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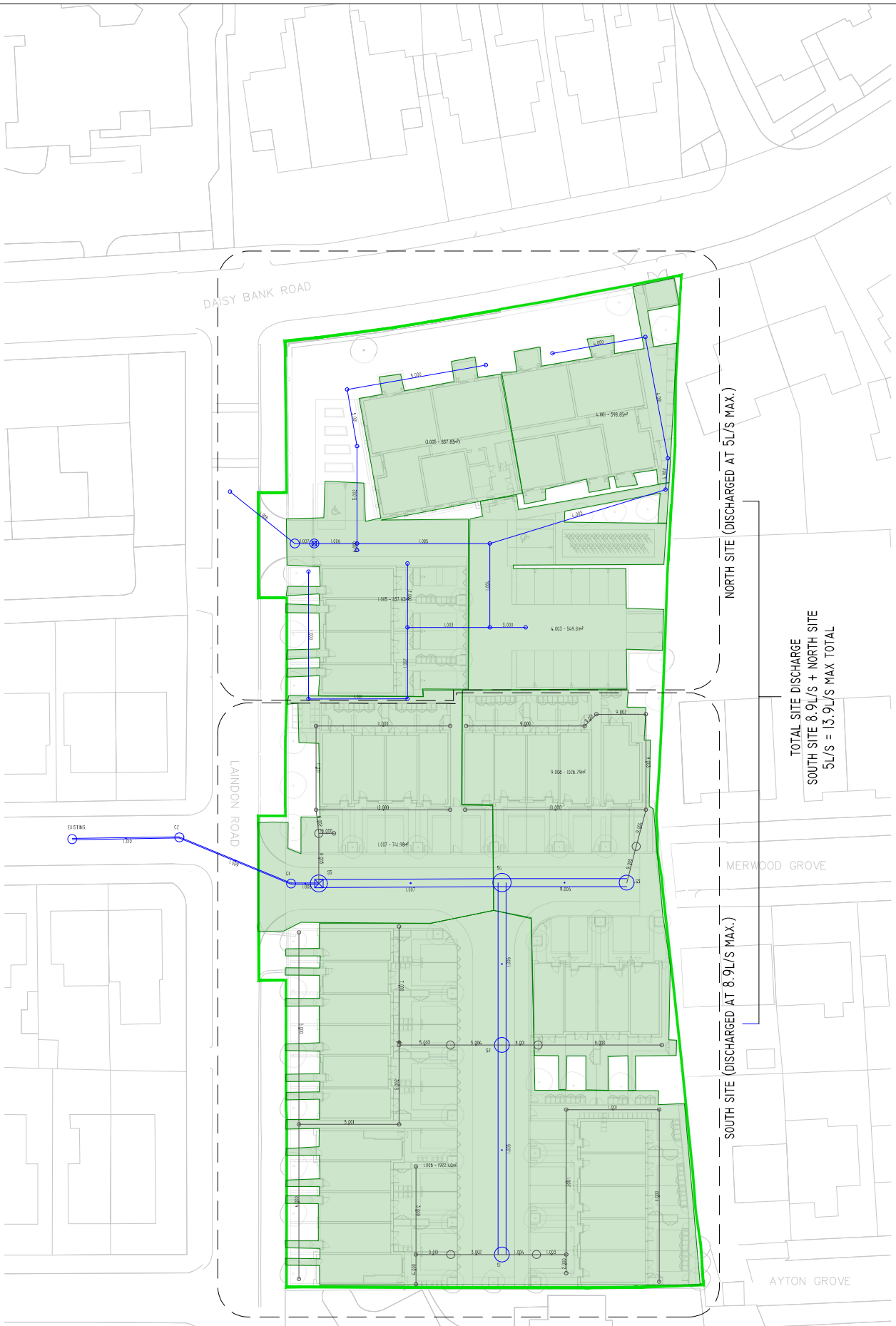
Victoria Park Probation Centre

South Site

Hydraulic Calculations
Condition Discharge
October 2023

	Initials	Signature	Date
Prepared by:	SJB	[redacted]	10 / 23
Checked by:	CM	[redacted]	10 / 23
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SITE REFERENCES SHOWING
LOCATION AND DRAINED AREAS
1:250 @ A1
1:500 @ A3

PRELIMINARY
SUBJECT TO APPROVAL FROM UU AND LLFA

REV	COMMENTS	DATE	BY

BMC

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CLIENT: BRITANNIA GROUP

PROJECT: VICTORIA PARK PROBATION CENTRE

TITLE: REFERENCE PLAN
INC. PRIVATE DRAINAGE
(DRAINED AREA PLAN)

