUT	27.09.22	AJB	DS
P01	ISSUED FOR COMMENT	29.11.21	AJB	DS
Rev	Description	Date	By	Chkd

Drawing Status
PRELIMINARY DRAWING

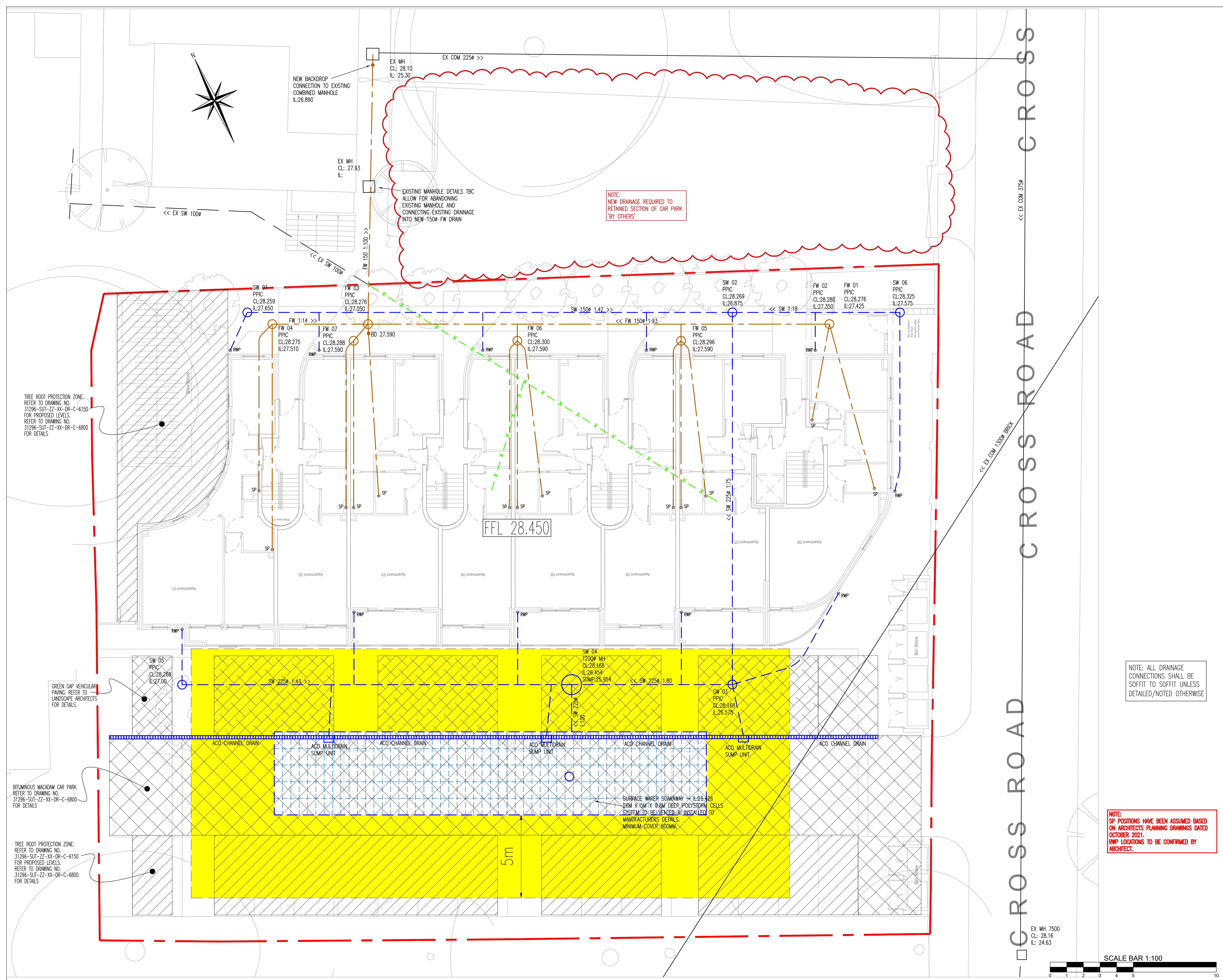
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Client
SOUTHWAY HOUSING TRUST

Project
**CHORLTON IRISH CLUB
CHORLTON-CUM-HARDY**

Drawing Title
PROPOSED LEVELS

Date NOV. 2021	Drawn By AJB	Checked By DS
Scale at A1 1:100	Project Number 31296	Revision P03
Sheet Number 31296-SUT-ZZ-XX-DR-C-6150		



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FOR FINAL SETTING OUT INFORMATION RELATING TO GRID LINES AND WALL POSITIONS REFER TO THE ARCHITECT'S DRAWINGS.

ALL FLOOR LEVELS TO BE CONFIRMED BY ARCHITECT PRIOR TO CONSTRUCTION

ALL COVER LEVELS TO BE CONFIRMED BY ARCHITECT PRIOR TO CONSTRUCTION

ALL DRAINAGE PENETRATING THROUGH FLOOR SLABS AND RAINWATER PIPE LOCATIONS TAKEN FROM ARCHITECTS DRAWINGS. ARCHITECT TO CONFIRM LOCATIONS PRIOR TO CONSTRUCTION.

EXISTING SERVICES

ROAD SIGNS AND LAMP COLUMNS ON THE SITE PERIPHERY TO BE RE-SITED IF REQUIRED BY, AND TO THE APPROVAL OF, THE LOCAL AUTHORITY.

CONTRACTOR TO SATISFY THEMSELVES THAT EXISTING AND PROPOSED SERVICES DO NOT CLASH WITH PROPOSED DRAINAGE

KNOWN EXISTING SERVICES ARE INDICATED ON THE PLAN WHERE THEY CROSS THE PROPOSED DRAINAGE. THE EXACT TYPE AND LOCATION SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO START OF WORKS TO ENSURE THAT PROPOSED DRAINAGE LEVELS DO NOT INTERFERE WITH SERVICES. ALL DISCREPANCIES SHALL BE REPORTED TO ENGINEER IMMEDIATELY.

THE CONTRACTOR SHALL CHECK ALL EXISTING COVER / INVERT LEVELS AND PIPE / SEWER SIZES ON SITE PRIOR TO START OF WORKS TO ENSURE PROPOSED DRAINAGE CAN BE CONSTRUCTED AS INDICATED ON THE DRAWING. ANY DISCREPANCIES TO BE REPORTED TO THE ENGINEER IMMEDIATELY.

