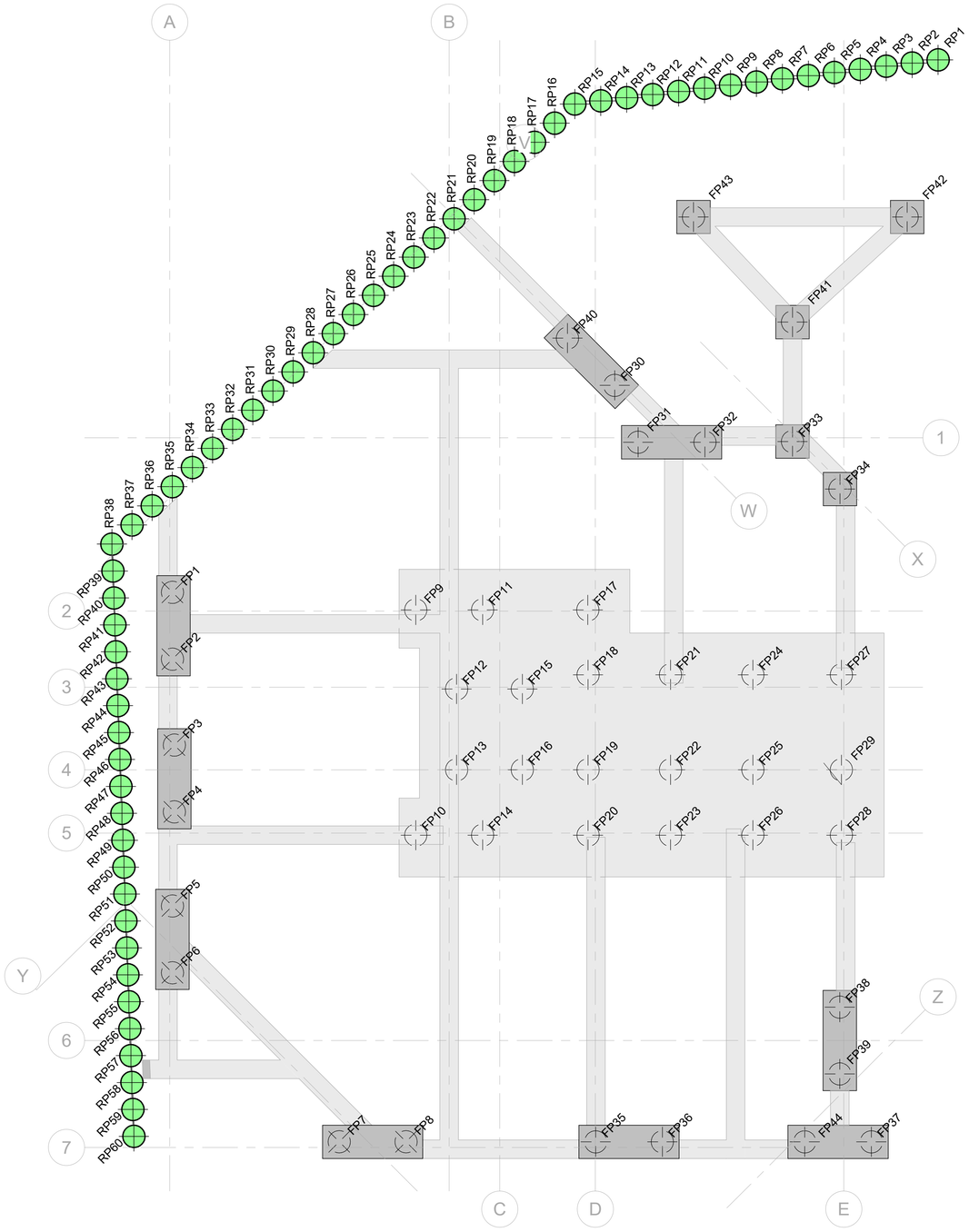


1 PILING GA

1 : 75

Retaining Pile Schedule			
Pile	Diameter	Pile Cut Off Level	Depth
RP1	600	41595	11000
RP2	600	41595	11000
RP3	600	41595	11000
RP4	600	41595	11000
RP5	600	41595	11000
RP6	600	41595	11000
RP7	600	41595	11000
RP8	600	41595	11000
RP9	600	41595	11000
RP10	600	41595	11000
RP11	600	41595	11000
RP12	600	41595	11000
RP13	600	41595	11000
RP14	600	41595	11000
RP15	600	41595	11000
RP16	600	41595	11000
RP17	600	41595	11000
RP18	600	41595	11000
RP19	600	41595	11000
RP20	600	41595	11000
RP21	600	41595	11000
RP22	600	41595	11000
RP23	600	41595	11000
RP24	600	41595	11000
RP25	600	41595	11000
RP26	600	41595	11000
RP27	600	41595	11000
RP28	600	41595	11000
RP29	600	41595	11000
RP30	600	41595	11000
RP31	600	41595	11000
RP32	600	41595	11000
RP33	600	41595	11000
RP34	600	41595	11000
RP35	600	41595	11000
RP36	600	41595	11000
RP37	600	41595	11000
RP38	600	41595	11000
RP39	600	41595	11000
RP40	600	41595	11000
RP41	600	41595	11000
RP42	600	41595	11000
RP43	600	41595	11000
RP44	600	41595	11000
RP45	600	41595	11000
RP46	600	41595	11000
RP47	600	41595	11000
RP48	600	41595	11000
RP49	600	41595	11000
RP50	600	41595	11000
RP51	600	41595	11000
RP52	600	40845	11000
RP53	600	40845	11000
RP54	600	40845	11000
RP55	600	40845	11000
RP56	600	40845	11000
RP57	600	40845	11000
RP58	600	40845	11000
RP59	600	40845	11000
RP60	600	40845	11000

The proposed method of piling is CFA. This method of piling is an augered piling solution where the piling rig drills into the ground using a screwing action. This method of piling forces arising to the surface and as the auger is withdrawn the shaft is filled with concrete using a shift within the centre of the auger to form the pile. CFA piling prevents contamination of the sub soils as no potentially contaminated material is push down into the underlying strata as may be the case with driven piles etc. The down the hole method of concrete placement also prevents potentially contaminated material falling down the hole