DRAWN BY	CHECKED BY	SCALE	DATE
SJB	CM	1:250	08/23

DRAWING NUMBER: J7986/ D08

REVISION:

South Site

Results for critical storm by water level

No flooding

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Surface Network 1

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	2	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.370	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Surface Network 1

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.282	4-8	0.087

Total Area Contributing (ha) = 0.369

Total Pipe Volume (m^3) = 64.394

Network Design Table for Surface Network 1

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	22.583	0.282	80.0	0.000	5.00	0.0	0.600	o	150	Pipe/Conduit	
1.001	12.204	0.153	80.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.002	19.010	0.238	80.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
2.000	2.450	0.031	80.0	0.000	5.00	0.0	0.600	o	100	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	5.33	46.765	0.000	0.0	0.0	0.0	1.12	19.9	0.0
1.001	50.00	5.52	46.483	0.000	0.0	0.0	0.0	1.12	19.9	0.0
1.002	50.00	5.73	46.255	0.000	0.0	0.0	0.0	1.46	58.2	0.0
2.000	50.00	5.05	46.874	0.000	0.0	0.0	0.0	0.86	6.8	0.0

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Network Design Table for Surface Network 1

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.003	3.893	0.049	80.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.004	4.889	0.054	89.7	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
3.000	11.535	0.144	80.0	0.000	5.00	0.0	0.600	o	100	Pipe/Conduit	
4.000	3.914	0.071	55.2	0.000	5.00	0.0	0.600	o	100	Pipe/Conduit	
3.001	4.581	0.057	80.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
3.002	6.358	0.086	73.9	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.005	27.398	0.068	400.0	0.000	0.00	0.0	0.600	o	900	Pipe/Conduit	
5.000	25.190	0.168	150.0	0.000	5.00	0.0	0.600	o	150	Pipe/Conduit	
6.000	20.307	0.451	45.1	0.000	5.00	0.0	0.600	o	150	Pipe/Conduit	
5.001	13.153	0.088	150.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
5.002	10.412	0.069	150.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
7.000	15.548	0.042	369.6	0.000	5.00	0.0	0.600	o	100	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.003	50.00	5.78	46.018	0.000	0.0	0.0	0.0	1.46	58.2	0.0
1.004	50.00	5.84	45.969	0.000	0.0	0.0	0.0	1.38	54.9	0.0
3.000	50.00	5.22	46.150	0.000	0.0	0.0	0.0	0.86	6.8	0.0
4.000	50.00	5.06	46.219	0.000	0.0	0.0	0.0	1.04	8.2	0.0
3.001	50.00	5.29	45.955	0.000	0.0	0.0	0.0	1.12	19.9	0.0
3.002	50.00	5.38	45.898	0.000	0.0	0.0	0.0	1.17	20.7	0.0
1.005	50.00	6.13	45.057	0.000	0.0	0.0	0.0	1.56	992.7	0.0
5.000	50.00	5.51	46.115	0.000	0.0	0.0	0.0	0.82	14.5	0.0
6.000	50.00	5.23	45.986	0.000	0.0	0.0	0.0	1.50	26.6	0.0
5.001	50.00	5.72	45.460	0.000	0.0	0.0	0.0	1.07	42.4	0.0
5.002	50.00	5.88	45.372	0.000	0.0	0.0	0.0	1.07	42.4	0.0
7.000	50.00	5.66	46.268	0.000	0.0	0.0	0.0	0.39	3.1	0.0

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Network Design Table for Surface Network 1

