



**DRAINAGE DESIGN REPORT
FOR THE
PROPOSED DPV BUILDING
HEINEKEN, MANCHESTER**



DECEMBER 2020



ENGINEERING



ARCHITECTURE



PROJECT MANAGEMENT



CDM SERVICES



Document Control

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		Signature		
		Filename		
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		Name		
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1 Introduction

This Drainage Design Report has been produced to demonstrate the development and design of the drainage for the disposal of surface water generated from the proposed Dual Purpose Vessel (DPV) building, at the existing Heineken site off Denmark Road in Manchester. The proposed works consist of the re-development of an area of existing service yard, to provide a new DPV building.

The Drainage Design Report is intended to address the requirements of the Lead Local Flood Authority (LLFA) to allow the drainage scheme to be approved prior to the issue of Planning Approval.

This report is prepared solely for the use of Heineken and their advisers.

2 Site information

2.1 Existing Site

The existing Heineken factory is located off Denmark Road, Manchester. A site location plan is attached under **Appendix A**.

The site is bound by Princess Road to the west with a supermarket and retail park beyond. To the north, the site is bounded by Greenheys Lane with a residential area beyond. Charles Halle Road and Denmark Road run adjacent to the eastern boundary with residential housing beyond, along with a primary school and a commercial unit. To the south the site is bounded by Moss Lane East, with a number of churches, social clubs and commercial units beyond.

The Bridgewater Canal runs approximately 1.75Km to the north west of the site and the River Irwell runs approximately 1.85Km to the north west of the site.

Both vehicular and pedestrian access can be taken into the site via Denmark Road.

The Heineken site is approximately 7.5 Ha in area and accommodates the main factory and production buildings, along with the main office building. The remainder of the site is used as car parking, lorry parking and storage. The site is covered in impermeable surfacing, being a mixture of concrete and asphalt pavements, with the exception of a relatively small area of landscaping at the site entrance and to some boundaries.

The proposed DPV building is located towards the south of the site, adjacent to other existing vessels. A separate planning application has been submitted for a new site service road which will be located adjacent to the DPV building, to maintain access through the site.

An existing site layout plan showing the location of the DPV building and existing ground levels is attached under **Appendix B**.

2.2 Proposed Development

The proposed development is a new DPV block, consisting of a single storey building with an 8 no. vertical Dual Purpose Vessel (DPV) installation. The proposal also includes an adjacent open access vertical escape ladder and overhead pipe bridge/walkway to link to the adjacent vessels.

The DPV block accommodates an area of approximately 485m². The development area is flat lying having a ground level between approximately 32.90m AOD and

32.75m AOD, and is currently occupied by a mix of tarmac and concrete service yard and road.

Both vehicular and pedestrian access into the site will continue to be taken via Denmark Road. The proposed layout plan is attached under **Appendix C**.

2.3 Flood Zone Classification

The Environment Agency flood mapping for Rivers shows that the proposed DPV Block is located within a Low Risk Flood Zone 2 area which has an annual probability of flooding from rivers of between 1 in 100 and 1 in 1000 (a 1% - 0.1% chance of flooding in any given year).

There are no allocated flood storage areas shown within the vicinity of the site area.

The proposed re-development, being the DPV Block, is considered to be classed as 'general industry' and would therefore be classed as a Less Vulnerable development in accordance with Table 2 : Flood Risk Vulnerability Classification, of the PPG, associated with the NPPF. Table 3 : Flood Risk Vulnerability and Flood Zone Compatibility, of the PPG, associated with the NPPF shows that, an exceptions test is not required and the development is appropriate for a Less Vulnerable development within Flood Zone 2.

The Environment Agency have provided flood mapping for reservoirs which shows that the whole site will potentially be affected due to the failure of reservoirs. It is noted by the Environment Agency that the failure of a reservoir is very unlikely and that the maps provided reflect a credible worst case scenario for the purpose of emergency planning.

The Government long term flood risk information service shows that the area of the proposed DPV block has a low risk of flooding due to surface water. A low risk of flooding means that it has a chance of flooding of between 0.1% and 1% each year. It is noted that the government website providing this information states that it is difficult to predict flooding from surface water as rainfall location and volume area difficult to forecast. In addition, local features can greatly affect the chance and severity of flooding. The site has no history of flooding from surcharged sewers or sheet run off and is shown to be at low risk of flooding.

A copy of the Environment Agency information is attached under **Appendix D**.

2.4 Existing Drainage Systems

United Utilities sewer records show a 2450mm brick combined sewer and a 750mm by 1020mm brick combined sewer run in a westerly direction within the carriageway of Moss Lane East. These sewers are shown to have a depth to invert of approximately 8m and 3.5m respectively.

620mm by 860mm brick culvert combined sewer runs in a southerly direction within Princess Road. The sewer records do not provide a depth for the sewer. The sewer connects onto the 2450mm combined sewer indicated above to continue to run in a westerly direction away from the site

A 610mm by 930mm combined sewer runs in a southerly direction within the site and under the proposed site service road. This sewer is shown to have a depth to invert of approximately 3m and runs into the 750mm by 1020mm brick combined sewer indicated above. This sewer also serves the adjacent residential development located to the east of the site.

A 680mm brick combined sewer, shown to increase to a 620mm by 770mm sewer, runs in a northerly direction within the carriageway of Denmark Road, commencing at the site entrance. This sewer is shown to have a depth to invert of approximately 2.25m. This sewer is shown to have a 750mm combined spur running into the site which stops at a dead end.

A 300mm diameter, 560mm by 710mm, and 600mm diameter combined sewers run into the site from the Police Station to its eastern boundary. These sewers are shown to connect to each other and run in a westerly direction, but are shown to end within the site boundary.

A 1805mm by 3300mm surface water sewer is shown to run in a southerly direction adjacent to the western side of Denmark Road. The sewer is shown to have a depth to invert of approximately 4m and is shown to end at the site entrance.

A copy of the United Utilities sewer records, are attached under **Appendix E**.

3 Development Surface Water Drainage

3.1 Surface Water Discharge Treatment

It is noted that the surface water run off from the DPV building is required to be discharged to the effluent treatment plant (ETP) and treated prior to discharge from the site. This is due to the potential contamination of surface water run off from the vessels, as a result of fobbing due to overactive yeast or glycol discharge from cooling jackets.

This therefore dictates that the surface water generated from the proposals is discharged to sewer, however, it is already known on the site that the ground is not suitable for infiltration and there are no watercourses within the vicinity of the works.

The ETP discharges the treated effluent from the site into the United Utilities combined system in Denmark Road, under the Trade Effluent licence reference : 694TLS26S069. This licence imposes a restricted discharge rate not exceeding 60 l/s with a maximum amount of trade effluent in a 24 hour day not exceeding 5,130m³. The measured average daily discharge during the month of August 2020 was 2,143m³ and during the month of September 2020 was 1,888m³, therefore demonstrating the available discharge capacity.

3.2 Surface Water Drainage Design Criteria

The DEFRA Non-statutory Technical Standards for sustainable drainage systems, in conjunction with the NPPF, requires that for developments that were previously developed, the peak run off rate from the development for the 1 in 1 year rainfall event and the 1 in 100 year rainfall event should be as close as reasonably practical to the green field runoff for the same rainfall event, but should never exceed the rate of discharge from the development prior to redevelopment for that event.

The site is located within the Conurbation Core Critical Drainage Area and any development in the Conurbation Core Critical Drainage Area is identified in the Manchester Trafford and Salford Strategic Flood Risk Assessment as requiring a reduction in surface water run off of at least 50% compared to the existing rates.

The proposed DPV block is approximately 485m² in area and is located in an existing area wholly covered in an impermeable hardstanding surfacing that is currently positively drained into an existing United Utilities sewer running adjacent to the proposed building.

The existing discharge rate for a 1 in 1 year storm has been calculated as 6.6 l/s using the Micro Drainage design program, and 13.0 l/s for a 1 in 100 year storm. A 50% reduction results in an allowable 1 in 1 year discharge of 3.3 l/s and a 1 in 100 year discharge of 6.5 l/s.

The R+D Technical Report W5=074/A/TR1 Revision E, titled Preliminary Rainfall Runoff Management for Developments, produced by the Environment Agency and DEFRA, notes on Page xvi, that the practical minimum limit on the discharge rate from a flow attenuation device is 5 l/s, to provide a satisfactory low flow whilst keeping the risk of blockage to an acceptable level.

The surface water discharge is therefore proposed to be restricted to 5 l/s with attenuation provided to accommodate a 1 in 100 year storm plus a 40% allowance for climate change.

3.3 Attenuation Volume Design

The surface water generated by the proposed flow restriction of 5 l/s is to be attenuated for a 1 in 100 year storm plus a 40% allowance for climate change.