ALL REDUNDANT DRAINAGE WITHIN SITE TO BE GRUBBED UP, CHASED AND SEALED TO APPROVAL OF LOCAL AUTHORITY

PROPOSED DRAINAGE

OUTSIDE EDGE OF ADOPTED SEWER SHALL NOT ENCRoACH WITHIN 1M OF KERB FACE. OUTSIDE EDGE OF ADOPTED MANHOLES SHALL NOT ENCRoACH WITHIN 0.5M OF KERB FACE.

ALL DRAINAGE IN HIGHWAYS/CAR PARKING AREAS/DRIVEWAYS LESS THAN 1.2M DEEP TO BE CONCRETE ENCASED.

RWP'S GREATER THAN 1.2M DEEP TO HAVE REDUCED ACCESS COVERS TO PREVENT MAN ENTRY

SURFACE WATER DRAINAGE:
ALL PRIVATE SURFACE WATER DRAINAGE TO BE 100MM Ø AT 1:80 GRADIENT UNLESS NOTED OTHERWISE.
ALLOWANCE TO BE MADE FOR DRIVEWAY GULLIES AND ASSOCIATED DRAIN RUNS WHERE APPLICABLE

FOUL WATER / COMBINED DRAINAGE:
ALL UNDERSLAB FOUL WATER DRAINAGE TO BE 100MM Ø AT 1:40 GRADIENT UNLESS NOTED OTHERWISE
ALL OTHER PRIVATE FOUL WATER DRAINAGE TO BE 100MM Ø AT 1:80 GRADIENT UNLESS NOTED OTHERWISE
INTERNAL VENTING OF DRAINAGE SYSTEM TO ARCHITECT'S DETAILS
ALL INTERNAL SP/SVP POSITIONS TO BE CONFIRMED BY ARCHITECT PRIOR TO CONSTRUCTION OF GROUND FLOOR SLAB

KEY

- Site boundary
- Existing Surface water drain/sewer
- Existing Foul water drain/sewer
- Existing Combined water drain/sewer
- Proposed Surface water private drain
- Proposed Foul water private drain
- Exlg drainage to be abandoned to Local Authority approval
- RWP = Rainwater pipe location
- SP = Soil pipe location
- BIC = Back inlet gully
- SG = Shower gully
- EX G = Existing Road gully
- RG = New road gully
- DG = New driveway gully
- Bluminox Macadam Car Park
- Tree root protection zone
- Green gap vehicular paving
- 5m Easement Soakaway

NOTE: INTERNAL SVP/SP POSITIONS TO BE CONFIRMED BY ARCHITECT PRIOR TO CONSTRUCTION OF GROUND FLOOR SLAB.

P06	SOAKAWAY EASEMENT ADDED	19.12.23	JG	DS
P05	PERMEABLE CAR PARK REMOVED	18.11.22	AJB	DS
P04	AMENDED TO SUIT REVISED LEVELS	27.09.22	AJB	DS
P03	AMENDED TO SUIT NEW LAYOUT	09.06.22	MJG	DS
P02	DRAINAGE AMENDED APT 08	07.01.22	AJB	DS
P01	ISSUED FOR COMMENT	29.11.21	AJB	DS
Rev	Description	Date	By	Chkd

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Project
**CHORLTON IRISH CLUB
CHORLTON-CUM-HARDY**

Drawing Title
PROPOSED DRAINAGE LAYOUT

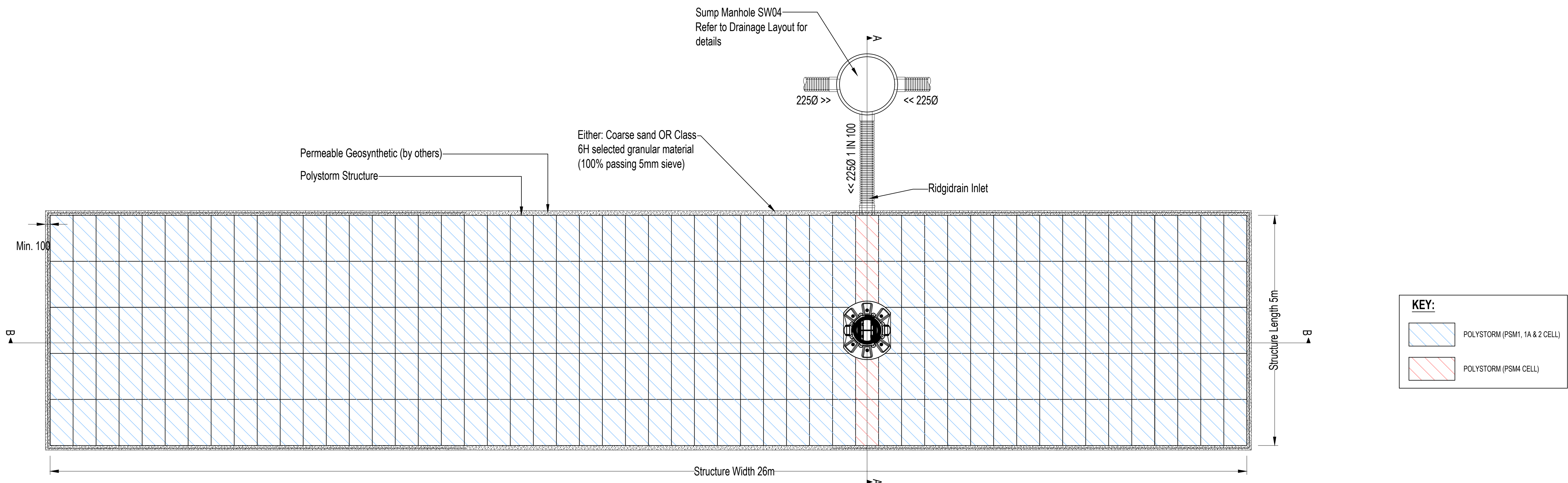
Date NOV. 2021	Drawn By AJB	Checked By DS
Scale at A1 1:100	Project Number 31296	Revision
Sheet Number 31296-SUT-ZZ-XX-DR-C-6200	P06	