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
5.003	6.735	0.045	150.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
5.004	6.406	0.045	142.2	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
8.000	16.220	0.203	80.0	0.000	5.00	0.0	0.600	o	100	Pipe/Conduit	
8.001	5.098	0.057	89.3	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.006	21.315	0.053	400.0	0.192	0.00	0.0	0.600	o	900	Pipe/Conduit	
9.000	15.554	0.194	80.0	0.000	5.00	0.0	0.600	o	100	Pipe/Conduit	
9.001	2.169	0.027	80.0	0.000	0.00	0.0	0.600	o	100	Pipe/Conduit	
9.002	6.490	0.081	80.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
9.003	12.553	0.157	80.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
10.000	23.713	0.296	80.0	0.000	5.00	0.0	0.600	o	150	Pipe/Conduit	
9.004	4.963	0.062	80.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
9.005	4.960	0.061	80.8	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
9.006	16.545	0.041	400.0	0.103	0.00	0.0	0.600	o	900	Pipe/Conduit	
1.007	23.283	0.058	400.0	0.074	0.00	0.0	0.600	o	900	Pipe/Conduit	
11.000	17.590	0.220	80.0	0.000	5.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
5.003	50.00	5.99	45.303	0.000	0.0	0.0	0.0	1.07	42.4	0.0
5.004	50.00	6.08	45.054	0.000	0.0	0.0	0.0	1.09	43.5	0.0
8.000	50.00	5.31	46.804	0.000	0.0	0.0	0.0	0.86	6.8	0.0
8.001	50.00	5.39	46.551	0.000	0.0	0.0	0.0	1.06	18.8	0.0
1.006	50.00	6.36	44.988	0.192	0.0	0.0	0.0	1.56	992.7	26.0
9.000	50.00	5.30	46.542	0.000	0.0	0.0	0.0	0.86	6.8	0.0
9.001	50.00	5.34	46.348	0.000	0.0	0.0	0.0	0.86	6.8	0.0
9.002	50.00	5.44	46.271	0.000	0.0	0.0	0.0	1.12	19.9	0.0
9.003	50.00	5.63	46.189	0.000	0.0	0.0	0.0	1.12	19.9	0.0
10.000	50.00	5.35	46.460	0.000	0.0	0.0	0.0	1.12	19.9	0.0
9.004	50.00	5.68	45.958	0.000	0.0	0.0	0.0	1.46	58.2	0.0
9.005	50.00	5.74	45.895	0.000	0.0	0.0	0.0	1.46	57.9	0.0
9.006	50.00	5.92	44.976	0.103	0.0	0.0	0.0	1.56	992.7	13.9
1.007	50.00	6.60	44.935	0.369	0.0	0.0	0.0	1.56	992.7	50.0
11.000	50.00	5.26	46.471	0.000	0.0	0.0	0.0	1.12	19.9	0.0

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Network Design Table for Surface Network 1

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
11.001	11.015	0.138	80.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
12.000	17.590	0.220	80.0	0.000	5.00	0.0	0.600	o	150	Pipe/Conduit	
11.002	3.181	0.039	81.8	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
13.000	1.600	0.013	120.0	0.000	5.00	0.0	0.600	o	300	Pipe/Conduit	
11.003	6.571	0.044	150.0	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.008	4.038	0.027	150.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.009	15.875	0.198	80.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.010	14.050	0.176	80.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
11.001	50.00	5.42	46.251	0.000	0.0	0.0	0.0	1.12	19.9	0.0
12.000	50.00	5.26	46.423	0.000	0.0	0.0	0.0	1.12	19.9	0.0
11.002	50.00	5.46	46.038	0.000	0.0	0.0	0.0	1.45	57.5	0.0
13.000	50.00	5.02	45.950	0.000	0.0	0.0	0.0	1.43	101.4	0.0
11.003	50.00	5.55	45.937	0.000	0.0	0.0	0.0	1.28	90.6	0.0
1.008	50.00	6.69	44.877	0.369	0.0	0.0	0.0	0.82	14.5«	50.0
1.009	50.00	6.92	44.850	0.369	0.0	0.0	0.0	1.12	19.9«	50.0
1.010	50.00	7.13	44.652	0.369	0.0	0.0	0.0	1.12	19.9«	50.0

Free Flowing Outfall Details for Surface Network 1

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
1.010	EXISTING	46.740	44.476	0.000	1200	0

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Simulation Criteria for Surface Network 1

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	0.000
Hot Start (mins)	0	Inlet Coeffiecient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	18.000	Storm Duration (mins)	30
Ratio R	0.370		

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Online Controls for Surface Network 1

Hydro-Brake® Optimum Manhole: S5, DS/PN: 1.008, Volume (m³): 26.3

Unit Reference	MD-SHE-0131-8900-1500-8900
Design Head (m)	1.500
Design Flow (l/s)	8.9
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	131
Invert Level (m)	44.877
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.500	8.9
Flush-Flo™	0.441	8.8
Kick-Flo®	0.926	7.1
Mean Flow over Head Range	-	7.7

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	4.7	1.200	8.0	3.000	12.3	7.000	18.5
0.200	8.0	1.400	8.6	3.500	13.3	7.500	19.1
0.300	8.6	1.600	9.2	4.000	14.1	8.000	19.7
0.400	8.8	1.800	9.7	4.500	14.9	8.500	20.3
0.500	8.8	2.000	10.2	5.000	15.7	9.000	20.8
0.600	8.7	2.200	10.6	5.500	16.4	9.500	21.4
0.800	8.1	2.400	11.1	6.000	17.1		
1.000	7.3	2.600	11.5	6.500	17.8		