The surface water drainage has been modelled using the Micro Drainage computer programme. The total measured hard standing area of 485m² has been modelled, with the flow control designed as a Hydrobreak having a limited discharge of 5 l/s, and attenuation provided in an oversized pipe.

A copy of the Micro Drainage network details and simulation results, for a 1 in 1 year storm event, 1 in 30 year storm event and a 1 in 100 year storm event plus 40% allowance for climate change, are attached under **Appendix G**.

A copy of the drainage layout drawing is attached under **Appendix H**.

3.4 Flow Paths and Flood Exceedance Routes

The drainage has been designed to accommodate a 1 in 100 year storm plus a 40% allowance for climate change. It is unlikely that this design storm event is exceeded and the system floods under these circumstances.

If the proposed drainage facilities were to fail or their capacity exceeded, the excess surface water would flow towards the north east within the existing road area.

The proposed site levels generally follow the existing contours of the site and run generally towards the north east, thereby maintaining existing flow paths and flood exceedance routes.

3.5 Off Site Works and Consents

All drainage serving the proposed site service road is picked up within the site boundary, therefore no offsite works are required in relation to this surface water drainage system.

An email has been received from United Utilities noting that they do not have any objection to the proposals. A copy of the United Utility response is attached under **Appendix E**.

3.6 Management and Maintenance

The management and maintenance regime for the drainage system will be incorporated into the Heineken current Maintenance and Management Plan.

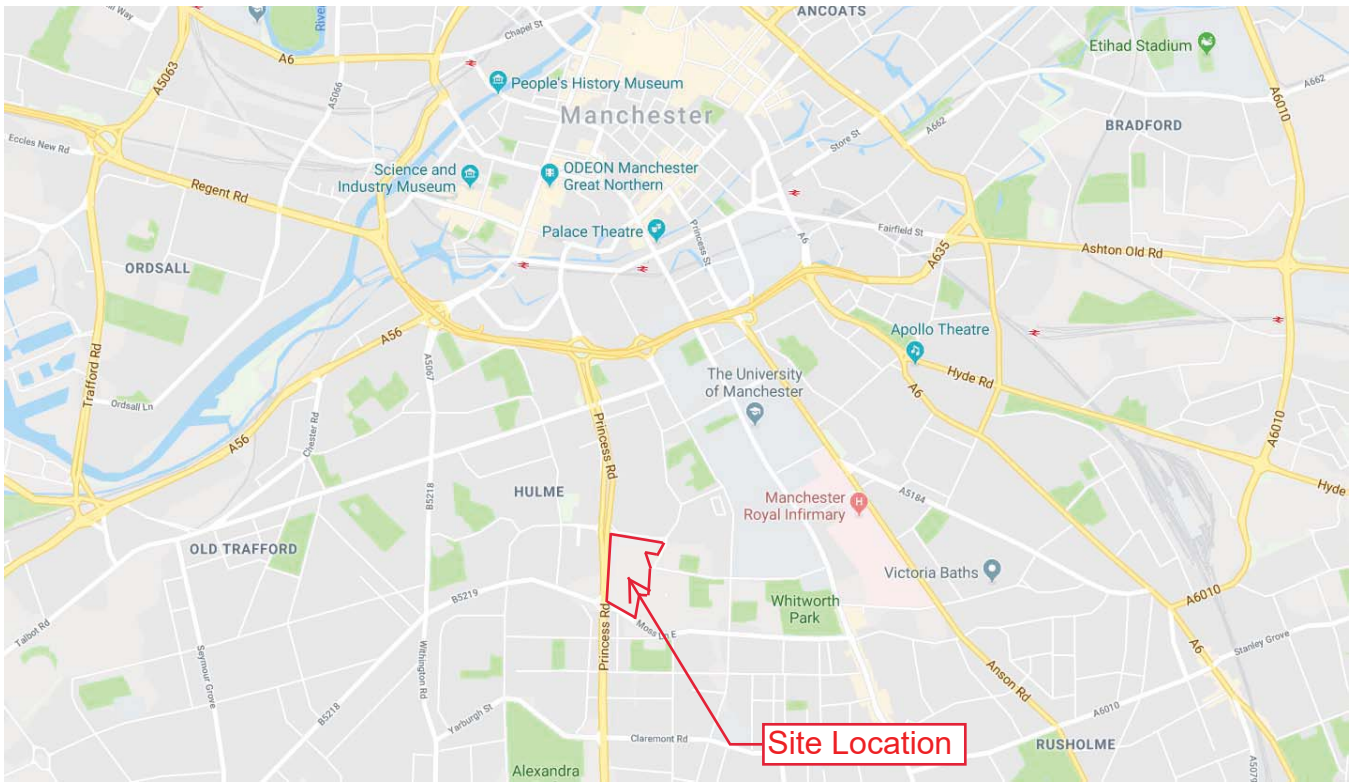
The maintenance and management regime for the proposed drainage systems will include :

- i) Regular site attendance for checking the control structure is clear of litter, leaves, silt and other debris, and is in good working order.
- ii) Regular site attendance for checking inlets and outlets to manhole chambers are free of litter, leaves, silt and other debris.

Appendix A – Site Location Plans

Heineken
Denmark Road, Manchester

Site Location Plan



General Notes:

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2. This drawing is to be read in conjunction with all other relevant Architects, Engineers and specialists drawings, details and specifications.
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4. Dimensions must not be scaled from this drawing.
5. Dimensions marked 'dimension to be site checked' are subject to confirmation by site measurement before construction proceeds.
6. Subcontractor surveys, such as for the purposes of steelwork installation, take precedence over 'site dimensions' indicated on this drawing. All surveys should ensure an accurate fit of components.
7. Temporary works associated with the construction of the project shall be the responsibility of the Principal Contractor.
8. Where any temporary works or scaffolding are required the Principal Contractor shall appoint a competent and suitably experienced Temporary Works Coordinator (TWC). Refer to BS 5975 for guidance on temporary works and the role of a TWC.
9. Calculations and drawings of all Contractor designed structural items associated with NDA's design responsibilities shall be submitted to the Engineer for comment at least 10 days prior to manufacture. Any comments made by NDA do not absolve the Contractor of his design responsibilities.

Heineken Site Boundary
DPV Vessel Boundary

P3	01.12.20	Building extents updated.	AF
P2	02.09.20	Planning Issue.	SW
P1	01.09.20	Preliminary Issue	AF
REV DATE	AMENDMENT		BY

CLIENT



PROJECT

Manchester Brewery
New 8 Vessel DPV Block

DRAWING TITLE

Site Location Plan



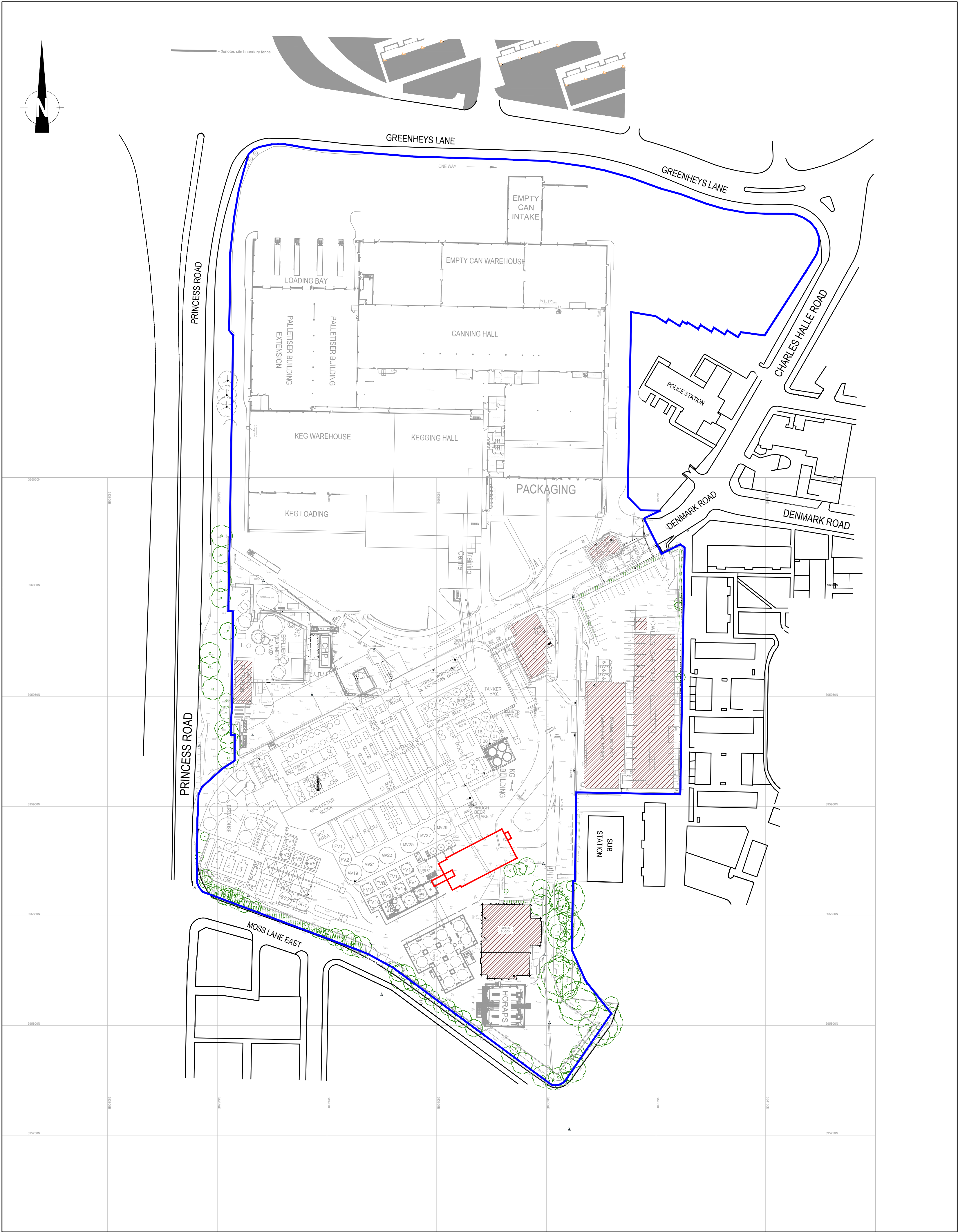
Beech Lawn, Green Lane, Belper, Derby DE56 1BY
Tel: 01773 824414 Fax: 01773 823305
e-mail: enquiries@norder.co.uk

