

THE CHRISTIE, TIERED CAR PARK, WITHINGTON, MANCHESTER



09/02/2021

DRAINAGE STRATEGY REPORT

The Alan Johnston Partnership LLP

Ref: CCP-AJP-ZZ-XX-RP-C-3010



Prepared By

Document Owner(s)	Project/Organization Role
John Murphy	Partner
John Speers	Senior Civil Engineer

Project Closure Report Version Control

Version	Date	Author	Change Description
P01	01/04/2020	J.Speers	First Issue
P02	09/02/2021	J.Speers	Updated as per comments
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Record of Approval

Originator	Approved	Date
J.Speers	J.Murphy	01/04/2020
J.Speers	J.Murphy	09/02/2021
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1.0 INTRODUCTION

This drainage strategy report has been prepared on behalf of The Christie NHS Foundation Trust in connection with the discharge of the drainage related planning conditions for planning application reference 117847/FO/2017. The planning application is for the construction of a new tiered car park. The proposed site is located on the existing Christie staff car parking area, to the east of Frith Road, Withington, Manchester.

The purpose of this report is to present the surface water and foul water outline drainage strategies for the site which have been developed by the Alan Johnston Partnership. To demonstrate how the specific drainage requirements are to be met, the report should be read in conjunction with the relevant drawings and supporting information, included within the appendices.

The drainage strategies outlined within this report, have been prepared in accordance with BS EN 752:2017 (Drain and sewer systems outside buildings), Sewers for Adoption, Building Regulations Approved Document H (Drainage and waste disposal), the National Planning Policy Framework and Planning Practice Guidance (Department for Communities and Local Government).

The site is located within Flood Zone 1, with a total site area under 1.0ha. Therefore, in accordance with DEFRA guidance, a Flood Risk Assessment (FRA) is not required. A review of the Manchester City, Salford City and Trafford Councils Level 2 Hybrid Strategic Flood Risk Assessment (SFRA) identifies that the site is located within the Conurbation Core Critical Drainage Area (CDA). However, based on guidance within the Manchester City, Salford City and Trafford Councils Level 2 Hybrid SFRA User Guide, only sites with an area greater than 0.5ha within a Critical Drainage Area require a Flood Risk Assessment. As the total site area is less than 0.5ha it has been deemed that an FRA is not required to support the planning application for this development.

2.0 SITE DESCRIPTION

The proposed development site is located to the on the existing Christie staff car parking area, to the east of Frith Road, Withington, Manchester at National Grid reference SJ 85174 92419. The location of the existing site is illustrated in Appendix A and the extent of the area to which the application relates is also shown on the aerial photo and existing site plan, both enclosed in Appendix B.

The proposed development site covers a total area of 1.565ha. As can be seen from the aerial photo, enclosed in Appendix B, this area currently consists of the existing Christie staff car parking area, the existing Christie student accommodation buildings and car park area and the existing access road. Therefore, the site is classified as a brownfield site. The site is bounded to the east and south by existing residential developments, to the north the existing highway Cotton Lane and to the west by existing Christie developments.

The main area of the site is fairly flat with levels sloping slightly from north-east to south-west, from approximately 37.20mAOD in the north-east to approximately 36.00mAOD in the south-west. The existing access road to the west of the main site slopes slightly from east to west from approximately 36.70mAOD in the east to approximately 35.60mAOD. The topographic survey is attached in Appendix C.

3.0 PROPOSED DEVELOPMENT

The proposed development consists of a new eight level decked car park structure, with associated landscaping and access road areas. To enable a viable scheme, the proposed site levels are constrained by the existing residential developments to the east and south, the existing highway to the north and the existing Christie development to the west. Therefore, site levels are likely to remain similar to the existing. A proposed site plan is attached in Appendix D.

4.0 DEVELOPMENT AND DRAINAGE STRATEGY

4.1 Effect of Proposed Development

As previously outlined in Section 2.0, the existing site covers a total area of 1.565ha and is a brownfield site. A comparison between the existing and proposed impermeable areas is included in Appendix E and as can be seen it indicates that the introduction of the new grasscrete parking areas and additional landscaped areas, will lead to a decrease in the overall impermeable areas.

As can be seen the majority of the 1.565ha existing site is either; to drain as per the existing scenario, is currently and/or is to remain as permeable or is to drain via porous paving (grasscrete) and is therefore classed as permeable. As such, only 0.763ha of the existing site and 0.720ha of the proposed site are classified as impermeable and are to be positively drained.

Total Development Site Area (ha)	Existing Impermeable Area (ha)	Proposed Impermeable Area (ha)
1.565	0.763	0.720

Therefore, the changes to the existing site will decrease the volume of impermeable areas and as such, the proposed development will lead to a decrease in the;

- Volume of surface water ponding on the site
- Volume of surface water runoff leaving the site or discharging into surrounding areas
- Peak discharge rate from the site.

Despite this, site-wide drainage systems are required to drain the surface water flows arising from the proposed development. Where possible, any existing drainage networks should be utilised. Appropriate design and construction of these systems as set out in Section 4.3 should ensure that there is no increase in offsite flood risk that would otherwise impact downstream areas.

4.2 Existing Drainage Systems

As can be seen from the United Utilities map, attached in Appendix F, there is no existing public sewers located within the site boundary. The only nearby public sewers are located within the surrounding highways, with a 375mm diameter public combined water sewer located on Cotton Lane to the north of the site and a 300mm diameter public combined water sewer located on Frith Road to the west of the site

However, as can be seen from the utility survey and as built drainage drawing for the adjacent site, both of which are also attached in Appendix F, there is existing private drainage across the site which drains the existing asphalt car park area and access roads. This is also confirmed within the Reid Jones Partnership Flood Risk Assessment (FRA), October 2017 which states that, *“some of the surface water from the car park drained to a combined sewer to the west of the site, with the remainder discharging to the surface water drains in the access road”* and that the remainder of the drainage *“was re-routed to connect to the existing drains in the access road at manhole S1.”*

As such, the existing car park area all drains to the existing private drainage along the existing access road to the west of the existing asphalt car park.