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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	0.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.368
Region	England and Wales	Cv (Summer)	0.750
M5-60 (mm)	18.000	Cv (Winter)	0.840

Margin for Flood Risk Warning (mm)	150.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	ON
DVD Status	OFF
Inertia Status	OFF

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440
Return Period(s) (years)	100
Climate Change (%)	40

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
1.000	PS1	15 Summer	100	+40%					46.765
1.001	PS2	15 Summer	100	+40%					46.483
1.002	PS3	120 Winter	100	+40%					46.396
2.000	PS4	15 Summer	100	+40%					46.874
1.003	PS5	120 Winter	100	+40%	100/30 Winter				46.397
1.004	PS8	120 Winter	100	+40%	100/30 Summer				46.397
3.000	PS9	120 Winter	100	+40%	100/30 Winter				46.397
4.000	PS24	120 Winter	100	+40%	100/30 Winter				46.397
3.001	PS25	120 Winter	100	+40%	100/15 Winter				46.397
3.002	PS26	120 Winter	100	+40%	100/15 Winter				46.397
1.005	S1	120 Winter	100	+40%	100/15 Winter				46.397
5.000	PS27	120 Winter	100	+40%	100/30 Winter				46.396
6.000	PS28	120 Winter	100	+40%	100/15 Winter				46.396
5.001	PS35	120 Winter	100	+40%	100/15 Summer				46.396
5.002	PS36	120 Winter	100	+40%	100/15 Summer				46.397
7.000	PS37	120 Winter	100	+40%	100/60 Winter				46.397
5.003	PS38	120 Winter	100	+40%	100/15 Summer				46.397
5.004	PS39	120 Winter	100	+40%	100/15 Summer				46.397
8.000	PS40	15 Summer	100	+40%					46.804

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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	PS1	-0.150	0.000	0.00			0.0	OK	
1.001	PS2	-0.150	0.000	0.00			0.0	OK	
1.002	PS3	-0.084	0.000	0.00			0.1	OK	
2.000	PS4	-0.100	0.000	0.00			0.0	OK	
1.003	PS5	0.154	0.000	0.01			0.4	SURCHARGED	
1.004	PS8	0.203	0.000	0.02			0.5	SURCHARGED	
3.000	PS9	0.147	0.000	0.00			0.0	SURCHARGED	
4.000	PS24	0.078	0.000	0.00			0.0	SURCHARGED	
3.001	PS25	0.291	0.000	0.01			0.1	SURCHARGED	
3.002	PS26	0.349	0.000	0.01			0.2	SURCHARGED	
1.005	S1	0.440	0.000	0.00			0.9	SURCHARGED	
5.000	PS27	0.131	0.000	0.00			0.0	SURCHARGED	
6.000	PS28	0.261	0.000	0.00			0.0	SURCHARGED	
5.001	PS35	0.711	0.000	0.01			0.3	SURCHARGED	
5.002	PS36	0.799	0.000	0.01			0.3	SURCHARGED	
7.000	PS37	0.029	0.000	0.01			0.0	SURCHARGED	
5.003	PS38	0.869	0.000	0.01			0.3	SURCHARGED	
5.004	PS39	1.118	0.000	0.02			0.5	SURCHARGED	
8.000	PS40	-0.100	0.000	0.00			0.0	OK	

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Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
8.001	PS41	15 Summer	100	+40%					46.551
1.006	S2	120 Winter	100	+40%	100/15 Summer				46.397
9.000	PS53	15 Summer	100	+40%					46.542
9.001	PS54	120 Winter	100	+40%					46.396
9.002	PS55	120 Winter	100	+40%					46.396
9.003	PS56	120 Winter	100	+40%	100/60 Winter				46.397
10.000	PS57	15 Summer	100	+40%					46.460
9.004	PS58	120 Winter	100	+40%	100/30 Summer				46.397
9.005	PS59	120 Winter	100	+40%	100/15 Winter				46.397
9.006	S3	120 Winter	100	+40%	100/15 Summer				46.397
1.007	S4	120 Winter	100	+40%	100/15 Summer				46.397
11.000	PS60	15 Summer	100	+40%					46.471
11.001	PS61	120 Winter	100	+40%					46.396
12.000	PS62	15 Summer	100	+40%					46.423
11.002	PS63	120 Winter	100	+40%	100/30 Winter				46.396
13.000	PS64	120 Winter	100	+40%	100/60 Winter				46.396
11.003	PS65	120 Winter	100	+40%	100/30 Winter				46.396
1.008	S5	120 Winter	100	+40%	100/15 Summer				46.397
1.009	C1	120 Winter	100	+40%					44.924
1.010	C2	120 Winter	100	+40%					44.726