SCALE

1:1000

SHEET SIZE A1

DATE	DRAWN	CHECKED
Sept 2020	AF	RSP
DRAWING NO.		REV
8540-NDA-XX-XX-DR-C-1001		P3



Appendix B – Existing Topographical Survey and DPV Location

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4. Dimensions must not be scaled from this drawing.
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6. Subcontractor surveys, such as for the purposes of steelwork installation, take precedence over 'site dimensions' indicated on this drawing. All surveys should ensure an accurate fit of components.
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Heineken Site Boundary
DPV Vessel Boundary

P2 01.12.20 Building extents updated. AF
P1 01.09.20 Preliminary Issue AF
REV DATE AMENDMENT BY
CLIENT



PROJECT

Manchester Brewery
New 8 Vessel DPV Block

DRAWING TITLE

Existing Topographical Survey

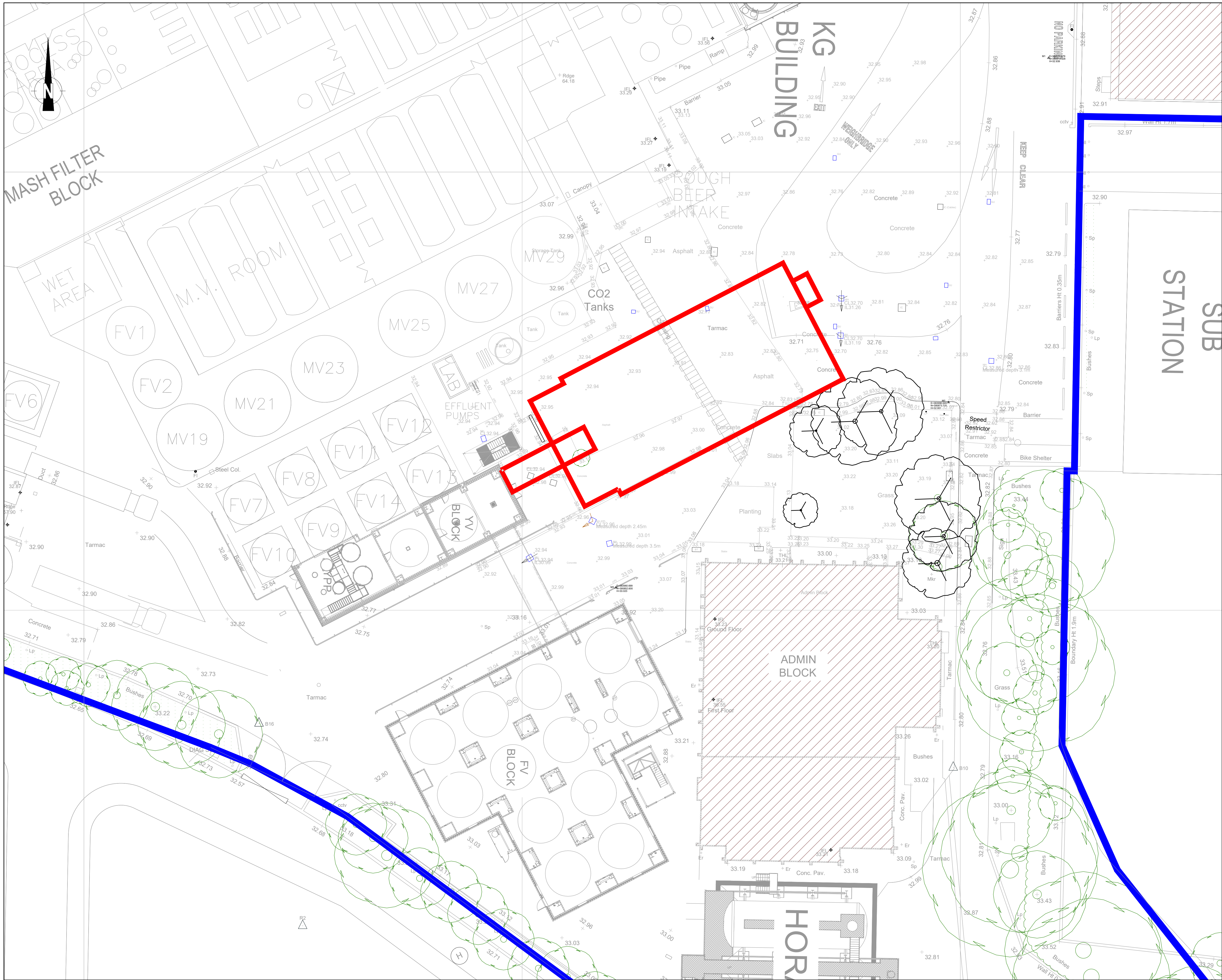


Beech Lawn, Green Lane, Belper, Derby DE56 1BY
Tel: 01773 824414 Fax: 01773 823305
e-mail: enquiries@norder.co.uk

SCALE

1:200 SHEET SIZE A1

DATE DRAWN CHECKED
Sept 2020 AF RSP
DRAWING NO. REV
8540-NDA-XX-XX-DR-C-1002 P2



Appendix C – Proposed Layout Plan

General Notes:

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3. Conflicting information shown on the NDA drawings or discrepancies between the information given by NDA and that provided by others must be referred to NDA before work proceeds.
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9. Calculations and drawings of all Contractor designed structural items associated with NDA's design responsibilities shall be submitted to the Engineer for comment at least 10 days prior to manufacture. Any comments made by NDA do not absolve the Contractor of his design responsibilities.

P3	02.11.20	First Stage Tender Issue	SW
P2	02.09.20	Planning Issue	SW
P1	27.08.20	Issued for Client comments/approval.	SW
REV	DATE	AMENDMENT	BY
	CLIENT		