4.3 Proposed Drainage Strategy

As outlined above in section 4.1, site-wide drainage systems are required to drain the surface water flows arising from the proposed development. The proposed drainage systems must ensure that there is no increase in offsite flood risk that would otherwise impact downstream areas.

4.3.1 Surface Water Drainage

The Building Regulations - Approved Document H (2002) details a hierarchy of potential methods for disposing of surface water as shown below in order of preference:

- Discharge via infiltration
- Discharge to watercourse
- Discharge to sewer

Considering the hierarchy above, the surface water network for the proposed site should infiltrate where possible. The WYG Environment Geo-Environmental Factual and Interpretative Report, January 2020 indicates that the general strata consists of made ground to a maximum recorded depth of 1.20mbgl, underlain by glacial deposits comprising of stiff slightly sandy/gravelly clay to a maximum depth of 2.40mbgl. This is then underlain by sandstone bedrock, to a maximum recorded depth of 16.60mbgl.

The Reid Jones Partnership FRA, October 2017 indicates that due to the underlying strata infiltration drainage may be suitable across the site. However, Building Regulations Part-H states that no soakaways can be located within 5m of a building or road and therefore, assessing the proposed development site there are limited areas to locate a soakaway or other infiltration techniques 5m from a building or road.

The WYG Environment Geo-Environmental Factual and Interpretative Report, January 2020 also indicates a relatively shallow groundwater table across the site, at levels between 1.7mbgl – 3.4mbgl. As outlined within CIRIA Report C753, The SuDS Manual, a minimum distance of 1m should be maintained between the base of a soakaway and the maximum groundwater level and therefore the relatively shallow groundwater levels also restrict the use of soakaways.

Therefore, as a result of Building Regulations Part-H and groundwater restrictions, only infiltration drainage via porous surfaces, across the proposed car park areas is considered to be a viable option for the discharge of surface water, as this would, be 1m above the groundwater. However, this should be confirmed via BRE365 complaint soakaway tests in the areas of the proposed porous paving.

As the possibility of infiltration has been partially ruled out by the Building Regulations and groundwater restrictions, the intention for the development would be to discharge to watercourse. The nearest watercourse is Shaw Brook which is located approximately 385m north of the site. Due to the distance across various plots of third-party land and properties, discharging to watercourse has been deemed impractical.

Therefore, the only practical solution for discharge of the surface water is discharge to public sewer. As such, it is proposed that the areas of the site where infiltration is not suitable will drain to public sewer via a connection to the existing private drainage network.

While every effort to utilise any suitable existing systems should be made, the drainage systems should be designed to suit the proposed site layout and topography which aims to provide an efficient design. In line with Sewers for Adoption (7th Edition), the requirements for the design of a new surface water drainage systems are as follows:

- Below ground piped drainage to be sized to accommodate the 1 in 2 year (50% AEP) design storm without surcharge.
- System to be designed not to flood any part of the site in a 1 in 30 year (3% AEP) design storm.
- For events in exceedance of the 1 in 30 year design storm and up to and including the 1 in 100 year event, site drainage and topography should be designed where practicable to route surface water run-off away from buildings to safe above-ground storage areas on site, thereby preventing this run-off from leaving the site and increasing flood risk elsewhere.

For each design case described above, the design storm is the critical storm duration for the site conditions. In the case of the 1 in 100 year design storm, a 40% increase in the peak rainfall intensity is applied to allow for the estimated worst case impacts of climate change. This is in accordance with new advice, issued by the Environment Agency, which updates previous climate change allowances outlined in the National Planning Policy Framework.

Suitable systems of below ground drainage will be required to contain as a minimum requirement, the 1 in 30 year event. Additionally, any surface water run-off from events that exceed the design capacity of the new drainage system, up to and including the 1 in 100 year (+40%) event, will be retained on-site in safe storage areas, away from buildings.

As the area consists mainly car park areas, in line with the requirements of Pollution Prevention Guidelines 3 (PPG3), measures to prevent oil and other contaminants being passed forward to the existing sewers are required. Therefore, an oil separator has been included in the proposed drainage design.

In line with common practice, it is proposed the surface water discharge from the proposed development should mimic that from the existing site.

It was envisaged that a minimum 50% betterment on the existing surface water brownfield runoff rate was to be applied, in line with Section 3.4.1 of the Manchester City, Salford City and Trafford Councils Level 2 Hybrid SFRA User Guide.

Therefore, an assessment of the existing surface water run-off from the brownfield site has been completed using a simple three pipe network modelled in Micro Drainage. For the existing 0.763ha developed area of the site, using an existing impermeable area of 0.763ha, an existing 2-Year brownfield discharge rate of 107.2 L/s was generated. The brownfield runoff assessment is attached in Appendix G.

Therefore, applying a 50% reduction to offer improvement, the maximum surface water discharge rate for the site as a whole is **53.6 L/s**.

The planning decision notice, informative for condition no.5 section outlines that at least a 50% reduction in runoff rate compared to the existing rates is required. Therefore, it is deemed that the 50% betterment on the existing surface water brownfield runoff rate is to be applied, as this also complies with Section 3.4.1 of the Manchester City, Salford City and Trafford Councils Level 2 Hybrid SFRA User Guide.

As such, the surface water drainage system is to be designed to restrict the discharge to 53.6 L/s, for all rainfall events up to and including a 1 in 100 year return period plus 40% climate change design storm.

Using a total post development impermeable area of 0.720ha at the restricted discharge rate of 53.6 L/s, for the 1 in 100 year + 40% return period design storm, a storage volume of **178.5m³** is required. The attenuation storage estimate is attached in Appendix H.

As seen in the Drainage Strategy Drawing, attached in Appendix I, it is anticipated that the surface water runoff will be collected and conveyed to the north-west of the proposed tiered car park building. Here it will be restricted using a vortex flow control device to the required 53.6 L/s discharge rate, prior to discharging into the existing private drainage manhole S1.

As the surface water discharge is to be restricted, the surface water runoff will surcharge the proposed drainage network. Therefore, the attenuation storage will be provided in the form of a below ground cellular attenuation tank, with a vortex flow control device used to restrict the surface water runoff to the required rate of 53.6 L/s.