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
8.001	PS41	-0.150	0.000	0.00			0.0	OK	
1.006	S2	0.509	0.000	0.04			27.1	SURCHARGED	
9.000	PS53	-0.100	0.000	0.00			0.0	OK	
9.001	PS54	-0.052	0.000	0.01			0.0	OK	
9.002	PS55	-0.024	0.000	0.00			0.1	OK	
9.003	PS56	0.057	0.000	0.01			0.1	SURCHARGED	
10.000	PS57	-0.150	0.000	0.00			0.0	OK	
9.004	PS58	0.214	0.000	0.01			0.3	SURCHARGED	
9.005	PS59	0.276	0.000	0.01			0.4	SURCHARGED	
9.006	S3	0.521	0.000	0.03			15.2	SURCHARGED	
1.007	S4	0.562	0.000	0.08			52.3	SURCHARGED	
11.000	PS60	-0.150	0.000	0.00			0.0	OK	
11.001	PS61	-0.005	0.000	0.01			0.1	OK	
12.000	PS62	-0.150	0.000	0.00			0.0	OK	
11.002	PS63	0.133	0.000	0.01			0.3	SURCHARGED	
13.000	PS64	0.146	0.000	0.17		63	9.6	SURCHARGED	
11.003	PS65	0.159	0.000	0.15			8.9	SURCHARGED	
1.008	S5	1.370	0.000	0.82			8.9	SURCHARGED	
1.009	C1	-0.076	0.000	0.49			8.9	OK	
1.010	C2	-0.076	0.000	0.49			8.9	OK	

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Technical Specification

Control Point	Head (m)	Flow (l/s)
Primary Design	1.500	8.900
Flush-Flo™	0.441	8.847
Kick-Flo®	0.926	7.084
Mean Flow		7.739

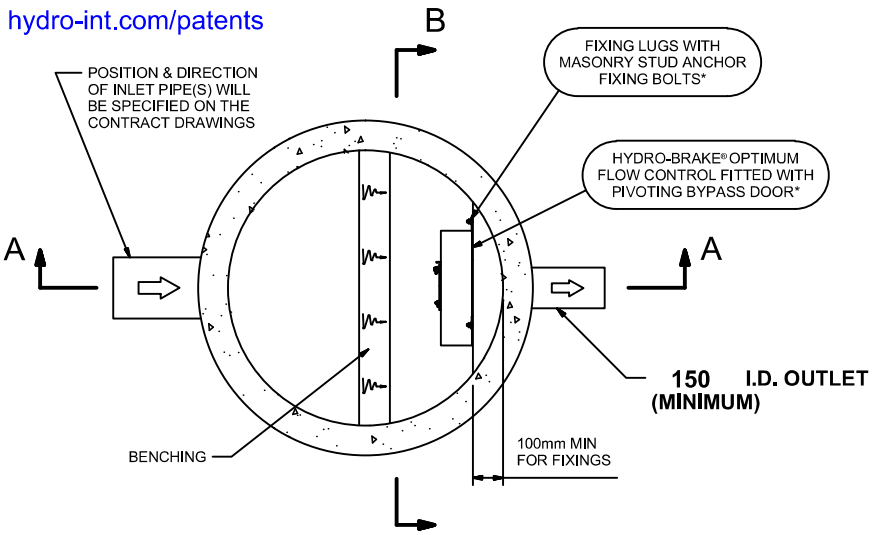
Hydro-Brake® Optimum Flow Control including:

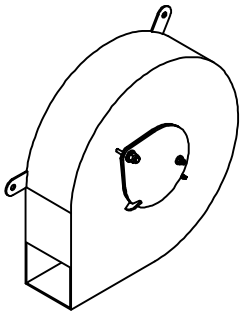
- 3 mm grade 304L stainless steel
- Integral stainless steel pivoting by-pass door allowing clear line of sight through to outlet, c/w stainless steel operating rope
- Beed blasted finish to maximise corrosion resistance
- Stainless steel fixings
- Rubber gasket to seal outlet
- Indicative Weight: 30 kg