Foundation Pile Schedule							
Pile	Diameter	Pile Cut Off Level	Depth	Axial Dead (kN)	Axial Imposed (kN)	Axial Wind (kN)	Shear (Kn)
FP1	600		11000	700	325	0	0
FP2	600		11000	700	325	0	0
FP3	600		11000	650	275	0	0
FP4	600		11000	650	275	0	0
FP5	600		11000	750	285	0	0
FP6	600		11000	750	285	0	0
FP7	600		11000	560	400	0	0
FP8	600		11000	560	400	0	0
FP9	600		22000	710	460	-910/900	+/-160
FP10	600		22000	930	630	-75/75	+/-160
FP11	600		22000	1450	750	-400/350	+/-160
FP12	600		22000	475	170	-250/125	+/-160
FP13	600		22000	440	210	-250/125	+/-160
FP14	600		22000	1300	840	-75/75	+/-160
FP15	600		22000	650	180	-350/175	+/-160
FP16	600		22000	1300	410	-775/550	+/-160
FP17	600		22000	1100	270	-910/800	+/-160
FP18	600		22000	1525	410	-350/300	+/-160
FP19	600		22000	1390	390	-500/550	+/-160
FP20	600		22000	760	250	-375/275	+/-160
FP21	600		22000	870	200	-250/225	+/-160
FP22	600		22000	1330	350	-275/300	+/-160
FP23	600		22000	1230	390	-375/425	+/-160
FP24	600		22000	1540	320	-575/575	+/-160
FP25	600		22000	1480	360	-225/425	+/-160
FP26	600		22000	1250	350	-400/475	+/-160
FP27	600		22000	1152	200	-550/880	+/-160
FP28	600		22000	990	240	-350/550	+/-160
FP29	600		22000	1210	250	-425/800	+/-160
FP30	600		11000	700	410	0	0
FP31	600		11000	450	200	0	0
FP32	600		11000	450	200	0	0
FP33	600		11000	540	185	0	0
FP34	600		11000	540	140	0	0
FP35	600		11000	850	375	0	0
FP36	600		11000	850	375	0	0
FP37	600		11000	550	250	0	0
FP38	600		11000	465	150	0	0
FP39	600		11000	465	150	0	0
FP40	600		11000	700	410	0	0
FP41	600		11000	700	375	0	0
FP42	600		11000	700	375	0	0
FP43	600		11000	700	375	0	0
FP44	600		11000	700	375	0	0