It is anticipated that due to lack of areas of sufficient size to safely store flooding at ground level, the proposed surface water drainage network and cellular attenuation tanks will be suitably sized to contain the surcharged runoff for all rainfall events up to and including the 1 in 100 year + 40% climate change rainfall event.

The surface water calculations which represent the surface water drainage indicated on the Drainage Strategy Drawing, attached in Appendix I, are also attached within Appendix I. The calculations show that there is no flooding for all rainfall events up to and including the 1 in 100 year + 40% climate change rainfall event.

As can be seen from the calculations, attached in Appendix I, a 120m³ cellular attenuation tank and the proposed surface water network itself provide sufficient capacity to store the surcharged runoff for all rainfall events up to and including the 1 in 100 year + 40% climate change rainfall event, without any flooding.

As outlined below, all other SuDS techniques have been considered for this development;

SuDS Technique	Suitable for This Development?	Reasoning
Green Roof	✗	Not suitable due to lack of available roof area.
Rainwater Harvesting	✗	Impractical due to no demand for potable water.
Basins & Ponds	✗	Unsuitable due to lack of available space.
Filter Strips & Swales	✗	Unsuitable due to lack of available space and Building Regulations and groundwater restrictions.
Infiltration Techniques	✓ & ✗	Only partially suitable due to Building Regulations and groundwater restrictions. Incorporated where possible.
Permeable Surfaces	✓ & ✗	Only partially suitable due to Building Regulations and groundwater restrictions. Incorporated where possible.
Tanked Systems	✓	Suitable and incorporated.

Further investigations have indicated that the existing manhole S1 has an invert level of 34.17mAOD therefore a package pump station is required to drain the proposed drainage within the basement area of the car park into the surrounding drainage.

The surface water drainage strategy and discharge rate should be confirmed via more detailed discussions with United Utilities and the Lead Local Flood Authority prior to the commencement of any works.

The exceedance rainfall events have also been considered throughout the design of the surface water drainage system. As can be seen from the exceedance event calculations, attached in Appendix J, there is no flooding within the 1 in 200 year rainfall event but there is approximately 24m³ of flooding within the 1 in 1000 year rainfall event.

It can be seen that the flooding will occur from the gullies/manholes in the car park areas to the south and east of the proposed tiered car park. Therefore, as indicated within the exceedance flow paths drawing, attached in Appendix J, the external ground levels around the proposed building shall fall away from the proposed building with any new levels being designed to ensure any exceedance flood routes, for events in excess of the 100 Year + 40% climate change event, exit towards the car park areas and access roads, for use in exceptional circumstances.

4.3.2 Foul Water Drainage

No foul water drainage is required across the proposed development, as it is a car park structure with no requirement for foul water drainage.

5.0 FUTURE MANAGEMENT & MAINTENANCE

The proposed drainage solution uses SUDS techniques in accordance with the CIRIA SUDS Manual C753. The surface water run-off from the proposed development is restricted using a vortex flow control device and the attenuated run-off is stored using a cellular attenuation tank. Grasscrete porous paving has also been incorporated, where possible.

It is envisaged that the following components have been or will be included within the drainage strategy for the proposed development:

- Inspection chambers, manholes and catchpits.
- Pipes.
- Drainage channels and gullies.
- Vortex flow control device.
- Cellular attenuation tank.
- Porous paving.

A suitable maintenance strategy should be adopted to ensure the drainage network is cleaned regularly and the routine maintenance and cleansing regime should be documented. It is assumed that the proposed drainage will remain private and therefore maintenance of the surface water drainage system will be the responsibility of The Christie NHS Foundation Trust or an on-site facilities management team.

As such, the maintenance of the specified SuDS should be included within the developments Operation and Maintenance Manual and should be detailed in accordance with the CIRIA SuDS Manual C753, as per the operation and maintenance guidance outlined below.

5.1 General Maintenance & Inspection Requirements

- No work shall be carried out on the drainage system without permission from a nominated person, who has access to information/a working knowledge of the system.
- Maintenance/inspection work shall be carried out in a safe/planned manner.
- All work is to be carried out by competent persons suitably trained and equipped in accordance with current statutory safe working policies.
- Entry into confined spaces shall be kept to a minimum and be restricted to suitably qualified/equipped persons working in accordance with current statutory safe working policies.
- Drainage systems shall be inspected on a regular basis or should any problems be suspected. Any debris/defects discovered shall be recorded and a programme of cleaning/ repair initiated. Urgent repairs/cleaning shall be actioned as soon as practicable.
- It is recommended that the drainage system is inspected as a minimum twice a year, with the system also being inspected after any major storm event.
- Clearing of the drainage system can be achieved by a number of methods depending on the nature of the work;
 - Rodding – Manual/Mechanical with flexible rods.
 - Jetting – High pressure water jetting.
 - Plunging.

5.2 Inspection Chambers, Manholes and Catchpits

The appropriate health and safety equipment must be used when accessing manholes/catchpits. Confined space certificates must be held by any personnel entering a manhole and the appropriate permits should be obtained from the Maintenance Manager prior to any access. The following operations should be carried out annually.

- Covers of inspection chambers and manholes shall be removed and the sides, benchings and channels cleared.
- Accumulated deposits of silt in inspection chambers, catchpits and manholes shall be removed. Any traps shall then be plunged and thoroughly flushed out with clean water. This should be completed in Autumn, after leaf fall.
- Main and branch drains shall be cleared as required and afterwards be flushed with clean water. Any obstructions found shall be removed and not flushed down the system.
- Covers of inspection chambers, manholes and catchpits shall be replaced, bedded in suitable grease or other sealing material as required and bolted/locked down as appropriate. Missing bolts and broken items shall be replaced in accordance with the manufacturer's details.

5.3 Pipes

Regular inspection and maintenance is important to identify areas which may have become obstructed/clogged and may not be draining correctly, as failure to do so would expose the development to a greater level of flood risk.

Pipes are proprietary products and therefore the materials used can vary across the site. As such, where used the manufacturer's recommendations should be followed. Access for maintenance of the pipes is provided through inspection chambers and manholes. The below table sets out the maintenance schedule for the pipe components of the proposed drainage systems.

Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Inspect for evidence of poor operation via water levels. If required take remedial action.	Monthly for 3 months, then six-monthly intervals. Also, 48 hours after large storms.
	Check and remove large debris and/or vegetation growth near pipe runs.	Monthly or as required.
Remedial Actions	Rod through poorly performing runs as initial remediation.	As required.
	If continued poor performance jet and CCTV survey poorly performing runs.	As required.
	Seek advice as to remediation techniques suitable for the type of performance issue.	As required, if above actions do not improve performance.
Monitoring	Initial Inspection should be provided as post construction CCTV survey.	N/A
	Inspect/check all inlets, outlets and overflows to ensure they are in good condition and operating as designed.	Monthly for 3 months, then six-monthly intervals. Also, 48 hours after large storms.

5.4 Drainage Channels and Gullies

Channels and gullies should be inspected and cleaned in accordance with the manufacturer's details. Channel units can be cleaned through the use of a high-pressure hose; this can be fed into the channel system through access units strategically placed along the channel run. The throat section of channel units should be kept clear at all times to ensure uninterrupted flow of surface water into the drainage channel and any debris within the throat should be removed.

Locking bolts should be replaced and sufficiently tightened, taking care that the bolt heads do not stand above the top surface of the cover or grate. If covers are allowed to move within their frame, this may cause damage to the frame or seating. The below table sets out the maintenance schedule for the drainage channel and gullies components of the proposed drainage systems.

Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Litter and debris removal.	Monthly or as required.
	Check and remove large debris and/or vegetation growth near pipe runs.	Monthly or as required.
	Inspect for evidence of poor operation and/or weed growth. If required take remedial action. Inspect silt accumulation rates and establish appropriate brushing frequencies.	Monthly for 3 months, then six-monthly intervals. Also, 48 hours after large storms.
Remedial Actions	Inspect access/outlet boxes and rod through poorly performing channels and outlets as initial remediation.	As required.
	Seek advice as to remediation techniques suitable for the type of performance issue.	As required, if above actions do not improve performance.
Monitoring	Initial Inspection including channel outlet boxes and gully sumps.	Monthly for 3 months, then six-monthly intervals. Also, 48 hours after large storms.

5.5 Vortex Flow Control Devices

Regular inspection and maintenance is important to identify if the vortex flow control device has become obstructed/clogged and may not be functioning correctly, as failure to do so would expose the development to a greater level of flood risk.

Vortex flow control devices are proprietary products and therefore can vary from manufacturer to manufacturer. As such, where used the manufacturer's recommendations should be followed.

- Normally, little maintenance is required as there are generally a vortex flow control device has no moving parts.
- If blockages occur they tend to do so at the intake of the vortex flow control device.
- Vortex flow control devices are generally fitted with a pivoting by-pass door, which allows the manhole chamber to be drained down should blockages occur.
- The smaller type conical units, below the minimum recommended size, are also supplied with roding facilities or vortex suppressor pipes as standard.
- Following installation of the vortex flow control device it is vitally important that any extraneous material i.e. Building materials are removed from the unit and the chamber.
- After the system is made live, it is recommended that each unit be inspected monthly for three months and thereafter at six monthly intervals with hose down if required. Units should also be inspected within 48 hours after large storms.

5.6 Cellular Attenuation Tanks

Regular inspection and maintenance is important to identify if the inlets/outlets have become obstructed/clogged and may not be functioning correctly, as failure to do so would expose the development to a greater level of flood risk. The maintenance requirements for cellular attenuation tanks are outlined within Table 21.3 of the CIRIA SUDS Manual C753. An extract of that table is given below to set out the maintenance schedule for the cellular attenuation tank component of the proposed drainage systems.

TABLE 21.3 Operation and maintenance requirements for attenuation storage tanks

Maintenance schedule	Required action	Typical frequency
Regular maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action	Monthly for 3 months, then annually
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly
	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary.	Annually
	Remove sediment from pre-treatment structures and/or internal forebays	Annually, or as required
Remedial actions	Repair/rehabilitate inlets, outlet, overflows and vents	As required
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually
	Survey inside of tank for sediment build-up and remove if necessary	Every 5 years or as required

5.7 Porous Paving

Regular inspection and maintenance is important to identify areas which may have become obstructed/clogged and may not be draining correctly, as failure to do so would expose the development to a greater level of flood risk.

Porous paving is a proprietary product and therefore the materials used can vary. As such, where used the manufacturer's recommendations should be followed. The maintenance requirements for porous paving are outlined within Table 20.15 of the CIRIA SUDS Manual C753. An extract of that table is given below to set out the maintenance schedule for the porous paving component of the proposed drainage systems.