PROJECT

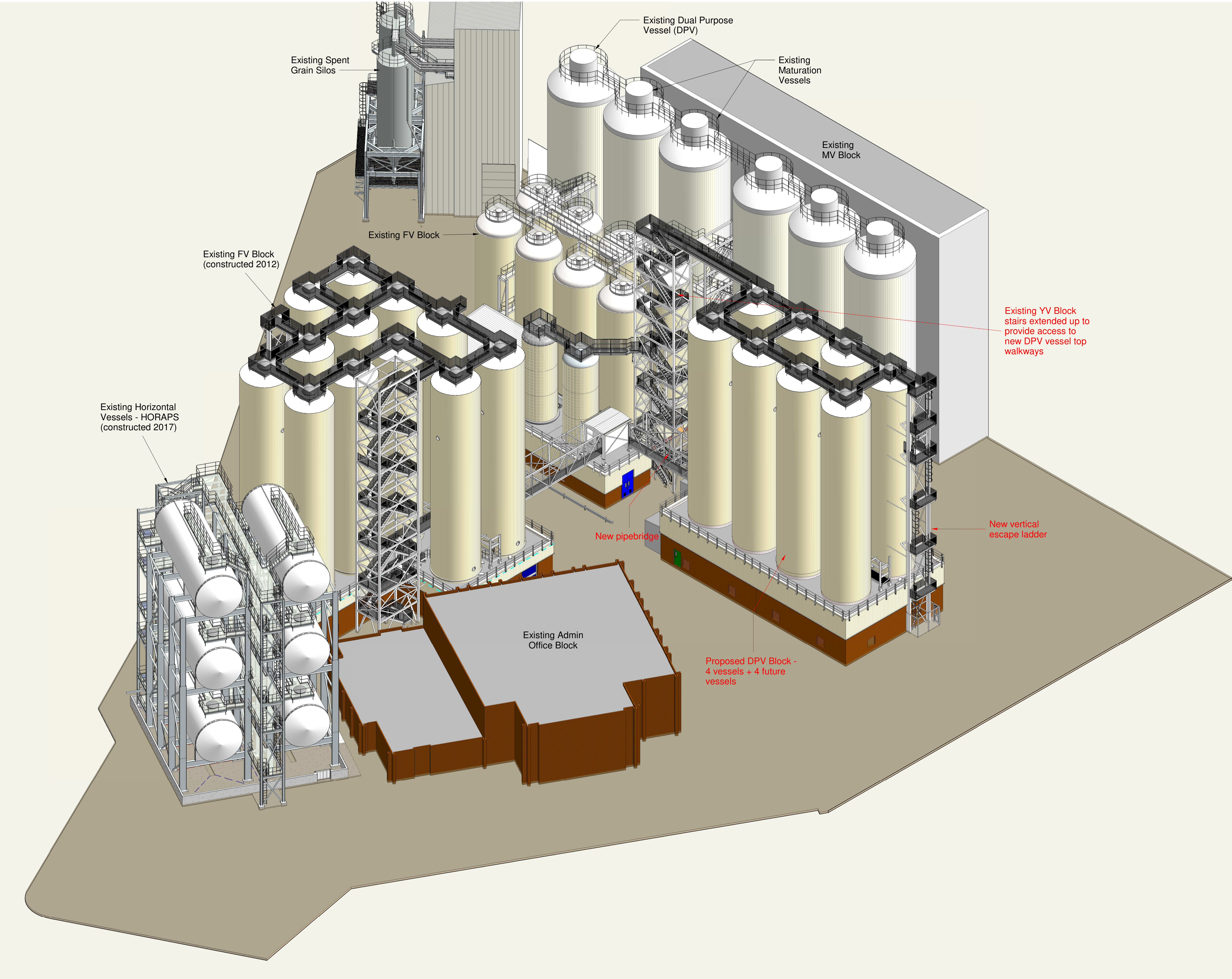
Manchester Brewery
New 4 Vessel DPV Block

DRAWING TITLE

Proposed 3D View

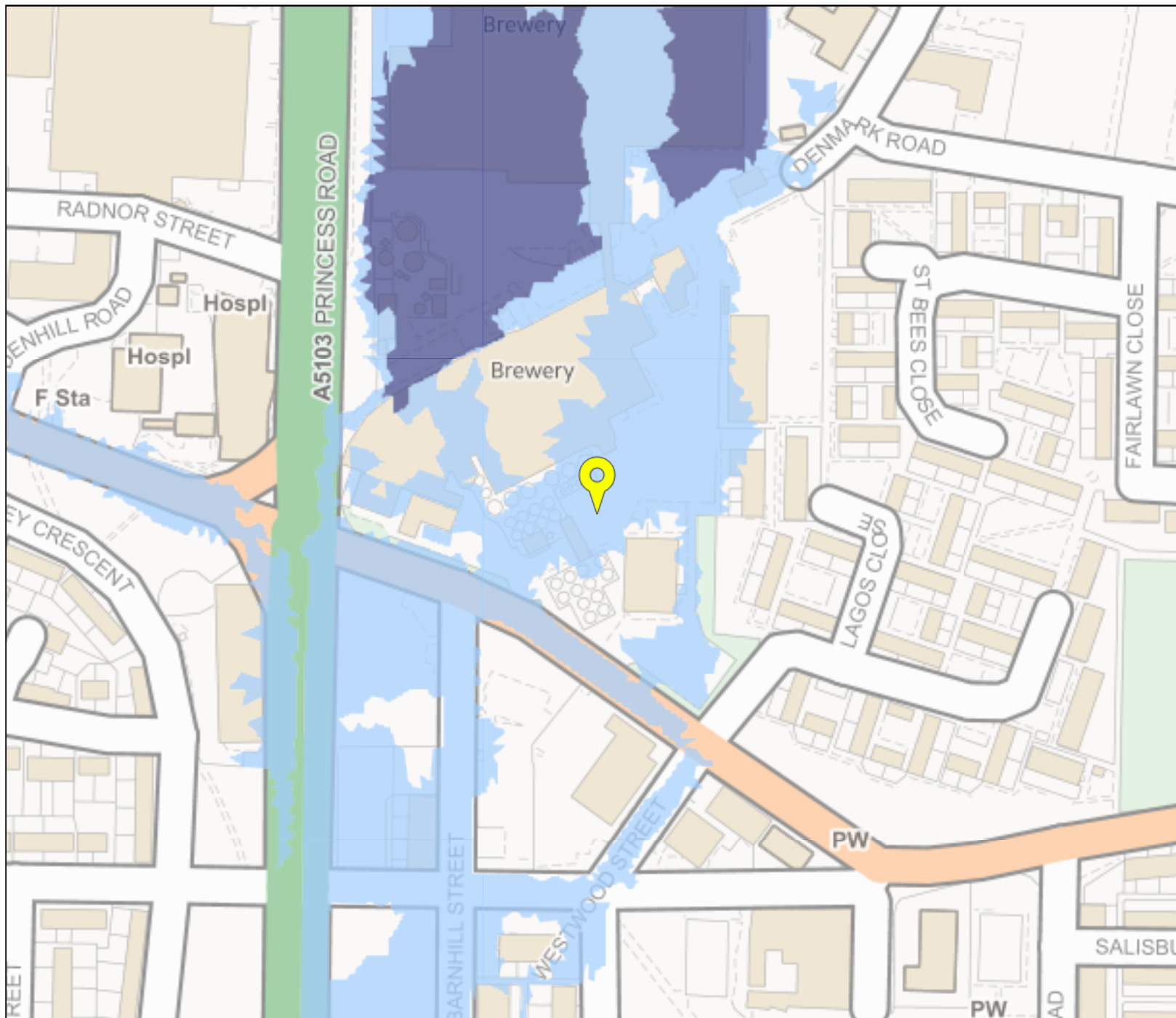

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SCALE			SHEET SIZE A1
DATE	DRAWN	CHECKED	
27 August 2020	SW	RSP	
DRAWING NO.			REV
8540-NDA-XX-XX-DR-A-9002			P3