GENERAL NOTES

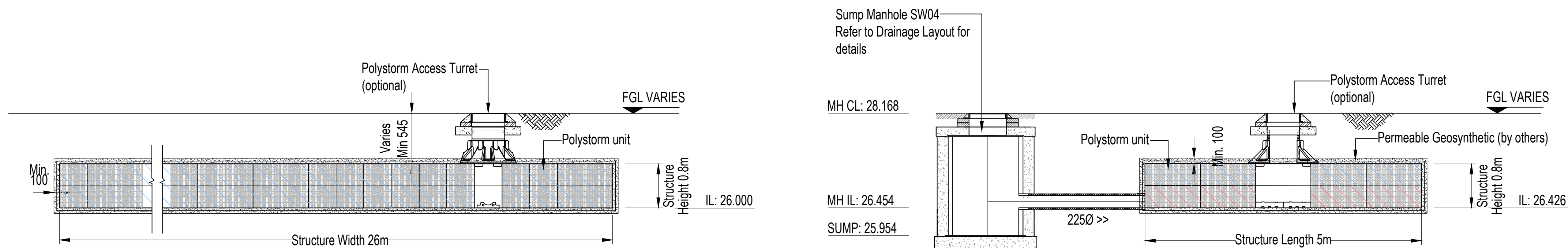
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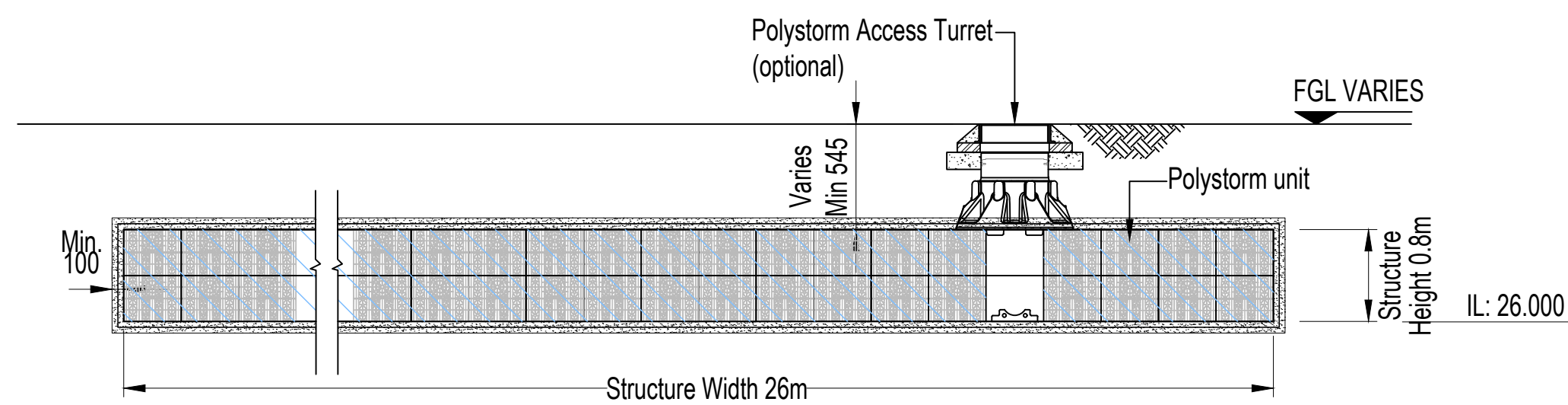
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TYPICAL SOAKAWAY TANK PLAN



SECTION A-A



SECTION B-B

P04	SIZE AMENDED	20.12.23	JG	DS
P03	HATCHING AMENDED	26.09.22	AJB	DS
P02	DIMS / LEVELS ADDED	29.07.22	AJB	DS
P01	ISSUED FOR COMMENT	30.11.21	AJB	DS
Rev	Description	Date	By	Chkd

Drawing Status
PRELIMINARY DRAWING



Client
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Project
CHORLTON IRISH CLUB
CHORLTON-CUM-HARDY

Drawing Title
SOAKAWAY DETAILS

Date NOV. 2021	Drawn By AJB	Checked By DS
Scale at A1 AS SHOWN	Project Number 31296	
Sheet Number 31296-SUT-ZZ-XX-DR-C-6660	Revision P04	