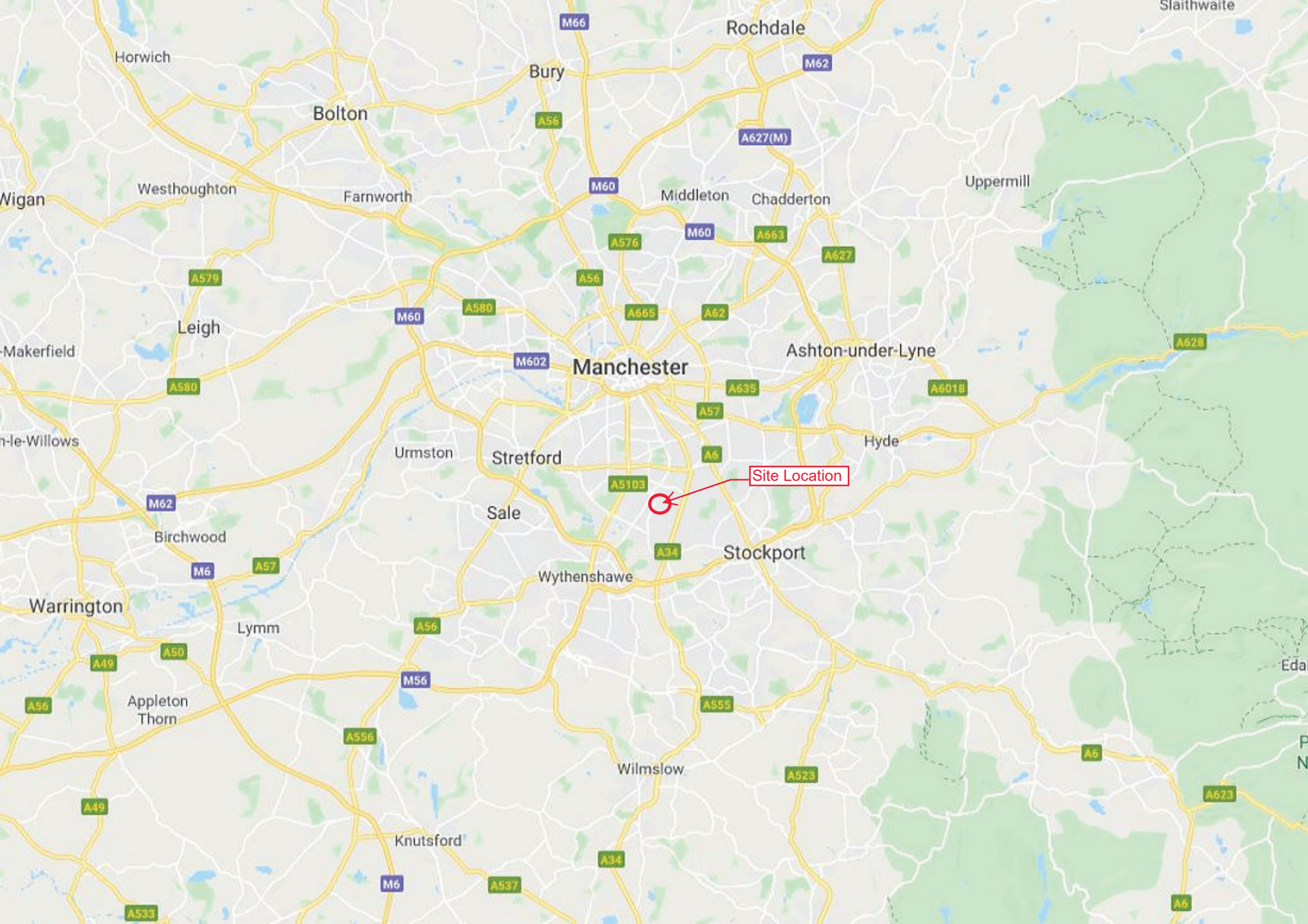
TABLE 20.15	Operation and maintenance requirements for pervious pavements		
	Maintenance schedule	Required action	Typical frequency
	Regular maintenance	Brushing and vacuuming (standard cosmetic sweep over whole surface)	Once a year, after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturer's recommendations – pay particular attention to areas where water runs onto pervious surface from adjacent impermeable areas as this area is most likely to collect the most sediment
	Occasional maintenance	Stabilise and mow contributing and adjacent areas	As required
		Removal of weeds or management using glyphosate applied directly into the weeds by an applicator rather than spraying	As required – once per year on less frequently used pavements
	Remedial Actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50 mm of the level of the paving	As required
		Remedial work to any depressions, rutting and cracked or broken blocks considered detrimental to the structural performance or a hazard to users, and replace lost jointing material	As required
		Rehabilitation of surface and upper substructure by remedial sweeping	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)
	Monitoring	Initial inspection	Monthly for three months after installation
		Inspect for evidence of poor operation and/or weed growth – if required, take remedial action	Three-monthly, 48 h after large storms in first six months
Inspect silt accumulation rates and establish appropriate brushing frequencies		Annually	
Monitor inspection chambers		Annually	

It should be noted that the Main Contractor should provide a Verification Report, including photographic evidence, to demonstrate that the drainage has been constructed as per the design drawings. As-built drawings should be supplied by the Main Contractor, where the construction of the drainage varies from the design construction status drawings.

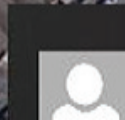
6.0 CONCLUSIONS

- The WYG Environment Geo-Environmental Factual and Interpretative Report, January 2020 and the Reid Jones Partnership FRA, October 2017 indicate that due to the underlying strata infiltration drainage may be suitable across the site. However, Building Regulations Part-H states that no soakaways can be located within 5m of a building or road and therefore, assessing the proposed development site there are limited areas to locate a soakaway or other infiltration techniques 5m from a building or road.
- The WYG Environment Geo-Environmental Factual and Interpretative Report, January 2020 also indicates a relatively shallow groundwater table across the site, at levels between 1.7mbgl – 3.4mbgl. As outlined within CIRIA Report C753, The SuDS Manual, a minimum distance of 1m should be maintained between the base of a soakaway and the maximum groundwater level and therefore the relatively shallow groundwater levels also restrict the use of soakaways.
- Therefore, as a result of Building Regulations Part-H and groundwater restrictions, only infiltration drainage via porous surfaces, across the proposed car park areas is considered to be a viable option for the discharge of surface water, as this would, be 1m above the groundwater. However, this should be confirmed via BRE365 complaint soakaway tests in the areas of the proposed porous paving.
- As the possibility of infiltration has been partially ruled out by the Building Regulations and groundwater restrictions, the intention for the development would be to discharge to watercourse. The nearest watercourse is Shaw Brook which is located approximately 385m north of the site. Due to the distance across various plots of third-party land and properties, discharging to watercourse has been deemed impractical.
- Therefore, the only practical solution for discharge of the surface water is discharge to public sewer. As such, it is proposed that the areas of the site where infiltration is not suitable will drain to public sewer via a connection to the existing private drainage network.
- Using the proposed vortex flow control device, the surface water discharge is to be restricted to the discharge rate of 53.6 L/s, during the 100 year plus 40% climate change design storm, to comply with the LLFA requirements. The surcharged runoff is to be stored using a below ground cellular attenuation tank and the proposed surface water drainage network itself. The network is to prevent any flooding for all rainfall events up to and including the 1 in 100 year + 40% climate change rainfall event.
- No foul water drainage is required across the proposed development, as it is a car park structure with no requirement for foul water drainage.
- The external ground levels around the proposed building shall fall away from the proposed building with any new levels being designed to ensure any exceedance flood routes, for events in excess of the 100 Year + 40% climate change event, exit towards the car park areas and access roads, for use in exceptional circumstances.
- Further investigations, including a survey of the existing drainage connection manhole should be completed to confirm the exact location, size and condition of the existing outfall manhole. The investigations should determine if the existing manhole is suitable for re-use.
- BRE365 complaint soakaway tests are required in the areas of the proposed porous paving (grasscrete) to confirm if infiltration is a viable method for the disposal of surface water runoff for these areas.
- Further detailed discussions with the Lead Local Flood Authority and United Utilities should be completed to ensure agreement of the surface water discharge rate and the overall approach.