1 3D View Proposed

Appendix D – Environment Agency Information



Flood map for planning

Your reference
Heineken Man

Location (easting/northing)
383958/395867

Scale
1:2500

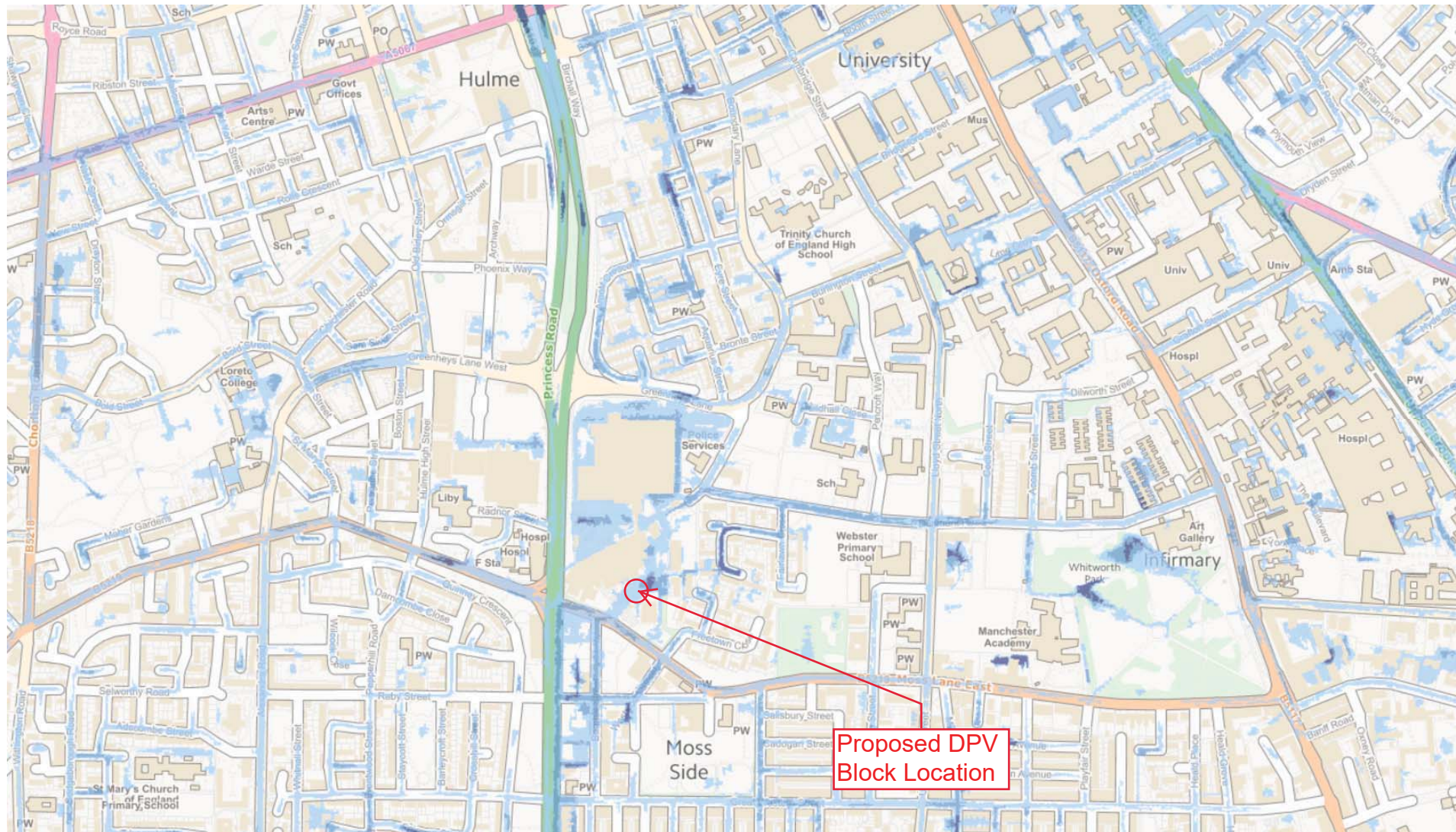
Created
30 Aug 2020 12:59

-  Selected point
-  Flood zone 3
-  Flood zone 3: areas benefitting from flood defences
-  Flood zone 2
-  Flood zone 1
-  Flood defence
-  Main river
-  Flood storage area

0 20 40 60m

Heineken Brewery Manchester

Flood Risk From Surface Water



Reservoir Flood Map



Note - this map provides a general indication of the largest area that might be flooded if a reservoir were to fail and release the water it holds. It is taken from a national assessment and displays a worst case scenario. The map is only intended as a guide and is not a prediction of what will happen.

Important

- This map has been produced for emergency planning purposes and displays a worst case scenario.
- It is not suitable for use at an individual property scale due to the method used.
- This map does not give any information on the likelihood of reservoir flooding or on the depth or speed of floodwaters. It also does not include any smaller reservoirs (which hold less than 25,000 cubic metres of water) or reservoirs commissioned or registered after Spring 2009.
- The information should not be interpreted as stating that the location you are interested in will or won't actually flood, but simply that it is in or not in an area that could be affected by reservoir flooding as shown on the maps.

The area within the red circle could be at risk of flooding from the following reservoirs:

Reservoir Name	Reservoir Owner	Location	Local Authority	Environment Agency Office
Gorton Lower	United Utilities Water plc	389386, 396041	Manchester	Environment Agency - Greater Manchester, Merseysid
Gorton Upper	United Utilities Water plc	389893, 396254	Manchester	Environment Agency - Greater Manchester, Merseysid
Audenshaw No. 3	United Utilities Water plc	390934, 396835	Tameside	Environment Agency - Greater Manchester, Merseysid
Audenshaw No.2	United Utilities Water plc	391643, 396093	Tameside	Environment Agency - Greater Manchester, Merseysid
Audenshaw No.1	United Utilities Water plc	391852, 396483	Tameside	Environment Agency - Greater Manchester, Merseysid

 Maximum extent of flood

1:10,000

Appendix E – United Utility Sewer Record Plans



LEGEND	
Abandoned	Foul
Surface Water	Combined
Public Sewer	Private Sewer
Section 104	Rising Main
Surge Main	Overflow
Water Course	Highway Drain
All point assets follow the standard colour convention: red - combined blue - surface water brown - foul purple - overflow	
- Manhole - Head of System - Extent of Survey - Rodding Eye - Inlet - Discharge Point - Valves - Catchpit - Washout Chamber - Valve - Air Valve - Non Return Valve - Manway - Gully - Cascade - Flow Meter - Hatch Box - Oil Interceptor - Summit - Drop Shaft - Orifice Plate	- Sally Entry Manhole - Outlet - Screen Chamber - Inspection Chamber - Bifurcation Chamber - Lamp Hole - Junction - Saddle - Catchpit - Valve Chamber - Vent Column - Valves Chamber - Penstock Chamber - Network Storage Tank - Sewer Overflow - We Treatment Works - We Pumping Station - Septic Tank - Control Kiosk - Change of Characteristic
MANHOLE FUNCTION	
FO Foul	SW Surface Water
CO Combined	OW Overflow
SEWER SHAPE	
CO Circular	TR Triangular
EG Egg	AR Arch
OV Oval	BA Barrel
FT Flat Top	HO Horseshoe
RE Rectangular	UN Unspecified
SQ Square	
SEWER MATERIAL	
AC Asbestos Cement	
BR Brick	
PE Polyethylene	
RP Reinforced Plastic Matrix	
CO Concrete	
CS Concrete Segment Baffled	
CB Concrete Segment Unbaffled	
CC Concrete Box Culverted	
PSC Plastic / Steel Composite	
GRC Glass Reinforced Plastic	
DI Ductile Iron	
PVC Polyvinyl Chloride	
CI Cast Iron	
SI Spun Iron	
ST Steel	
VC Vitrified Clay	
PP Polypropylene	
PF Fibre Fibre	
MAC Masonry, Courant	
MAR Masonry, Random	
U Unspecified	
Address or Site Reference: Heineken Manchester Denmark Road,	
OS sheet: SJ8395NE Scale: 1:1250 Date: 28/12/2018 Nodes: 173 Sheet: 4 of 4	Printed by: Property Searches



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VC	13.945622	
CO	74.330305	
BR	22.847332	
BR	22.847332	
PVC	31.442276	
CO	37.017332	1 in 112
BR	72.917776	1 in 228
VC	15.247998	
VC	15.247998	
BR	24.046306	
VC	5.945171	
CO	18.248229	1 in 304
CO	18.248229	1 in 304
VC	54.488553	1 in 196
CO	27.892855	1 in 174
VC	11.382332	
VC	11.382332	
VC	6.805898	
VC	6.805898	
VC	10.46231	
VC	31.821489	
VC	49.879133	1 in 200

VC	13.945622	
CO	74.330305	
BR	22.847332	
BR	22.847332	
PVC	31.442276	
CO	37.017332	1 in 112
BR	72.917776	1 in 228
VC	15.247998	
VC	15.247998	
BR	24.046306	
VC	5.945171	
CO	18.248229	1 in 304
CO	18.248229	1 in 304
VC	54.488553	1 in 196
CO	27.892855	1 in 174
VC	11.382332	
VC	11.382332	
VC	6.805898	
VC	6.805898	
VC	10.46231	
VC	31.821489	
VC	49.879133	1 in 200

Abandoned Foul Surface Water Combined

Public Sewer
Private Sewer
Section 104
Rising Main
Sludge Main
Overflow
Water Course
Highway Drain

red - combined blue - surface water
brown - foul purple - overflow

FO Foul
SW Surface Water
CO Combined
OV Overflow

CI	Circular	TR	Trapezoidal
EG	Egg	AR	Arch
OV	Oval	BA	Barrel
FT	Flat Top	HO	HorseShoe
RE	Rectangular	UN	Unspecified
SQ	Square		

- AC. Asthesic Cement
- BR Brick
- PE Polyethylene
- RP Reinforced Plastic Matrix
- OC Concrete
- CDB Concrete Segment Bolted
- CSU Concrete Segment Unbolted
- OC Concrete Box Cooled
- PSC Plastic / Steel Composite
- GR Glass Reinforced Plastic
- DI Ductile Iron
- PVC Polyvinyl Chloride
- CI Cast Iron
- SI Spin Iron
- ST Steel
- VC Vitrified Clay
- PP Polypropylene
- PF Pitch Fibre
- MAC Masonry, Coursed

Heineken Manchester Denmark Road,

Number: 1-1250 Date: 02/12/2010

Sheet: 3 of 4

**SEWER
RECORDS**

Anne Fox

From: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Sent: 01 December 2020 16:27
To: Anne Fox
Subject: FW: GENQ 4200034873 - 8540 DPV Building, Heineken Brewery, Manchester REF : DC/20/3421
Attachments: DC-20-3421-UU response.pdf; 8540-NDA-XX-XX-DR-A-9002 P3 Proposed 3D View.pdf; Existing Topographical Survey.pdf

Hi Anne,

As discussed, please see below response sent on 13th November.