General Notes

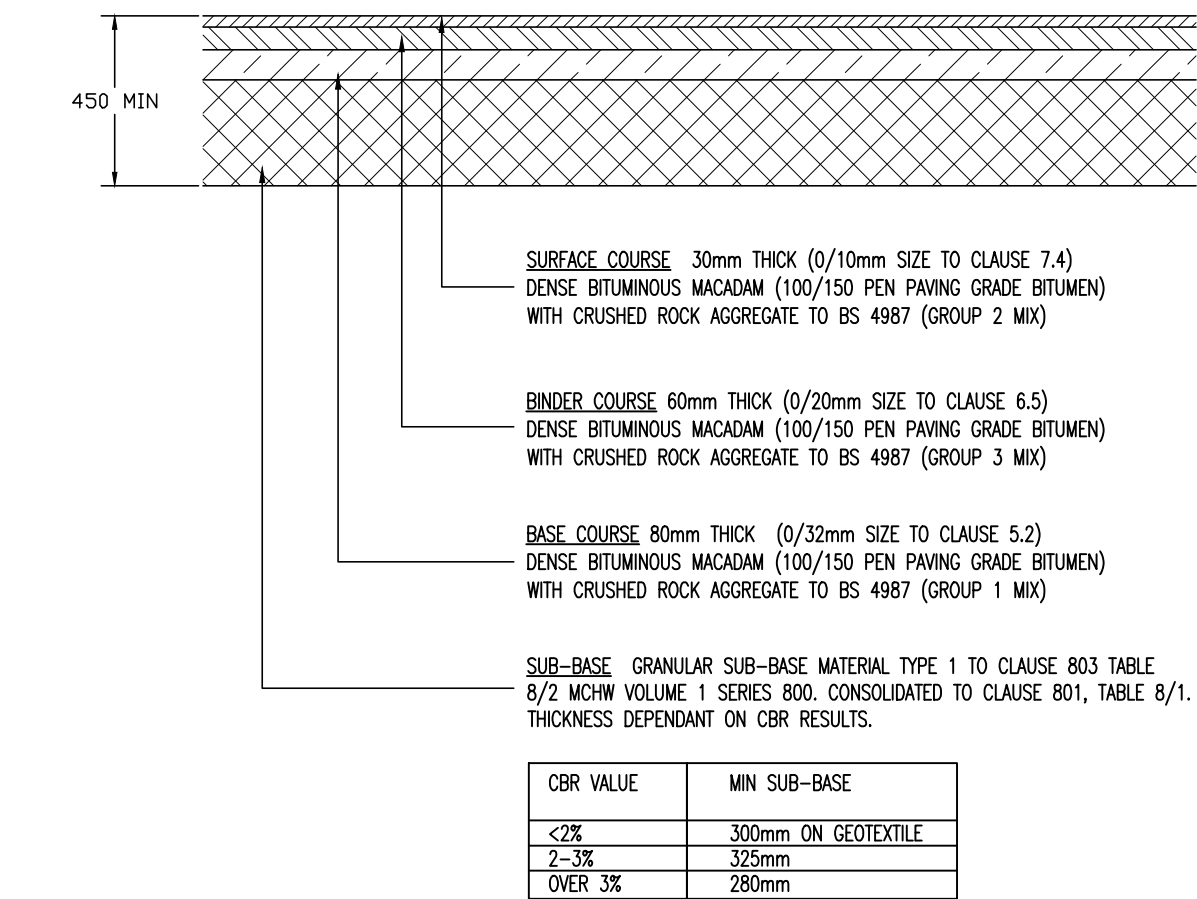
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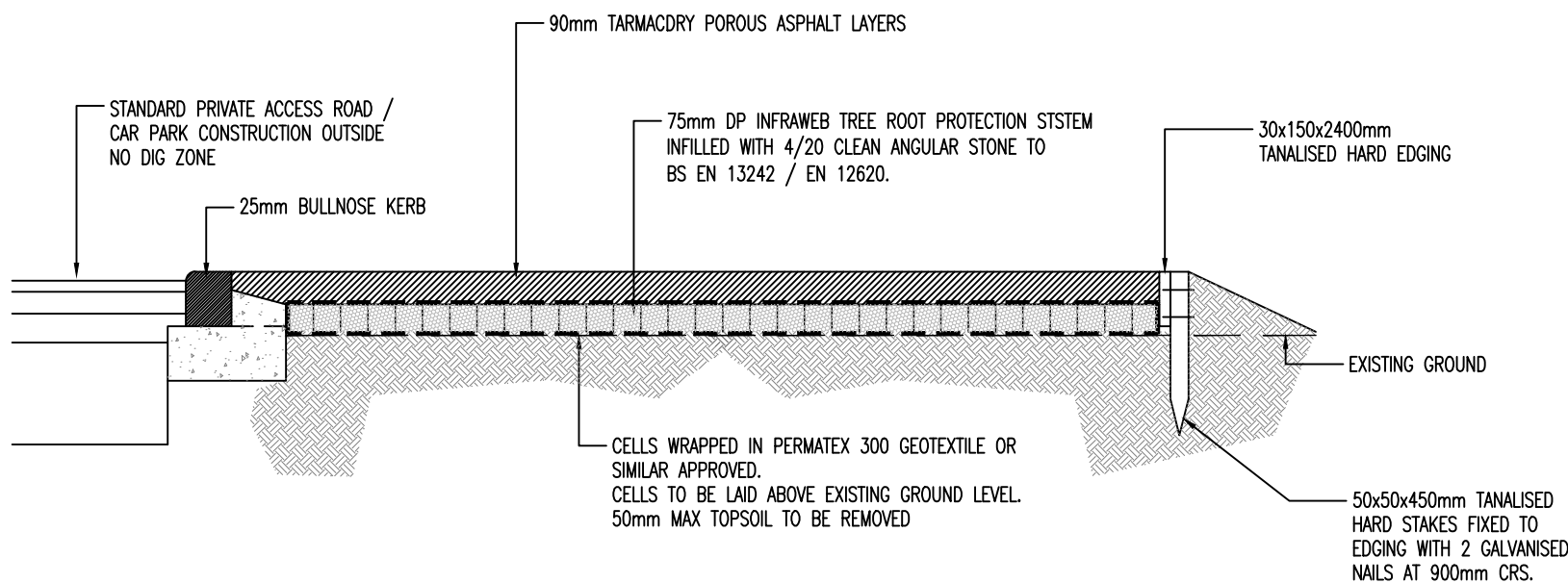
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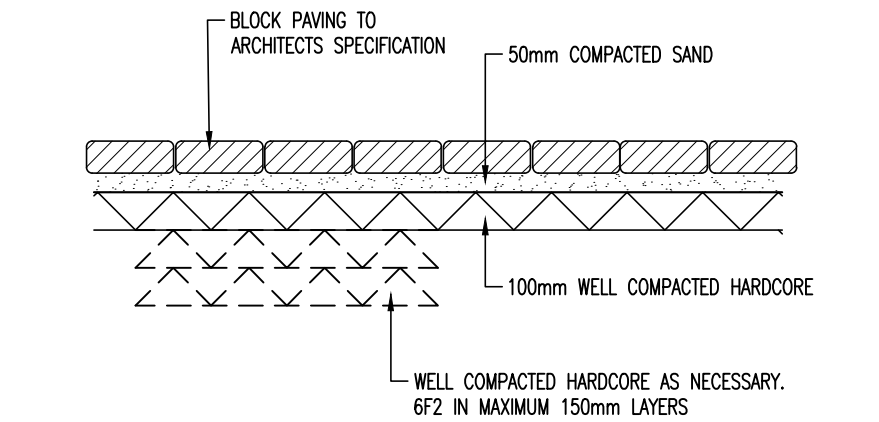
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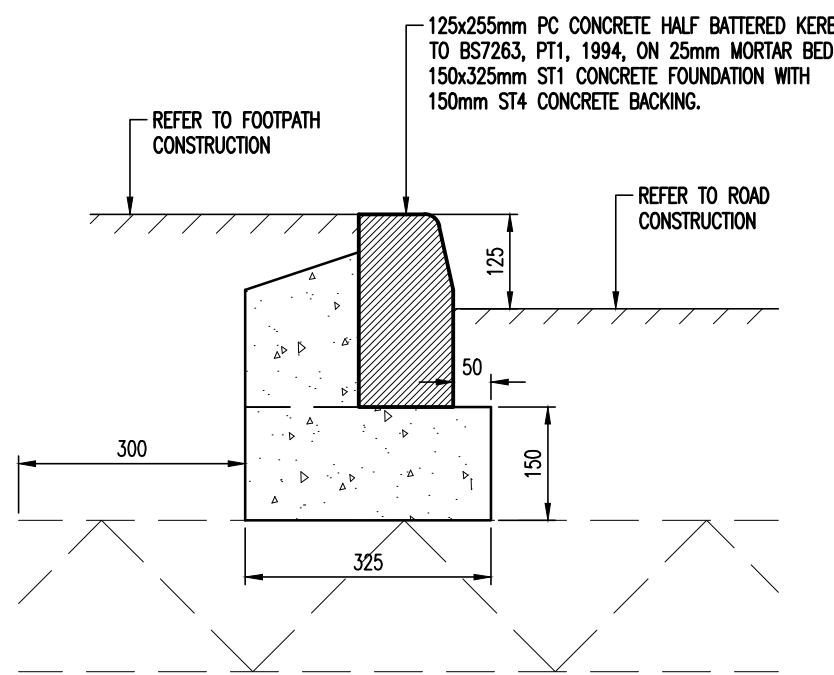
PRIVATE ACCESS ROAD / CAR PARK CONSTRUCTION
SCALE 1:20