Appendix A Site Location Plan



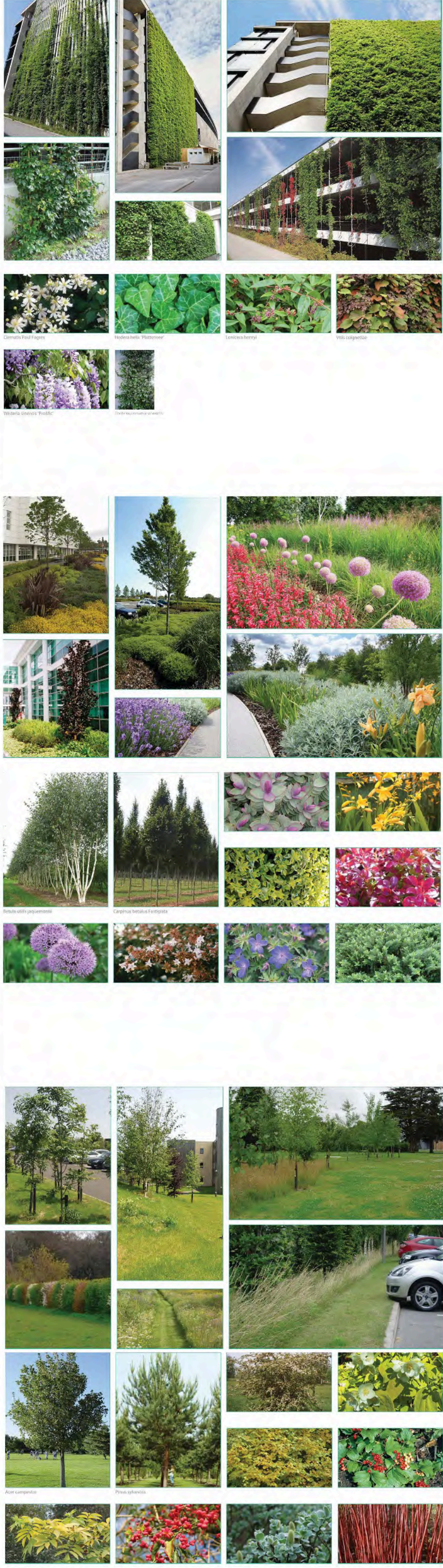
Appendix B Aerial Photo



Appendix C Topographic Survey

Appendix D Proposed Site Plan

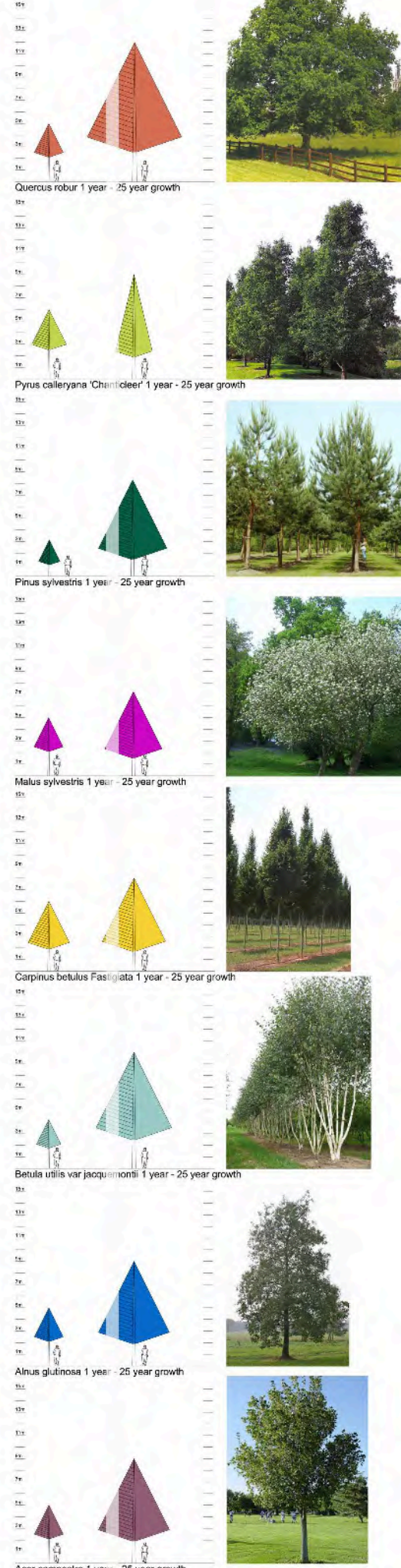
Indicative Shrub Schedule		
	Species	Spread or height (cm)
	Abelia grandiflora	30 - 45
	Allium Purple Sensation	n/a
	Amelanchier lamarkii	90-120
	Berberis thunbergii Rose Glow	30-40
	Berginia cordifolia 'Purpurea'	20-30 (s)
	Buddleia davidii 'Black Knight'	90-120
	Carex ormithosa Evergold	
	Ceanothus thyrsiflorus repens	30-45 (s)
	Choisya ortune Aztec Pearl	45-60
	Clematis 'Paul Forges'	1.75-2.0m
	Cornus alba Sibirica	60-90
	Cornus stolonifera Flaviramea	45-60
	Crocosmia 'Citronella'	n/a
	Euonymus ortune 'Silver Queen'	20-30 (s)
	Geranium Johnson's Blue	
	Hamamelis mollis Pallida	90-120
	Hebe albicans	20-30
	Hebe 'Mrs Winder'	20-30
	Hebe rakiensis	30-45
	Hedera helix 'Plattensee'	1.75-2.0m
	Lavandula angustifolia 'Hidcote'	20-30
	Lonicera henryi	1.75-2.0m
	Lonicera nitia Mai Green	45-60 (s)
	Miscanthus sinensis 'Gracillimus'	
	Pennisetum alopecuroides 'Hameln'	
	Philadelphus coronarius Aureus	45-60
	Phormium Bronze Baby	60-90
	Rosa alba Meidiland*	30-45
	Salix lanata	30-45 (s)
	Sambucus nigra purpurea	60-90
	Trachelospermum jasminoides	1.75-2.0m
	Viburnum bodnantense Dawn	90-120
	Viburnum davidii	30-45 (s)
	Vitis coignetiae	1.75-2.0m
	Wisteria sinensis 'Prolific'	1.75-2.0m
Indicative Semi-Mature Tree Schedule		
	Species	Girth or height (cm)
	Betula utilis 'Jacquemontii'	350-400
	Carpinus betulus Fastigiata	30-35
	Pinus sylvestris	250-300
	Pyrus calleryana 'Chanticleer'	30-35
Indicative ONS Tree Schedule		
	Species	Girth or height (cm)
	Acer campestre	16-18
	Alnus glutinosa	16-18
	Cornus sanguinea	60-90
	Corylus avellana	90-120
	Crataegus monogyna	90-120
	Euonymus europaeus	90-120
	Ilex aquifolium	60-90
	Malus sylvestris	16-18
	Quercus robur	16-18
	Viburnum opulus	60-90