Kind regards,

Tom



Thomas Bethell
Assistant Engineer
Developer Services & Metering
Customer Services
M: 07880 339 195
unitedutilities.com

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From: Wastewater Developer Services
Sent: 13 November 2020 15:31
To: 'AnneFox@norder.co.uk' <AnneFox@norder.co.uk>
Cc: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Subject: FW: GENQ 4200034873 - 8540 DPV Building, Heineken Brewery, Manchester REF : DC/20/3421

Good Afternoon Anne,

Thank you for detailing the drainage situation.

Considering the below, we would have no objections to this proposal for surface water. Please note that there will likely be a requirement to amend the trade effluent consent – this should be discussed with the water retailer.

With regards to our planning response, we have only recommended conditions to the Local Planning Authority (LPA). It is decided by the LPA what conditions are put on any decision notice, and I therefore recommend detailing the below to them so that they can confirm if this alleviates the need for a surface water condition.

Please see the following link may be of use: <https://www.unitedutilities.com/Business-services/trade-effluent/>

Kind regards,

Tom



Thomas Bethell
Assistant Engineer
Developer Services & Metering
Customer Services
M: 07880 339 195
unitedutilities.com

If you have received a great service today why not tell us?

Visit: unitedutilities.com/wow

From: Anne Fox [<mailto:AnneFox@norder.co.uk>]

Sent: 04 November 2020 17:09

To: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>

Subject: FAO Mr Thomas Bethell - 8540 DPV Building, Heineken Brewery, Manchester REF : DC/20/3421

FAO Mr Thomas Bethell

Dear Thomas

Further to our recent telephone conversation, please find attached the United Utilities letter response to the planning application for the proposed Dual Purpose Vessel (DPV) building at the Heineken Brewery site in Manchester. United Utilities reference DC/20/3421.

United Utilities have proposed the standard conditions as they have not received any information explaining the drainage proposals. I am currently in the process of producing a drainage design report but would summarise the main items as follows.

The proposed development consists of a new DPV block, a single storey building with an 8 no. vertical Dual Purpose Vessel (DPV) installation. The proposal also includes an adjacent open access vertical escape ladder and overhead pipe bridge/walkway to link to the adjacent vessels, I attach a 3D view showing the proposals.

The DPV building accommodates an area of approximately 485m², which is currently occupied by a mix of tarmac and concrete service yard and road. The development area is flat lying having a ground level between approximately 32.90m AOD and 32.75m AOD. I attach an existing topographical survey showing the development area.

The surface water run off from the DPV building is required to be discharged to the effluent treatment plant (ETP) and treated prior to discharge from the site. This is due to the potential contamination of surface water run off from the vessels, as a result of fobbing due to overactive yeast or glycol discharge from cooling jackets.

The ETP discharges the treated effluent from the site into the United Utilities combined system in Denmark Road, under the Trade Effluent licence reference : 694TLS26S069. This licence imposes a restricted discharge rate not exceeding 60 l/s with a maximum amount of trade effluent in a 24 hour day not exceeding 5,130m³. The average daily discharge through August was 2,143m³ and September 2020 was 1,888m³, showing that there is sufficient spare capacity to accommodate the surface water discharge from the DPV building.

Please could you provide me with your initial thoughts regarding the above proposal so that I can incorporate any requirements into the drainage design report. Should you wish to discuss the above or require any clarifications, please do not hesitate to contact me on 07837 888 475.

Kind Regards

Anne Fox
Senior Infrastructure Engineer
07837 888 475



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Web Page : www.norder.co.uk



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Appendix F – Micro Drainage Results Print Outs

Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	DPV Building, Heineken, Manchester Existing Brownfield Runoff 1 in 1 Year Storm	
Date 01/12/2020 19:50 File BROWNFIELD RUN OFF DPV ...	Designed by AF Checked by AF	
Innovyze	Network 2019.1	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	1	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.361	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	100	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	10.000	0.100	100.0	0.048	4.00	0.0	0.600	o	150	Pipe/Conduit	
1.001	10.000	0.100	100.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.44	4.17	8.500	0.048	0.0	0.0	0.0	1.00	17.8	6.6
1.001	49.68	4.33	8.400	0.048	0.0	0.0	0.0	1.00	17.8	6.6

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Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	DPV Building, Heineken, Manchester Existing Brownfield Runoff 1 in 100 Year Storm	
Date 01/12/2020 19:49	Designed by AF	
File BROWNFIELD RUN OFF DPV ...	Checked by AF	
Innovyze	Network 2019.1	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.361	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	100	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

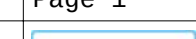
Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	10.000	0.100	100.0	0.048	4.00	0.0	0.600	o	150	Pipe/Conduit	
1.001	10.000	0.100	100.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	100.00	4.17	8.500	0.048	0.0	0.0	0.0	1.00	17.8	13.0
1.001	100.00	4.33	8.400	0.048	0.0	0.0	0.0	1.00	17.8	13.0

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Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester DPV Building Network Details	
Date 01/12/2020 20:44	Designed by AF	
File DPV EFFLUENT DRAINAGE N...	Checked by AF	
Innovyze	Network 2019.1	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	18.000	Add Flow / Climate Change (%)	0
Ratio R	0.361	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	100	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.043	4-8	0.005

Total Area Contributing (ha) = 0.048

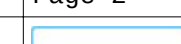
Total Pipe Volume (m³) = 3.189

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	13.400	0.133	100.8	0.048	4.00	0.0	0.600	o	300	Pipe/Conduit	
1.001	5.640	0.094	60.0	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.002	2.173	0.036	60.0	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.003	12.141	0.202	60.1	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.004	8.801	0.146	60.3	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.005	2.961	0.049	60.4	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	100.00	4.14	31.000	0.048	0.0	0.0	0.0	1.57	110.7	13.0
1.001	100.00	4.19	30.867	0.048	0.0	0.0	0.0	2.03	143.7	13.0
1.002	100.00	4.21	30.773	0.048	0.0	0.0	0.0	2.03	143.7	13.0
1.003	100.00	4.31	30.737	0.048	0.0	0.0	0.0	2.03	143.6	13.0
1.004	100.00	4.38	30.535	0.048	0.0	0.0	0.0	2.03	143.4	13.0
1.005	100.00	4.40	30.389	0.048	0.0	0.0	0.0	2.03	143.2	13.0

Norder Design Assoc		Page 2
Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester DPV Building Network Details	
Date 01/12/2020 20:44	Designed by AF	
File DPV EFFLUENT DRAINAGE N...	Checked by AF	
Innovyze	Network 2019.1	

Online Controls for Storm

Hydro-Brake® Optimum Manhole: 3, DS/PN: 1.002, Volume (m³): 2.7

Unit Reference	MD-SHE-0092-5000-2000-5000
Design Head (m)	2.000
Design Flow (l/s)	5.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	92
Invert Level (m)	30.773
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	2.000	5.0
Flush-Flo™	0.398	4.1
Kick-Flo®	0.816	3.3
Mean Flow over Head Range	-	3.9

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	2.9	1.200	3.9	3.000	6.0	7.000	9.0
0.200	3.8	1.400	4.2	3.500	6.5	7.500	9.3
0.300	4.1	1.600	4.5	4.000	6.9	8.000	9.6
0.400	4.1	1.800	4.8	4.500	7.3	8.500	9.9
0.500	4.1	2.000	5.0	5.000	7.7	9.000	10.2
0.600	4.0	2.200	5.2	5.500	8.0	9.500	10.4
0.800	3.4	2.400	5.4	6.000	8.4		
1.000	3.6	2.600	5.6	6.500	8.7		

Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester DPV Building Simulation 1 in 1 Year	
Date 01/12/2020 20:45	Designed by AF	
File DPV EFFLUENT DRAINAGE N...	Checked by AF	
Innovyze	Network 2019.1	

Summary of Critical Results by Maximum Level (Rank 1) for Storm

Number of Input Hydrographs	0	Number of Storage Structures	0
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Margin for Flood Risk Warning (mm)	300.0	DVD Status	OFF
Analysis Timestep	Fine	Inertia Status	OFF
DTS Status	ON		

PN	US/MH		Storm	Return Period	Climate Change	First (X) Surchage	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
	Name									
1.000	1	15	Summer	1	+0%					31.051
1.001	2	15	Winter	1	+0%					31.006
1.002	3	15	Winter	1	+0%					31.004
1.003	4	15	Winter	1	+0%					30.773
1.004	5	15	Winter	1	+0%					30.573
1.005	6	15	Winter	1	+0%					30.438