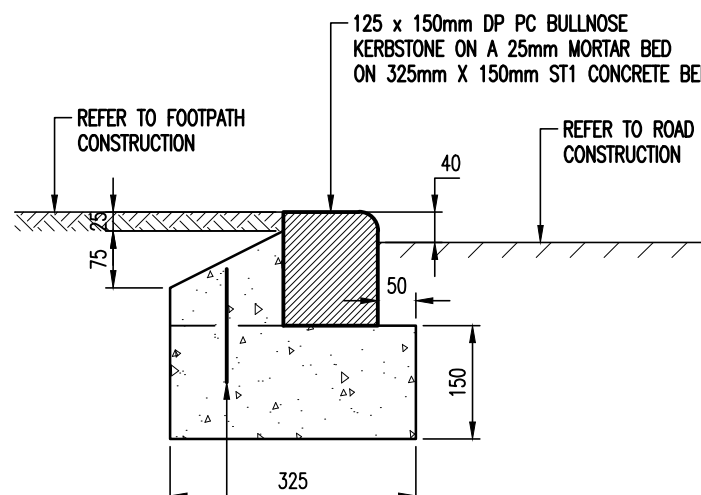
CAR PARK CONSTRUCTION IN TREE ROOT PROTECTION ZONE
SCALE 1:20



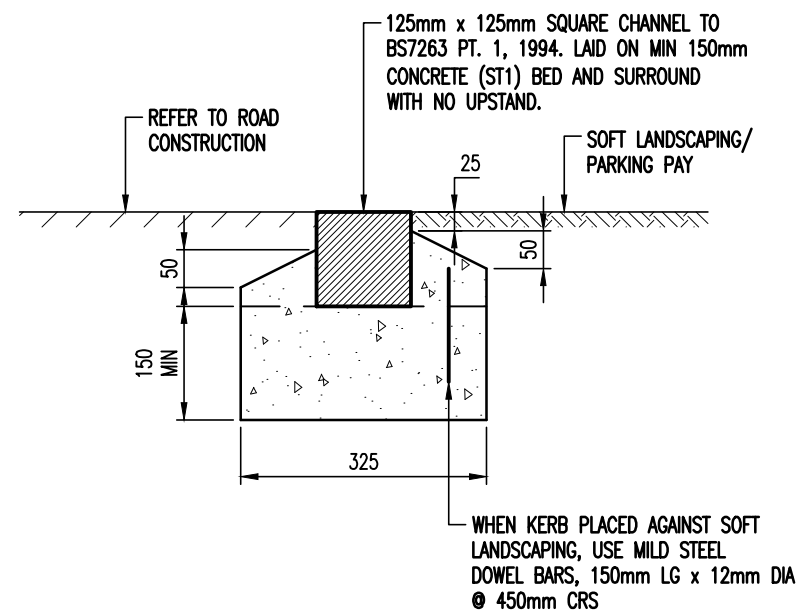
TYPICAL SECTION THROUGH PRIVATE FOOTPATH CONSTRUCTION (BLOCK PAVING)
SCALE 1:20



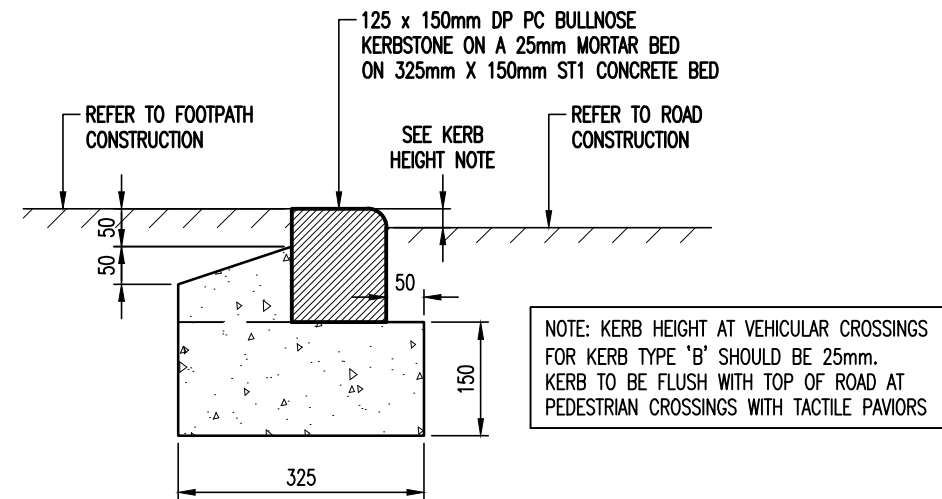
STANDARD KERB (TYPE A)
SCALE 1:10



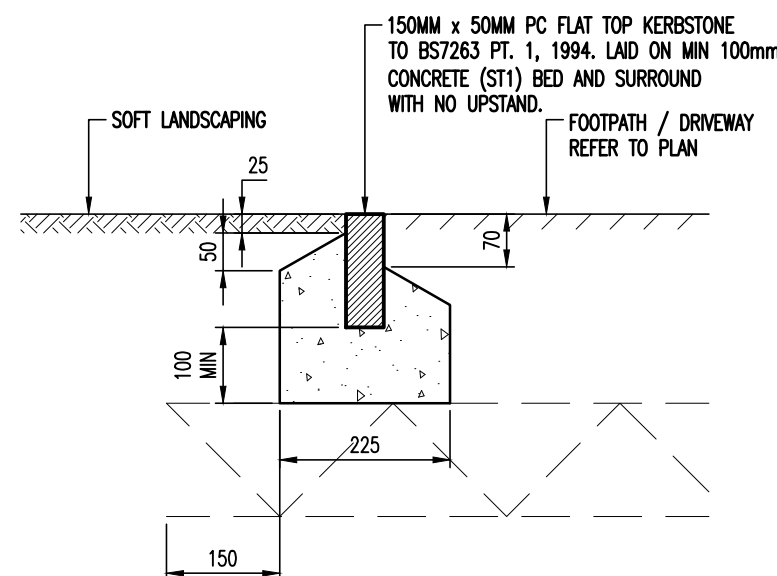
BULLNOSE KERB (TYPE B1)
SCALE 1:10
(AGAINST SOFT LANDSCAPING)