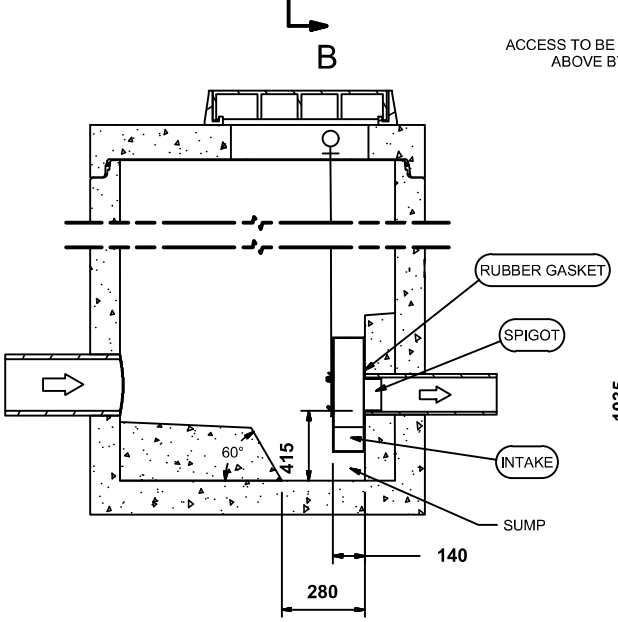
BBA APPROVAL INSPECTION TESTING CERTIFICATION CERTIFICATE 08/4596

wrc approved PT/503/0422 Valid: 01/04/2022 - 31/03/2027

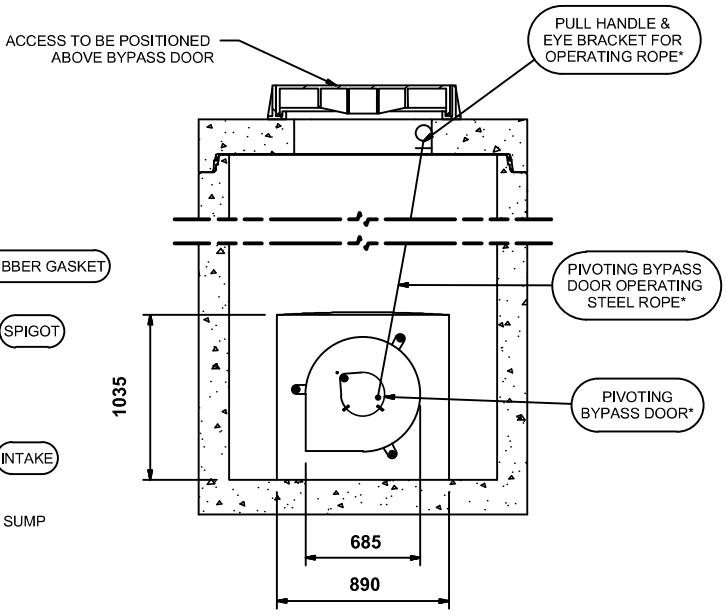
hydro-int.com/patents







SECTION A-A



SECTION B-B

IMPORTANT: ⬭ LIMIT OF HYDRO INTERNATIONAL SUPPLY
THE DEVICE WILL BE HANDED TO SUIT SITE CONDITIONS
FOR SITE SPECIFIC DETAILS AND MINIMUM CHAMBER SIZE REFER TO HYDRO INTERNATIONAL
ALL CIVIL AND INSTALLATION WORK BY OTHERS
* WHERE SUPPLIED
HYDRO-BRAKE® FLOW CONTROL & HYDRO-BRAKE® OPTIMUM FLOW CONTROL ARE REGISTERED TRADEMARKS FOR FLOW CONTROLS DESIGNED AND MANUFACTURED EXCLUSIVELY BY HYDRO INTERNATIONAL

THIS DESIGN LAYOUT IS FOR ILLUSTRATIVE PURPOSES ONLY. NOT TO SCALE.

DESIGN ADVICE

The head/flow characteristics of this SHE-0131-8900-1500-8900 Hydro-Brake® Optimum Flow Control are unique. Dynamic hydraulic modelling evaluates the full head/flow characteristic curve.
The use of any other flow control will invalidate any design based on this data and could constitute a flood risk.

Hydro International®

DATE	11/08/2023 14:12
SITE	Victoria Park
DESIGNER	Simon Berry
REF	J7986 S

SHE-0131-8900-1500-8900
Hydro-Brake® Optimum

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