PN	US/MH Name	Surcharged	Flooded	Flow / Cap.	Overflow (l/s)	Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)			Flow (l/s)		
1.000	1	-0.249	0.000	0.07		6.2	OK	
1.001	2	-0.161	0.000	0.06		4.9	OK	
1.002	3	-0.069	0.000	0.06		3.9	OK	
1.003	4	-0.264	0.000	0.03		3.9	OK	
1.004	5	-0.261	0.000	0.04		3.9	OK	
1.005	6	-0.251	0.000	0.06		3.9	OK	

Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester DPV Building Simulation 1 in 30 Year	
Date 01/12/2020 20:47	Designed by AF	
File DPV EFFLUENT DRAINAGE N...	Checked by AF	
Innovyze	Network 2019.1	

Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
 Hot Start Level (mm) 0 Inlet Coefficient 0.800
 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000
 Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0 Number of Storage Structures 0
 Number of Online Controls 1 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

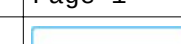
Rainfall Model FSR Ratio R 0.363
 Region England and Wales Cv (Summer) 0.750
 M5-60 (mm) 18.000 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0 DVD Status OFF
 Analysis Timestep Fine Inertia Status OFF
 DTS Status ON

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600,
 720, 960, 1440, 2160, 2880, 4320, 5760,
 7200, 8640, 10080
 Return Period(s) (years) 30
 Climate Change (%) 0

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
1.000	1	30 Winter	30	+0%	30/15 Summer				31.567
1.001	2	30 Winter	30	+0%	30/15 Summer				31.564
1.002	3	30 Winter	30	+0%	30/15 Summer				31.562
1.003	4	15 Winter	30	+0%					30.774
1.004	5	60 Winter	30	+0%					30.575
1.005	6	30 Winter	30	+0%					30.439

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m ³)	Flow / Cap.	Overflow (l/s)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	1	0.267	0.000	0.11		9.7	SURCHARGED	
1.001	2	0.397	0.000	0.08		5.9	SURCHARGED	
1.002	3	0.489	0.000	0.07		4.1	SURCHARGED	
1.003	4	-0.263	0.000	0.04		4.1	OK	
1.004	5	-0.260	0.000	0.04		4.1	OK	
1.005	6	-0.249	0.000	0.07		4.1	OK	

Norder Design Assoc		Page 1
Beech Lawn Green Lane, Belper DE56 1BY	Heineken, Manchester DPV Building Simulation 1 in 100 Year + CC	
Date 01/12/2020 20:48	Designed by AF	
File DPV EFFLUENT DRAINAGE N...	Checked by AF	
Innovyze	Network 2019.1	

Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
 Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
 Hot Start Level (mm) 0 Inlet Coefficient 0.800
 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (l/per/day) 0.000
 Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0 Number of Storage Structures 0
 Number of Online Controls 1 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR Ratio R 0.363
 Region England and Wales Cv (Summer) 0.750
 M5-60 (mm) 18.000 Cv (Winter) 0.840

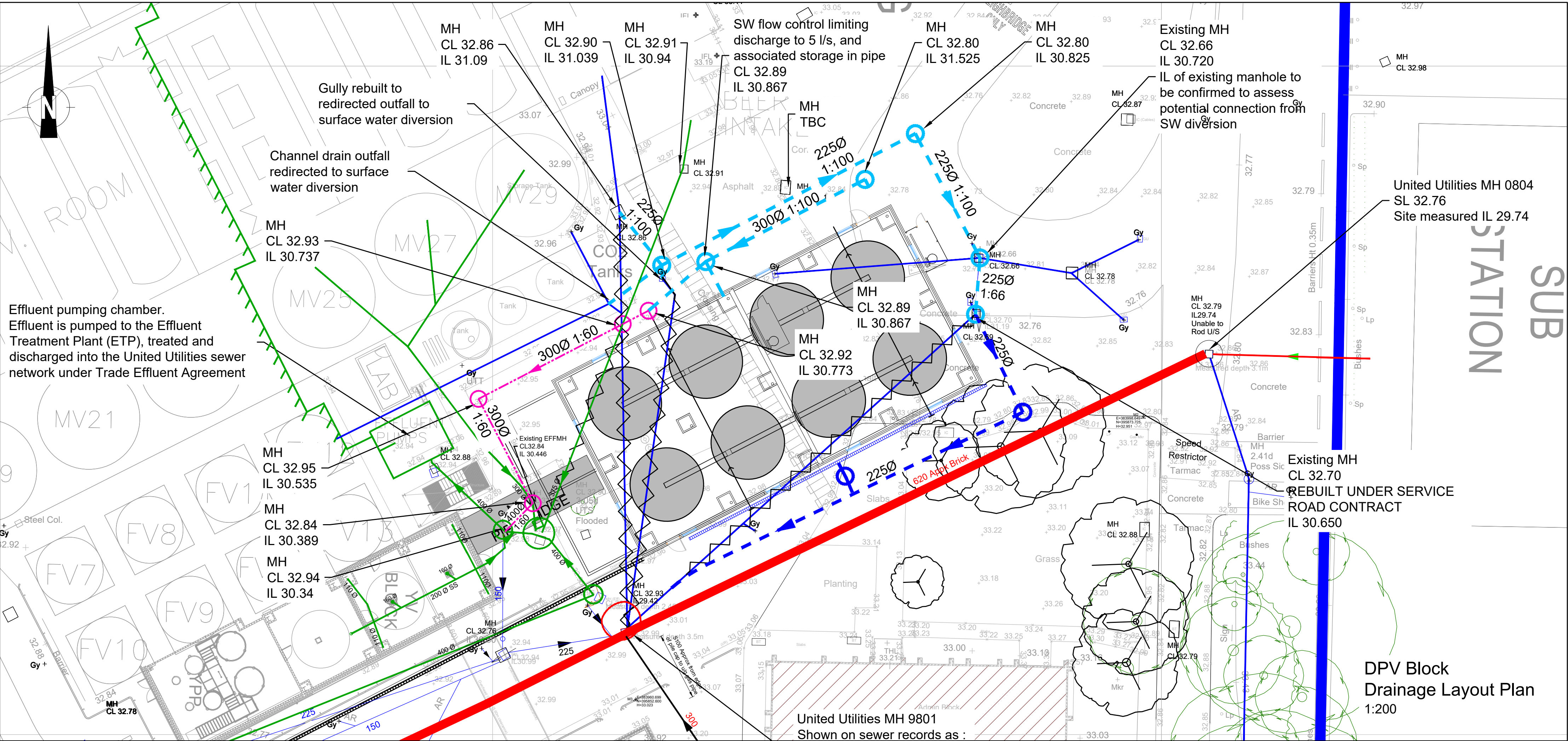
Margin for Flood Risk Warning (mm) 300.0 DVD Status OFF
 Analysis Timestep Fine Inertia Status OFF
 DTS Status ON

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600,
 720, 960, 1440, 2160, 2880, 4320, 5760,
 7200, 8640, 10080
 Return Period(s) (years) 100
 Climate Change (%) 30

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
1.000	1	30 Winter	100	+30%	100/15	Summer			32.717
1.001	2	30 Winter	100	+30%	100/15	Summer			32.713
1.002	3	30 Winter	100	+30%	100/15	Summer			32.711
1.003	4	30 Winter	100	+30%					30.777
1.004	5	30 Winter	100	+30%					30.578
1.005	6	30 Winter	100	+30%					30.445

PN	US/MH Name	Surcharged		Flooded		Pipe		Status	Level Exceeded
		Depth (m)	Volume (m ³)	Flow / Cap.	Overflow (l/s)	Flow (l/s)			
1.000	1	1.417	0.000	0.14		12.5	FLOOD	RISK	
1.001	2	1.546	0.000	0.11		8.1	FLOOD	RISK	
1.002	3	1.638	0.000	0.08		4.9	FLOOD	RISK	
1.003	4	-0.260	0.000	0.04		4.9		OK	
1.004	5	-0.257	0.000	0.05		4.9		OK	
1.005	6	-0.244	0.000	0.08		4.9		OK	