CHANNEL KERB (TYPE D)
SCALE 1:10



BULLNOSE KERB (TYPE B)
SCALE 1:10



EDGING KERB (TYPE C)
SCALE 1:10

P03	CAR PARK CONSTRUCTION AMENDED	18.11.22	AJB	DS
P02	DETAILS UPDATED	27.09.22	AJB	DS
P01	PRELIMINARY ISSUE	30.11.21	AJB	DS
Rev	Description	Date	By	Chkd

Drawing Status

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Project **CHORLTON IRISH CLUB
CHORLTON-CUM-HARDY**

Drawing Title
**TYPICAL EXTERNAL WORKS
DETAILS - PRIVATE**

Date NOV. 21	Drawn By AJB	Checked By DS
Scale at A1 AS SHOWN	Project Number 31296	Revision
Sheet Number 31296-SUT-ZZ-XX-DR-C- 6800	P03	

Network Details

Manhole Schedule

Manhole	Catchment Area (ha)	Diameter (m)	Type	CL (m)	IL (m)	Depth To Soffit (m)	Easting (m)	Northing (m)
PS1	0.025	0.450	Type E	28.259	27.650	0.459	381708.085	393607.435
PS6	0.009	0.450	Type E	28.308	27.575	0.633	381743.608	393590.696
PS2	0.000	0.450	Type E	28.282	26.875	1.182	381734.491	393594.992
PS3	0.013	0.450	Type E	28.168	26.575	1.368	381724.942	393574.730
PS5	0.094	0.450	Type E	28.268	27.000	1.043	381694.981	393588.838
PS4	0.000	1.200	Type E	28.168	26.154	1.489	381716.193	393578.852
PS7	0.000	0.600	Type E	28.186	26.351	1.610	381713.775	393573.875
PS8	0.000	0.600	Type E	28.431	27.300	0.981	381710.467	393567.864

Pipe Schedule

Pipe Number	US Manhole	US IL (m)	DS Manhole	DS IL (m)	Diameter (m)	Length (m)	Gradient (1:x)	Roughness (mm)	US Depth To Soffit (m)	DS Depth To Soffit (m)
1.000	PS1	27.650	PS2	26.950	0.150	29.191	41.7	0.600	0.459	1.182
2.000	PS6	27.575	PS2	27.000	0.100	10.079	17.5	0.600	0.633	1.182
1.001	PS2	26.875	PS3	26.575	0.225	22.400	74.7	0.600	1.182	1.368
1.002	PS3	26.575	PS4	26.454	0.225	9.672	79.9	0.600	1.368	1.489
3.000	PS5	27.000	PS4	26.454	0.225	23.445	42.9	0.600	1.043	1.489
1.003	PS4	26.454	PS7	26.351	0.225	5.533	53.7	0.600	1.489	1.610
1.004	PS7	27.369	PS8	27.300	0.150	6.862	100.0	0.600	0.667	0.981

Outfall Details

Outfall Manhole PS8 : Free Discharge

Flow Control Details

Tank Structure at Manhole PS7

Tank Invert (m)	Tank Height (m)	Void Ratio (%)	Area (m2)	Effective Area (m2) Area x Void Ratio	Max Storage (m3) Effective Area x Height	Infil Base (m/hr)	Infil Side (m/hr)	Safety Factor
26.351	0.800	95.00	130.000	123.500	98.800	0.15300000	0.15300000	2.00

Simulation Settings

FSR: M5-60=18.00, R=0.38, Locale=England and Wales

Summer (Cv: 0.75), Winter (Cv: 0.84)

Global Time of Entry: 5.0 mins

Durations (mins): 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Periods (yrs) + Climate Change: (100, +45%)

Simulated Rainfall Events

Storm	Average Intensity (mm/hr)	Runoff Continuity %	Flow Continuity %	Storm	Average Intensity (mm/hr)	Runoff Continuity %	Flow Continuity %
100Yr +45% 15Min Winter	125.198	0.00	-0.59	100Yr +45% 720Min Winter	8.207	0.00	0.00
100Yr +45% 15Min Summer	125.198	0.00	-0.16	100Yr +45% 960Min Summer	6.550	0.00	0.00
100Yr +45% 30Min Winter	83.057	0.00	-0.45	100Yr +45% 960Min Winter	6.550	0.00	0.00
100Yr +45% 30Min Summer	83.057	0.00	-0.46	100Yr +45% 1440Min Summer	4.758	0.00	0.00
100Yr +45% 60Min Summer	52.627	0.00	-0.35	100Yr +45% 1440Min Winter	4.758	0.00	0.00
100Yr +45% 60Min Winter	52.627	0.00	-0.30	100Yr +45% 2160Min Summer	3.451	0.00	0.00
100Yr +45% 120Min Winter	32.289	0.00	-0.15	100Yr +45% 2160Min Winter	3.451	0.00	0.00
100Yr +45% 120Min Summer	32.289	0.00	-0.19	100Yr +45% 2880Min Summer	2.744	0.00	0.00
100Yr +45% 180Min Summer	23.964	0.00	-0.14	100Yr +45% 2880Min Winter	2.744	0.00	0.00
100Yr +45% 180Min Winter	23.964	0.00	-0.10	100Yr +45% 4320Min Summer	1.984	0.00	0.00
100Yr +45% 240Min Summer	19.288	0.00	-0.12	100Yr +45% 4320Min Winter	1.984	0.00	0.00
100Yr +45% 240Min Winter	19.288	0.00	-0.09	100Yr +45% 5760Min Summer	1.575	0.00	0.00
100Yr +45% 360Min Winter	14.084	0.00	-0.04	100Yr +45% 5760Min Winter	1.575	0.00	0.00
100Yr +45% 360Min Summer	14.084	0.00	-0.07	100Yr +45% 7200Min Summer	1.315	0.00	0.00
100Yr +45% 480Min Summer	11.260	0.00	-0.05	100Yr +45% 7200Min Winter	1.315	0.00	0.00
100Yr +45% 480Min Winter	11.260	0.00	-0.03	100Yr +45% 8640Min Summer	1.135	0.00	0.00
100Yr +45% 600Min Summer	9.464	0.00	-0.04	100Yr +45% 8640Min Winter	1.135	0.00	0.00
100Yr +45% 600Min Winter	9.464	0.00	0.00	100Yr +45% 10080Min Winter	1.002	0.00	0.00
100Yr +45% 720Min Summer	8.207	0.00	0.00	100Yr +45% 10080Min Summer	1.002	0.00	0.00