Scale -1:500

Legend

- Proposed Tiered Car Park Surface Finish
- External Grasscrete Finish
- External Asphalt Road Finish
- External Grassed Area
- External Paved Area
- Existing Trees - Retained
- Existing Trees - Removed
- Proposed car park trees with root zone system
- Proposed native tree buffer planting
- Proposed ornamental shrub planting
- Proposed climbing plants with stainless steel tension wire support system
- Semi-native shrub buffer planting
- Proposed native mix hedge



NOTES
All dimensions and levels to be checked on site.
Any discrepancies are to be reported to the Architect before any work commences.
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P 3	04.05.17	SEH	PLANNING ISSUE
P 2	18.08.17	SEH	Issued for Planning Package
P 1	06.08.17	MD	Issued for sign-off

P22 Tiered Car Park

Location

The Christie

Employer

Interserve



Client

Christie

AFL Architects

1st Floor

St Georges House

56 Peter Street

Manchester M2 3AQ

T: +44(0)161 236 6263

F: +44(0)161 236 6484

W: www.afl-architects.com

E: info@afl-uk.com

Twitter: @AFLArchitects

Key plan

Drawing title:

Proposed Landscape Plan

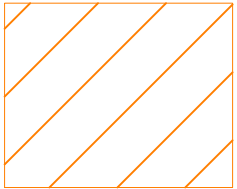
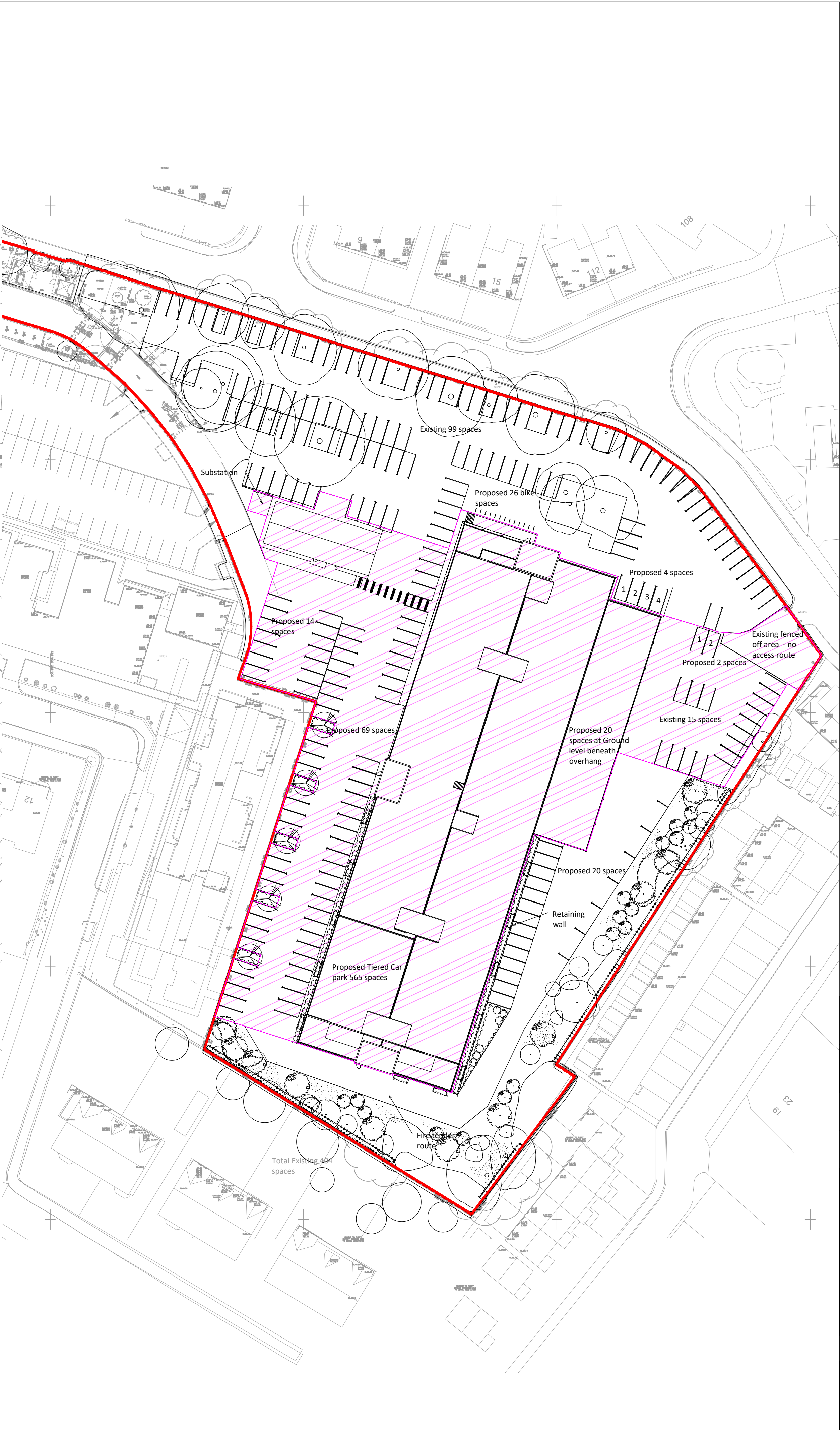
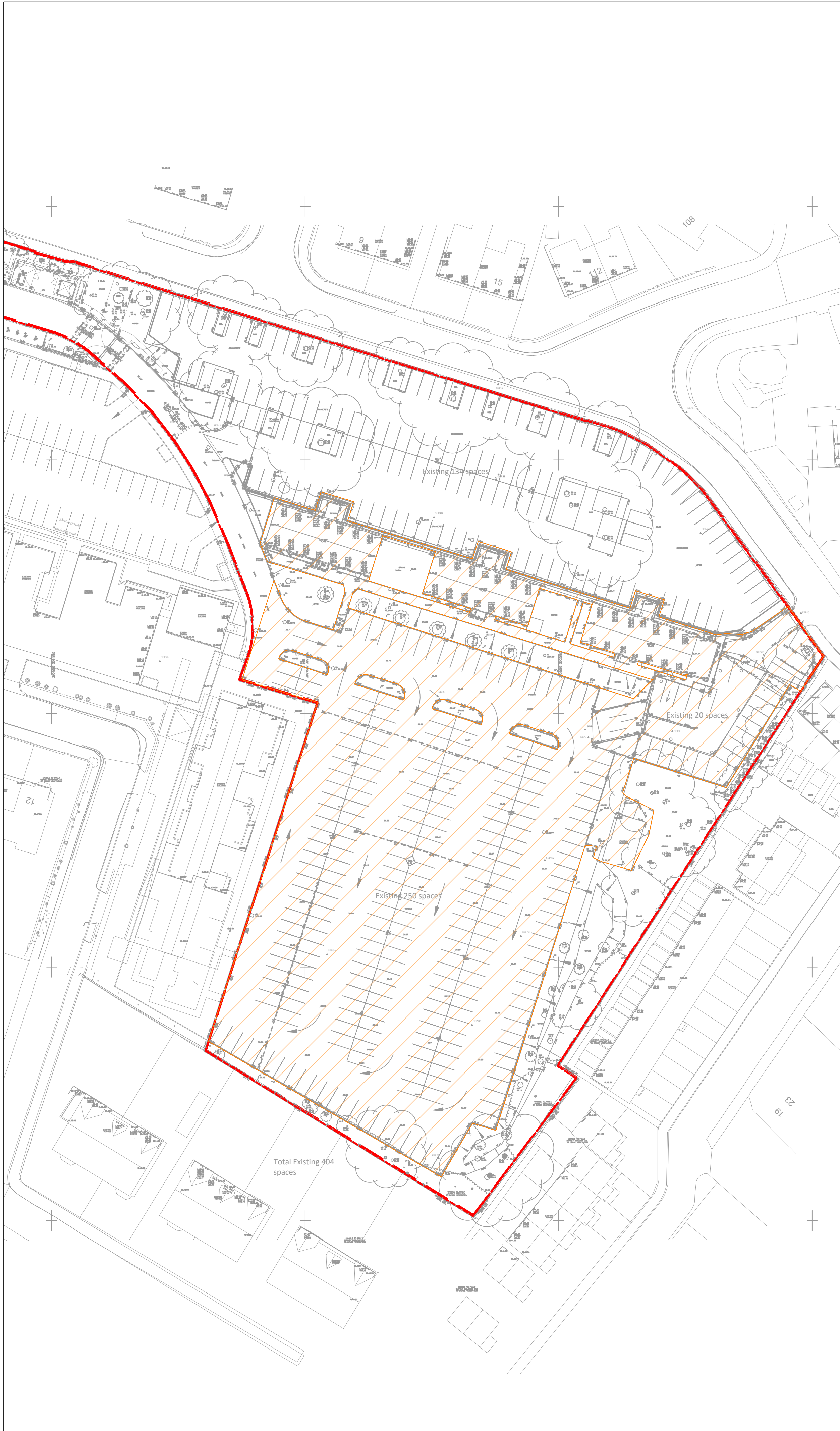
AFL Job number	scale @ A0	drawn: SEH	checked: GR