Appendix G – Drainage Layout Drawing



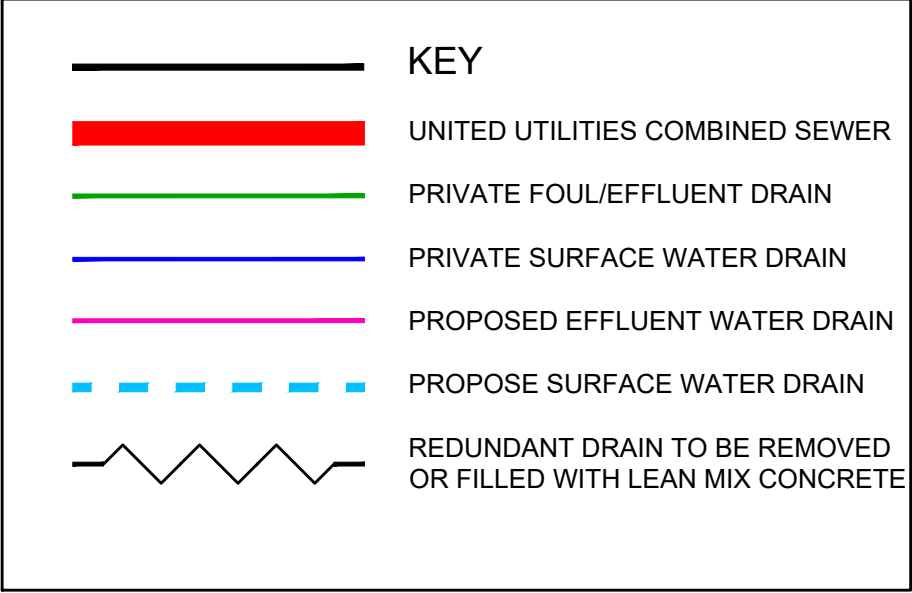
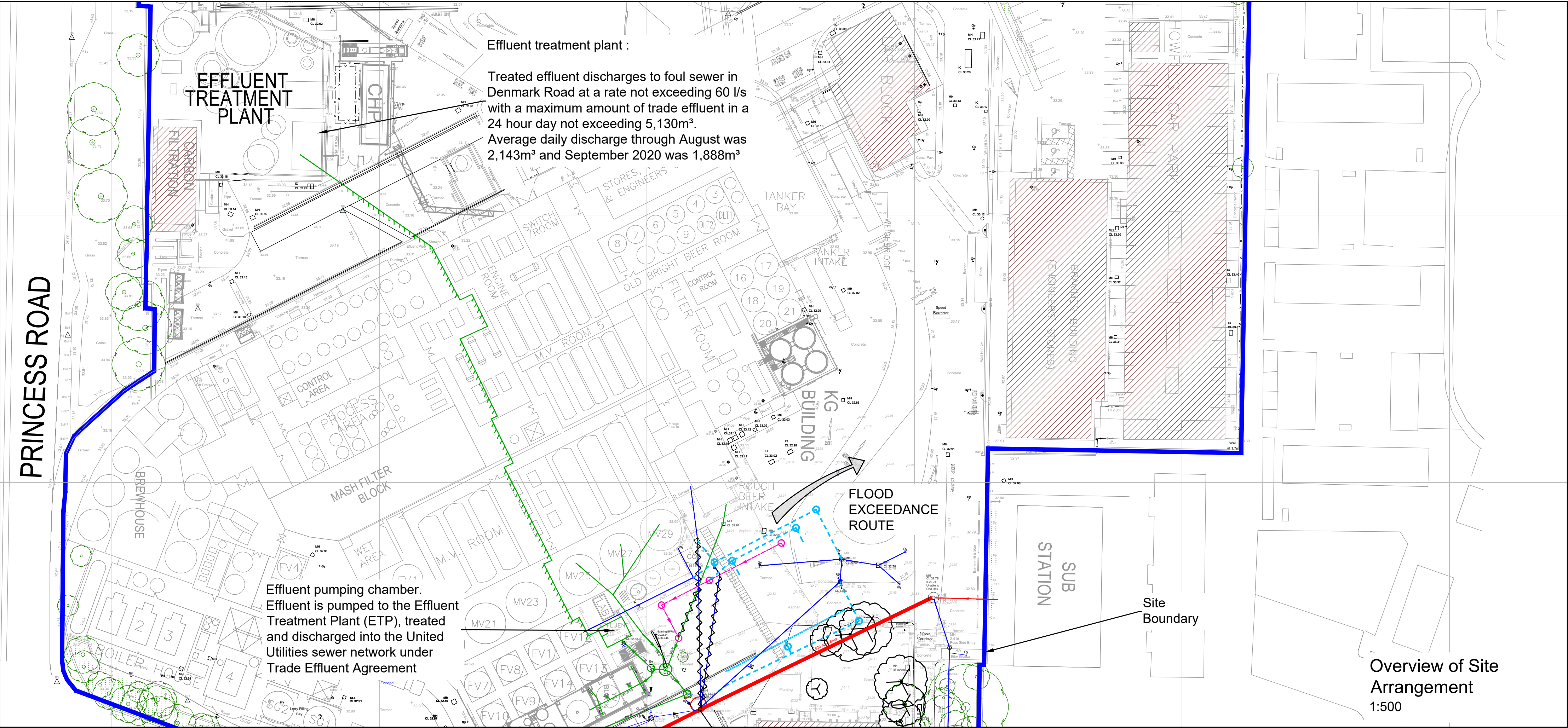
Construction Notes

- The Contractor shall verify all dimensions and existing conditions on site prior to commencing the works. Any discrepancies noted on site are to be reported to the Engineer immediately.
- For details of ground conditions refer to Ground Investigation Report. The Engineer shall be immediately notified of any variations to the reported ground conditions. The Contractor is to supply a full method statement for the traffic management and control of pedestrian routes.
- The Contractor shall undertake a condition survey of the adjacent structures, footpaths and roadways. Any damage caused by the contractor to be made good to the Engineers requirements.
- The Contractor shall locate, identify and protect all existing services prior to the commencement of the Works. Protective measures shall be undertaken in accordance with the utility providers requirements.
- The contractor shall work within any Planning Authority restriction with regard to working hours, noise regulations and any other limitations that may apply.
- All works are to be undertaken in accordance with the requirements of the Specification for Highway Works.
- Prior to importation of fill materials, a representative sample, together with a grading certificate, shall be delivered to site for inspection by the Engineer.
- Following acceptance, imported material shall be laid and compacted, according to its classification, in accordance with the requirements of the Specification for Highway Works.
- All materials within 450mm of the finished road levels to be non-frost susceptible.
- Excavation to depths greater than those shown on the drawing(s) shall not be undertaken without prior approval of the Engineer.
- 2 no CBR tests to be undertaken on formation and testing of construction materials is to be undertaken in accordance with the Engineers requirements. Testing to be undertaken by a NAMAS approved geotechnical laboratory.
- All obstructions, old foundations etc shall be removed and backfilled with 6F2 granular material.
- All formations shall be proof rolled and any subsequent soft spots excavated and backfilled with 6F2 granular material.
- Contractor to allow for 1 no. day of white lining being superimposed thermoplastic material conforming to BS 3262, and removal of adjacent white lining to allow tie in of works.
- Existing services and drainage covers located within the new service road slab area to be raised or lowered to suit the new slab level and reinforcement arrangement to be as per standard detail.
- The Contractor is to undertake a CCTV survey of the United Utilities combined sewer which runs through the works, both before the works commence and following completion of the works.

Print Date and Time: 18/12/2017 14:08:53

General Notes:

- The drawings, design and all information contained therein are the sole copyright of Norder Design Associates (NDA) and reproduction in any form is forbidden unless permission is obtained in writing.
- This drawing is to be read in conjunction with all other relevant Architects, Engineers and specialists drawings, details and specifications.
- Conflicting information shown on the NDA drawings or discrepancies between the information given by NDA and that provided by others must be referred to NDA before work proceeds.
- Dimensions must not be scaled from this drawing.
- Dimensions marked 'dimension to be site checked' are subject to confirmation by site measurement before construction proceeds.
- Subcontractor surveys, such as for the purposes of steelwork installation, take precedence over 'site dimensions' indicated on this drawing. All surveys should ensure an accurate fit of components.
- Temporary works associated with the construction of the project shall be the responsibility of the Principal Contractor.
- Where any temporary works or scaffolding are required the Principal Contractor shall appoint a competent and suitably experienced Temporary Works Coordinator (TWC). Refer to BS 5975 for guidance on temporary works and the role of a TWC.
- Calculations and drawings of all Contractor designed structural items associated with NDA's design responsibilities shall be submitted to the Engineer for comment at least 10 days prior to manufacture. Any comments made by NDA do not absolve the Contractor of his design responsibilities.



P1_01.12.20	Preliminary Issue	AF
REV DATE	AMENDMENT	BY
CLIENT		



PROJECT

Manchester Brewery
DPV Building

DRAWING TITLE

Proposed Drainage Layout

NORDER
Consulting Engineers and Architects

Beech Lawn, Green Lane, Belper, Derby DE56 1BY
Tel: 01773 824414 Fax: 01773 823305
e-mail: enquiries@norder.co.uk

SCALE	AS SHOWN	SHEET SIZE A1	
DATE	Dec 2020	DRAWN	AF
		CHECKED	RSP
DRAWING NO.	8540-NDA-XX-XX-DR-C-1012	REV	P1