Simulation Results

Return Period Yrs: 100.0

Climate Change %: 45

Manholes

Manhole	Critical Storm	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Flood (m3)	Status
PS1	15 min Winter	8	27.725	0.075	13.902		OK
PS6	15 min Winter	8	27.616	0.041	5.056		OK
PS2	15 min Winter	8	26.960	0.085	18.719		OK
PS3	120 min Winter	92	26.735	0.160	2.234		OK
PS5	15 min Winter	8	27.132	0.132	52.638		OK
PS4	120 min Winter	92	26.735	0.581	6.625		Surcharged
PS7	120 min Winter	92	26.734	0.383	6.629		OK
PS8	10080 min Summer	-1	27.300	0.000	0.000		Outfall

Conduits

Pipe No.	Critical Storm	Peak (mins)	US Manhole	DS Manhole	Flow Depth (m)	Velocity (m/s)	Flow (l/s)	Flow / Capacity	Status
1.000	15 min Winter	8	PS1	PS2	0.074	1.565	13.690	0.496	OK
2.000	15 min Winter	8	PS6	PS2	0.041	1.681	5.029	0.346	OK
1.001	15 min Winter	8	PS2	PS3	0.094	1.175	18.589	0.309	OK
1.002	120 min Winter	92	PS3	PS4	0.193	0.062	2.236	0.038	OK
3.000	15 min Winter	8	PS5	PS4	0.166	1.668	52.406	0.659	OK
1.003	120 min Winter	92	PS4	PS7	0.225	0.167	6.629	0.093	OK
1.004	120 min Winter	92	PS7	PS8	0.000	0.000	0.000	0.000	OK

From: [Perry, Graham](#)
To: [Dave Samuels](#); [Lucy Furber](#)
Cc: [Aljafarawi, Mohammed](#); [Wastewater Developer Services](#); [J.Turner@southwayhousing.co.uk](#); [tom.williams@neoprojects.co.uk](#); [Graham Swanson](#)
Subject: RE: genq 4200047444 Cross Road, Chorlton
Date: 17 March 2022 14:26:25
Attachments: [image001.png](#)
[image002.png](#)
[image003.png](#)
[image004.png](#)
[image005.png](#)

Hello Dave

I can confirm that this is acceptable. A build over agreement is not necessary if the 3m offset is maintained but we would ask that the foundation is designed in a way so that it does not impart loading onto the sewer and vibration is minimised when carrying out the construction works.

Thanks

Graham Perry



Graham Perry
Development Engineer
Developer Services
M: 07557 577548
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From: Dave Samuels [mailto:dave.samuels@sutcliffe.co.uk]
Sent: 17 March 2022 14:23
To: Perry, Graham <Graham.Perry@uuplc.co.uk>; Lucy Furber <LFurber@iceniprojects.com>
Cc: Aljafarawi, Mohammed <Mohammed.Aljafarawi@uuplc.co.uk>; Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>; J.Turner@southwayhousing.co.uk; tom.williams@neoprojects.co.uk; Graham Swanson <graham.swanson@sutcliffe.co.uk>
Subject: RE: genq 4200047444 Cross Road, Chorlton
Importance: High

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Hi Graham

Thanks for clarification.

We have met, as a Design Team, and will review the building layout based on a **3.675m** easement measured from the centre line of the 1350 DIA sewer giving a 3m distance from the edge of the

sewer.

Can you please confirm this is acceptable to UU ?

Your quick response to this would be appreciated as the architect is currently ongoing with their layout review.

Any queries please do not hesitate to call.

Regards
Dave Samuels
BEng(Hons) CEng MStructE MaPS
Executive Associate

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From: Perry, Graham <Graham.Perry@uuplc.co.uk>
Sent: 16 March 2022 10:21
To: Lucy Furber <L.Furber@iceniprojects.com>
Cc: Aljafarawi, Mohammed <Mohammed.Aljafarawi@uuplc.co.uk>; Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>; Dave Samuels <dave.samuels@sutcliffe.co.uk>; J.Turner@southwayhousing.co.uk; tom.williams@neoprojects.co.uk; Graham Swanson <graham.swanson@sutcliffe.co.uk>
Subject: RE: genq 4200047444 Cross Road, Chorlton

Hello All

To avoid confusion the easement must be measured from the edge of the pipe the building. Our

preference would be to have a 5m offset between the edge of the pipe to the building but we would accept a 3m offset between the building and edge of the pipe.

Thanks

Graham Perry



Graham Perry

Development Engineer

Developer Services

M: 07557 577548

T: 01925 679405

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From: Perry, Graham

Sent: 16 March 2022 10:09

To: 'Lucy Furber' <LFurber@iceniprjects.com>

Cc: Aljafarawi, Mohammed <Mohammed.Aljafarawi@uuplc.co.uk>; Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>; 'Dave Samuels' <dave.samuels@sutcliffe.co.uk>;

'J.Turner@southwayhousing.co.uk' <J.Turner@southwayhousing.co.uk>;

'tom.williams@neoprojects.co.uk' <tom.williams@neoprojects.co.uk>;

'graham.swanson@sutcliffe.co.uk' <graham.swanson@sutcliffe.co.uk>

Subject: RE: genq 4200047444 Cross Road, Chorlton

Hello All

Following a meeting yesterday on another similar scheme our position has changed and it is clear that there is a reluctance to allow build overs of significant assets , we must therefore reject any proposal to build over the sewer crossing the site.

Although we would prefer to see a minimum easement of 5m, the minimum we would accept would be a 3m easement in line with Building Regulations.

Thanks

Graham Perry



Graham Perry

Development Engineer

Developer Services

M: 07557 577548

T: 01925 679405

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From: Lucy Furber [<mailto:LFurber@iceniprojects.com>]
Sent: 14 March 2022 20:27
To: Perry, Graham <Graham.Perry@uuplc.co.uk>
Cc: Aljafarawi, Mohammed <Mohammed.Aljafarawi@uuplc.co.uk>; Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Subject: RE: genq 4200047444 Cross Road, Chorlton

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Hi Graham,

Many thanks for your response, it is appreciated.