Piling Notes

Piles are to be designed by the specialist Subcontractor. Design is to be based on the unfactored vertical loads and shears (and moments where applicable) shown in the table.

All piles to be CFA (350 diameter assumed).

All concrete is to be mechanically vibrated.

All loose reinforcement is to conform with BS 4449:2005 and all mesh reinforcement to conform with BS 4483:2005. All reinforcement is to be high tensile Type 2 deformed bars denoted `H` mild steel plain bars denoted `R`. Bars to be left protruding 50 x bar diameter above pile cut-off level.

The Contractor is responsible for locating all services and informing the main Contractor of any clashes with the piling works.

The Contractor is to ensure access to the site is adequate for plant and materials that the piling platform is suitable.

For existing ground conditions refer to the following SI report produced by e3p
Reference: 15-854-R4-1
Date: July 24

If the Piling Contractor requires further Ground Investigation information they should make the Client aware at the earliest opportunity.

Contractor to have excavator on standby during the piling works to grub out any obstructions that may be encountered.

Piles shall be accurately installed in the correct position. A maximum tolerance of 75mm on position of piles shall be accepted on the discretion of the Engineer. The maximum tolerance to the perpendicular of vertical piles shall be 40mm variance in 3m provided that moments or forces created by inaccurate driving are taken directly by the piles within the accepted factor of safety.

The setting out of grid lines shall be done by the Contractor who shall provide the permanent datum level and back mark for the purpose of establishing cut off levels. Setting out of individual piles shall be carried out by the Contractor.

The Contractor shall accurately log each day total piling length, empty boring length, delay time, obstruction time etc. cross reference number.

The Contractor shall provide the Engineer with a copy of the driving records detailing items required by the Engineer as the work proceeds.

Piles shall be concreted up to a level 300mm above the required cut off level.

Testing of piles to be agreed and to assumed to be based on Medium Risk in Table 3.1 of Federation of Piling Specialists Hand Book for Pile Load Testing.

All piles are required to be Integrity Tested. 4No. load bearing piles (1/100) to be Working Load Tested, 2No. tank piles and 2No. steel structure piles

Working Pile Load Tests to be carried out to 1.5x working load to be carried out, locations to be proposed by contractor based on site constraints. Pile deflection should be monitored witrh maximum 15mm settlement permitted.

DO NOT SCALE THIS DRAWING A1

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Revisions

Rev	Date	Description	Drawn	Checked
P1	29/05/25	Preliminary Issue	KS	JG

STAGE 4 ISSUE FOR BSR APPROVAL NOT FOR CONSTRUCTION

DRAWING STATUS: Preliminary

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CLIENT: Merebridge Construction

PROJECT: Store Street, Manchester

DRAWING TITLE: PILING GA

DATE CREATED	DRAWING SCALE:	DRAWN BY:	CHECKED:	QA CATEGORY:
May 2025	As indicated	SB	JG	

Drawing Number: LV0118-TC- SS-FND-DR-S-201 REV: P1