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163409	rev date: 04.09.17
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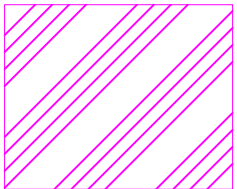
PLANNING	P 3
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Appendix E Pre & Post Development Impermeable Areas Drawing



PRE-DEVELOPMENT
IMPERMEABLE AREAS = 7,629m²



POST-DEVELOPMENT
IMPERMEABLE AREAS = 7,203m²

ALL REMAINING AREAS INSIDE THE 15,647m² SITE BOUNDARY ARE EITHER EXISTING PERMEABLE AREAS, IMPERMEABLE AREAS TO DRAIN AS PER THE EXISTING SCENARIO OR PERMEABLE AREAS TO INFILTRATE VIA GRASSCRETE PAVING AREAS.

REV	DATE	DESCRIPTION	BY	CHK	APP
DRAWING STATUS:					
INFORMATION					
CLIENT:					
THE CHRISTIE NHS FOUNDATION TRUST					
ARCHITECT:					
IBI GROUP					
PROJECT:					
THE CHRISTIE P22 TIERED CAR PARK					
TITLE:					
PRE & POST DEVELOPMENT IMPERMEABLE AREAS					
STATUS:	PROJECT No:	PROJECT	ORIGINATOR	VOL/SYS	LEVEL
S2	218-310	CCP - AJP - ZZ - XX - SK - C - 0001			
					REV:
					P01
SCALE @ A1:	DESIGNED:	DRAWN:	CHECKED:	APPROVED:	DATE:
1:500	JRS	JRS	JMM	JMM	MAR 2020
alan johnston partnership LLP					
CONSULTING STRUCTURAL & CIVIL ENGINEERS					
1 Dale Street, Liverpool L2 2ET					
Tel: 0151 227 1462					
Fax: 0151 225 1187					
E-Mail: Mail@ajpstructeng.com					

Appendix F United Utilities Map & Existing Drainage Information