If the build over agreement is not possible and we have to amend the scheme and still make the May committee, receiving a response from you by Friday 25th March would give us time to amend the plans, and time for the statutory consultees time to review.

If we can aim for Friday 25th I would be eternally grateful but please advise if this isn't possible.

Kind regards

Lucy

Lucy Furber
Planner, Planning

telephone: 0161 509 2844
mobile: 07825 517 718
email: LFurber@iceniprojects.com



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From: Perry, Graham <Graham.Perry@uuplc.co.uk>

Sent: 11 March 2022 20:17

To: Lucy Furber <LFurber@iceniprojects.com>

Cc: Aljafarawi, Mohammed <Mohammed.Aljafarawi@uuplc.co.uk>; Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>

Subject: FW: genq 4200047444 Cross Road, Chorlton

Hello Lucy

I cannot give a specific date for the approval/rejection of the build over but I will try to have the scheme prioritized to that you have a firm answer well before the May deadline date.

Do you have a 'must have a response' date?

Thanks

Graham Perry



Graham Perry
Development Engineer
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From: Lucy Furber [<mailto:LFurber@iceniprojects.com>]

Sent: 09 March 2022 09:17

To: Perry, Graham <Graham.Perry@uuplc.co.uk>

Cc: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>

Subject: RE: genq 4200047444 Cross Road, Chorlton

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Dear Graham,

I hope you're well,

I have been forwarded this email chain as I am the planning agent for the application at Cross Road, Chorlton, where up until now, I have left conversations regarding the potential for a build over agreement to Dave Samuels at Sutcliffe. However I have now picked up this matter as it has become more of a planning issue.

The email from you in the chain below indicates that a build over may be feasible, however we are still waiting on comments from the principal engineer. In the meantime, separate to the project team's conversations with you via Dave Samuels, Manchester City Council received the statutory planning consultee feedback from UU last week (attached). Having spoken on the phone to the Developer Engineer who wrote the consultee response letter, Mohammed Aljafarawi, it is clear that the letter doesn't reflect the most recent conversations we've been having with you separately. Mohammed said he would try and speak to you about this application once you returned from leave, but I'm not sure if he has.

In light of the UU comments, the Planning Officer at MCC has asked that the determination of the application is delayed until the 31st May planning committee (it was meant to be March) in order to get this matter resolved. I would therefore be grateful if you can please advise when a principle civil engineer will be able to carry out a full assessment of the details?

I appreciate the service is extremely busy at the moment, and you have been very helpful throughout this process to date, therefore even just an indication of when the full assessment may be carried out would be greatly appreciated, so we can update the planning officer on progress and I can manage client expectations with regards to timescales. It would be fantastic if we could make the May planning committee as this is the only outstanding matter we need to resolve.

I am available to discuss via mobile all week if you are able to provide any updates.

Kind regards,

Lucy

Lucy Furber
Planner, Planning

telephone: 0161 509 2844
mobile: 07825 517 718
email: LFurber@iceniprojects.com



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From: Dave Samuels <dave.samuels@sutcliffe.co.uk>

Sent: 01 March 2022 17:01

To: Ian Jones <ian@viewassociates.co.uk>; Jonathan Turner <J.Turner@southwayhousing.co.uk>; Tom Williams <tom.williams@neoprojects.co.uk>

Cc: Graham Swanson <graham.swanson@sutcliffe.co.uk>

Subject: FW: genq 4200047444 Cross Road, Chorlton

All

Please see below email from Graham Perry at United Utilities.

Although positive we still need to wait for UU's Principle Engineer comments.

Regards
Dave Samuels
BEng(Hons) CEng MStructE MaPS
Executive Associate

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From: Perry, Graham <Graham.Perry@uuplc.co.uk>
Sent: 01 March 2022 16:04
To: Dave Samuels <dave.samuels@sutcliffe.co.uk>
Cc: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Subject: FW: genq 4200047444 Cross Road, Chorlton

Hello Dave

I have reviewed the information submitted by Ian and believe that a build over may be feasible but will have to go our principle civil engineer for full assessment.

I have now passed the information over to the relevant team and awaiting a response. Please be aware that the workload for the principle engineer is quite high at the moment

Thanks

Graham Perry



Graham Perry

Development Engineer
Developer Services

M: 07557 577548

T: 01925 679405

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From: Dave Samuels [<mailto:dave.samuels@sutcliffe.co.uk>]

Sent: 17 February 2022 14:52

To: Perry, Graham <Graham.Perry@uuplc.co.uk>; Wastewater Developer Services
<WastewaterDeveloperServices@uuplc.co.uk>

Subject: FW: Cross Road, Chorlton

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Hi Graham

Just had a Progress Meeting on the above referenced project and following Ian Jones' email below (12th January) can you please respond as this item is critical to the proposed layout / planning application that is currently running

Regards
Dave Samuels
BEng(Hons) CEng MStructE MaPS
Executive Associate

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From: Ian Jones

Sent: 12 January 2022 17:02

To: Graham.Perry@uuplc.co.uk

Subject: Cross Road, Chorlton

Graham

Firstly I hope you had a good Christmas break.

You may recall we had a conversation pre Christmas regarding the existing sewer on the above site and I emailed you (as attached) on the 15/12.

I now attach, as promised, a set of documents prepared by Sutcliffe, the engineers for the project. These documents describe how the foundations will be formed to respect the sewer easement and how the sewer can be maintained by excavation if required.

We are aware that Sutcliffe have previously asked for a relaxation on the easement in this regard and that has been refused. I am now writing to ask if that decision can possible be revisited for the following reasons.

- We have been working on the planning submission for this site for over 2 years. That has involved a significant amount of design work for an extensive design team, extensive agreement with 3rd parties, something like 5 pre app meeting with Manchester City Council Planning department, consultation with elected members, consultation with councillors together with a full public consultations. This process has lead to a completed design which is now ready for submission to the planning authority.
- The application is for 100% affordable housing which in its own right is a considerable achievement.
- We only became aware of the fact that the building sits over the sewer easement in one corner after the engineers undertook an full sewer survey. Up until that point we had been working to the sewer records and we have designed the building to respect the sewer

easement on the sewer records.

- And finally the sewer has recently been maintained without excavation and that will need to be the case in the future because this specific sewer already passes under houses and building on the other side of the road.

I am more than happy to run through any matters on the phone or at a meeting but we would be very grateful if this matter can be looked at again to see if there is any scope to amend the decision or have some form of compromise.

Your assistance is much appreciated.

Regards

Ian

Ian Jones

B.A. (Hons) Dip. Arch RIBA

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